

CITY OF PRESCOTT GENERAL ENGINEERING STANDARDS

2016

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ARTICLE 1/ INTRODUCTION

SECTION 1.1/ INTRODUCTION

The City of Prescott (COP) has determined that to provide clear concise direction to design professionals, the development community, and contractors a single point of reference is needed. The City has prepared, and will continue to update, the General Engineering Standards (GES), as deemed necessary. This manual is being provided as a tool to the design community to aid in designs being conducted for the City.

1.1.1 PURPOSE

- A. The purpose of establishing the GES is to provide uniform and functional facilities that ensure health and safety, and enhance the quality of life for the residents of the City of Prescott.
- B. The GES also provides owners, developers and contractors with direction, construction requirements and the City's expectations for the development of infrastructure improvements. The design concepts and specific technical data presented in this Manual are provided to guide and inform the design and development community of the City's expectations.
- C. The goal of the GES is to assist in the orderly development and improvement of property with the goals of protecting the public's health, safety and welfare; improving the long-term value of the City's infrastructure assets; and improving quality of life for the residents and visitors of Prescott.

1.1.2 APPLICABILITY

- A. The GES along with Prescott City Code (PCC), Standard Specifications and Standard Details described herein shall govern all infrastructure improvements for City of Prescott Capital Improvement Projects and private development projects.
- B. This document is supported by various publications, which comprise the standard references for the types of work encompassed in the Public Works Department. In the event of any conflict or discrepancy between the GES, and any of the related publications listed below, the GES shall take precedence.
- C. The following are considered to be Design Standards by the Public Works Department and follow a hierarchy in which the local standard governs. In cases where the local and county standards do not address the design issue; then the appropriate state standard governs. In cases where the local, county and state

standards do not discuss the design issue; then the appropriate federal standard governs.

1. Local Standards.
City of Prescott General Engineering Standards (as amended)
City of Prescott Drainage Criteria Manual (as amended)
City of Prescott Standard Details (as amended)

 2. County Standards
Maricopa County Roadway Design Manual
Maricopa County Association of Governments (MAG) – Uniform Standard Specifications and Standard Details for Public Works Construction

 3. State Standards
Arizona Department of Transportation (ADOT)
 Local Public Agency (LPA) Projects Manual
 Roadway Design Construction Standards and Drawings
 Standard Specifications for Roadway and Bridge Construction
 Arizona Supplement to Manual on Uniform Traffic Control Devices (MUTCD)
 Traffic Signals and Lighting
 Manual of Approved Signs
Arizona Department of Environmental Quality (ADEQ)
Arizona Administrative Code, Bulletin 8 & 10
Arizona Department of Water Resources (ADWR)

 4. Federal Standards
Federal Highway Authority (FHWA)
Manual on Uniform Traffic Control Devices (MUTCD) (with Arizona Supplement)
Federal Emergency Management Agency (FEMA) – National Flood Insurance Program (NFIP)
Clean Water Act
National Environmental Policy Act (NEPA)
- D. The most-recent versions of each of the following are considered incorporated by reference into this document:
1. City of Prescott General Plan

 2. City of Prescott Land Development Code (LDC)

 3. Prescott City Code (PCC)
 Title II Departments, Public Works
 Title III Building Regulations

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| Title VII | Health and Sanitation |
| Title VIII | Public Ways and Property |
| Title IX | Traffic Regulations |
| Title X | Development Regulations |
| Title XII | Subdivisions |
| Title XIII | Floodplain Regulations |
| Title XIV | Off-Site Improvements |
| Title XVI | Street and Utility and Drainage Regulations |

4. Other Publications:

American Association of State Highway and Transportation Officials (AASHTO)

- A Policy on Geometric Design on Highways
- Low-Volume Road Design
- Roadside Design Guide
- Guide for Development of Bicycle Facilities
- Design Handbook for Older Drivers and Pedestrians

American Public Works Association (APWA)

American Society of Civil Engineers (ASCE)

American Society for Testing and Materials (ASTM)

American Water Works Association (AWWA)

Arizona Revised Statutes (ARS)

Arizona State Board of Technical Registration (ASBTR)

Arizona Administrative Code (AAC)

City of Prescott Area Plans, Design Plans and Specific Plans

City of Prescott Bike & Pedestrian Master Plan

City of Prescott Fire Department

City of Prescott Road Pavement Condition Report (Pavement Management System Report)

City of Prescott Traffic Calming Policy

Institute of Transportation Engineers (ITE)

- Residential Street Design and Traffic Control
- Residential Streets
- Traffic Calming – State of the Practice
- Trip Generation: An informational report
- Parking Generation

International Code (IBC)

Local and Regional Transportation Studies/Plans

Local and Regional Drainage Studies

OSHA Construction Safety Orders

Transportation Research Board (TRB)

- Highway Capacity Manual

University of Southern California Foundation for Cross Connection Control and Hydraulic Research (USC)

Other publications and standards may be referenced from time to time as appropriate

SECTION 1.2/ SCOPE

- A. The GES consists of three major components: (1) Design Standards, (2) Construction Specifications, and (3) Standard Details. The Design Standards and Standard Details are presented together in each of the following Articles:
 - 1. Introduction
 - 2. Grading (Site Preparation & Earthwork)
 - 3. Drainage
 - 4. Water
 - 5. Wastewater
 - 6. Transportation/Traffic
 - 7. Dry Utilities
 - 8. Survey
 - 9. Alterations and Modifications
 - 10. Record Drawings (As-builts)

- B. The Standard Details for each of these areas are included in [Appendix B](#) to the document.

- C. The GES establishes the minimum requirements for the design and construction of any public improvement in the City of Prescott, or City of Prescott service area (e.g. Chino Valley (CV), Yavapai County (YC), and Yavapai-Prescott Indian Tribe (YPIT)).
 - 1. Public improvements defined as:
 - a. Infrastructure, which will be accepted for operation and maintenance by the City of Prescott

- b. A City-operated, Special District
 - c. An independent Special District, which does not have its own requirements in these areas
 - d. For any subdivision or land use permit, where the improvement is determined to be of sufficient public benefit that compliance with these standards is required by the conditions of approval
- D. Additionally, the GES establishes requirements for grading on private property. Article 2 provides more information on grading requirements. Final authority for City approval of improvement plans, or acceptance of constructed improvements, rests with the City Engineer.

SECTION 1.3/ REGULATIONS AND PLANNING

1.3.1 CITY CODES, ORDINANCES AND REGULATIONS

- A. The GES clarifies and supplements requirements in the Prescott City Code (PCC), COP Land Development Code (LDC), Maricopa Association of Governments (MAG), Uniform Standard Specifications and Standard Details for Public Works Construction, Arizona Department of Transportation (ADOT) and other local, state and federal codes and regulations applicable for development within the City.
- B. This GES is not intended to interfere with, abrogate, or annul any other ordinance, rule or regulations, statute, or other provision of law except as provided in the GES. Where any provision of the GES imposes restrictions different from those imposed by any other provision of law, the provision that is more restrictive or imposes higher standards upon development and use of land shall control.

1.3.2 COMPREHENSIVE PLANNING

The City has adopted a General Plan, as well as several area specific plans, as planning documents for future development within the City. The City has developed water, sewer, drainage, and transportation master plans to ensure that future infrastructure improvements are achieved through an orderly development of the City planning areas. Developers and engineers shall review these documents to assure any proposed new developments or improvements are in concert with the City's future growth objectives.

1.3.3 OTHER REGULATORY AGENCIES

Regulatory permits and/or agreements may be required by other State and Federal agencies. This includes but is not limited to, the Arizona Department of Water Resources (ADWR), Arizona Department of Environmental Quality (ADEQ), Arizona Department of Transportation (ADOT), Yavapai County Environmental Services (YCES), and the U.S. Army Corps of Engineers. Approval of any improvement plans or construction activity by the City does not exempt the project owner from the requirement to comply with the regulations of any of these agencies. Prior to construction of public improvements, the Developer shall provide copies of all such permits to the Public Works Department, or document that such permits are not required.

SECTION 1.4/ FINANCIAL ASSURANCES

1.4.1 PURPOSE

The posting of financial assurance is a guarantee that funds will be available to the City to complete public improvements, stabilize and re-establish the project site under construction in the event of abandonment by the owner, developer, subdivider or his contractor.

1.4.2 APPLICABILITY

Site improvement guarantees/financial assurances shall be applicable to all non-residential site development, subdivisions, land splits, grading, bank stabilization and hillside development in the City, except as otherwise specified herein.

1.4.3 GENERAL INFORMATION

- A. The amount of the financial assurance shall be based on an estimate of construction costs prepared and sealed by a licensed Civil Engineer in the State of Arizona or through a duly executed construction contract for the work with a licensed and registered contractor that will be performing the work.
- B. The City Engineer or his designee shall review and accept the amount of the engineer's estimate, prior to the posting of the financial assurance.
- C. All financial assurances shall contain a minimum contingency amount of ten (10%) percent of the estimated cost of all public improvements required, unless otherwise noted herein.
- D. Financial assurances, which include erosion/sediment controls, stormwater pollution prevention plan (SWPPP), or permanent slope stabilization measures, shall contain a minimum contingency amount of twenty (20%) percent for work specific to those items.

- E. Financial assurances shall not have an expiration date.

1.4.4 EXEMPTIONS

- A. Single-family residential on a single lot or home site. Two or more single-family lots, which are adjacent to each other and are part of a larger common plan of development, shall not be exempt.
- B. If the total value of the improvements subject to financial assurances does not exceed twenty-thousand (\$20,000) dollars and the contractor to perform the work provides the City with documentation of a performance bond that has a minimum value of twelve-thousand five-hundred (\$12,500) dollars posted with the registrar of contractors, project specific financial assurances may not be required.

1.4.5 FINANCIAL ASSURANCE SUBMITTAL

- A. The owner of the project shall post a financial assurance in an amount in accordance with [Section 1.4.3](#). An application shall be completed and submitted with the financial assurance for consideration by the City prior to the issuance of a permit.
- B. The following forms of assurance will be considered by the City:
 - 1. Performance Bond

A bond executed by an authorized insurance company holding a license to do business in the State of Arizona. A copy of the certificate shall be attached to the applicable bid bonds, payment bond and performance bond to be evidenced by certificate of authority as defined in ARS Sec. 20-217, in a form approved by the City. The Performance Bond shall be approved as to form and legality by the City Attorney and shall have no expiration date.
 - 2. Trust Agreement

Place on deposit in a bank or trust company, in the name of the City, and approved by the City, in a trust account, the cost and the time of completion as estimated by the Public Works Director. Selection of the trustee shall be subject to approval by the City. The trust agreement shall be executed in a form acceptable to the City and approved as to form and legality by the City Attorney.

3. Unconditional Guarantee from Bank

File with the City Clerk a letter, in a form approved by the City, signed by a principal officer of a savings and loan association or bank, acceptable to the City, agreeing to pay to the City, on demand, a stipulated sum of money to apply to the estimated cost of installation of all improvements for which the owner is responsible. The letter shall state the name and description of the project and shall list the improvements that the permittee is required to provide. The unconditional guarantee shall be executed in a form acceptable to the City and approved as to form and legality by the City Attorney.

4. Cash Deposit with City of Prescott

Place a deposit with a financial institution to the City for the improvements in an amount accepted by the City Engineer for the cost of the improvements.

5. Other Forms of Assurance

Any other form of assurance acceptable to the City, and approved as to form and legality by the City Attorney.

1.4.6 RELEASE OF FINANCIAL ASSURANCES

A. Release or partial release of the financial assurance may occur as incremental progress is demonstrated to the satisfaction of the Public Works Director or when the City has formally accepted the improvements that are the subject of such security.

1. Requests for release or partial release shall be submitted on a form provided by the City.

2. A minimum of ten (10) business days shall be required to approve or not approve any written request for the release or draw down of funds.

B. The City Engineer or his designee, shall inspect the construction of improvements while in progress, and, shall likewise inspect such improvements upon completion of construction. The design engineer of record shall certify that construction was completed per the approved plan by submitting a "Certificate of Completion" to the City. Any changes to the approved plan must also have been approved by the City Engineer prior to construction.

C. The City Engineer shall reject any and all construction that fails to comply with the standards and specifications contained in the General Engineering Standards or the Land Development Code (LDC). If the City Engineer rejects such

construction, the City Attorney shall proceed to enforce the financial assurance guarantees provided in the General Engineering Standards.

- D. Upon completion of construction, the permittee shall require from his construction contractors, with whom he contracts for furnishing materials and for installation of the improvements required under this Section, and shall himself be required, to furnish to the City a written guarantee that all workmanship and materials shall be free of defects for a period of two (2) years, the "Warranty Period", from the date of acceptance by the Public Works Director. A minimum of ten (10%) percent of the value of the originally posted financial assurance shall be retained by the City for the two-year warranty period.
- E. If due to climactic conditions, permanent slope stabilization is not completed or established at the time of the final release, an additional ten (10%) percent of the total financial assurance or the actual cost of slope stabilization, whichever is less, shall be withheld pending the completion of slope stabilization, per approved plans, the General Engineering Standards and Land Development Code requirements. At the end of the two (2) year warranty period or when the City determines that the slope stabilization has been achieved, the financial assurance may be released.
- F. At the completion of the Warranty Period the City Engineer or his designee, shall inspect the project for defects or damage. When all repairs are complete to the satisfaction of the City, the City will release the financial assurance in its entirety.

1.4.7 FAILURE TO PERFORM

- A. The City Engineer shall accept construction improvements only if it complies with the General Engineering Standards, Land Development Code, and technical specifications of the City. If the City Engineer does not approve such construction and the applicant fails to make required improvements, the City Attorney shall proceed to enforce the guarantees provided in this section.
- B. The financial assurance may be utilized by the City to complete project construction in the event of abandonment by the owner and his contractor.
- C. If during the warranty period, the City determines that the design and/or construction of the improvements does not accommodate proper traffic and pedestrian circulation and drainage function, or does not function properly, then the owner shall be responsible to reconstruct and/or redesign those improvements to the satisfaction of the City. In the event that the City elects to provide for such reconstruction and/or redesign, then and in that event, the owner shall be obligated to reimburse the City for all such costs incurred. Further, if the owner does not reconstruct and/or redesign those improvements as so

directed, the City may also suspend issuance of permits within the development until corrections to the improvements are provided to the satisfaction of the City.

SECTION 1.5/ DEFINITIONS

In the GES, the intent and meaning of the terms that are used shall be as defined in Section I of the State Specifications except as herein below noted or revised.

- A. *Bedrock*
The solid and/or undisturbed in place either at the ground surface or beneath surface deposits of gravel, sand or soil
- B. *Boulder Stacked Retaining Wall*
The stacking of rocks and boulders on top of each other with layers of soil to form a decorative wall with no imposed load without the benefit of reinforcement.
- C. *Building Official*
The Building Official of the City of Prescott Community Development Department
- D. *City*
The City of Prescott (COP), Arizona
- E. *City Engineer*
The City of Prescott, City Engineer
- F. *Clearing or grubbing*
The removal of any plant, bush, tree, cacti, or earth or rock
- G. *Commercial Use*
The use of persons or property for any fare, fee, rate, charge or other consideration in connection with any business or other undertaking intended for profit, including three or more residential uses (attached or detached) on the same parcel or lot.
- H. *Contractor*
Any person or persons, firm, partnership, corporation or combination thereof, who has/have entered into a contract with any person, persons, corporation, company, special district, or the City of Prescott as party or parties of the second part, or his/her/their legal representatives, for the construction of any public improvement or portion of any public improvement within the City of Prescott or its service area

- I. *Department*
The City of Prescott acting by and through its Department of Public Works, including the Director and his/her duly authorized representatives, either employed by or contracting with the Department, acting within the scope of the particular duties delegated to him/her
- J. *Developer*
The owner or his/her designated representative, of land where any public improvement, is proposed to be constructed
- K. *Director, Public Works*
The Director of the Department of Public Works, City of Prescott. In charge of managing the many aspects of planning, constructing, and maintaining City-funded projects and facilities intended for public use.
- L. *Dry Stack Retaining Wall*
The stacking of bricks or blocks in a manner that interlocks (i.e. versa-lock) for the purpose of retaining (or holding back) soil, earth, or rocks to prevent erosion.
- M. *Earthwork*
The cut, fill, import or disposal of excess earth, sand, gravel, rock or other earthen material
- N. *Easement*
The grant of one or more property rights (e.g., access) by the owner to, or for the use by the public, a corporation, or another person or entity and shall include the total area lying between the exterior edges of the easement.
- O. *Engineer*
Engineer shall mean City Engineer of the Department of Public Works, Engineering Services, of the City of Prescott, acting directly or through his/her duly authorized representatives, either employed by or contracting with the Department, acting within the scope of the particular duties delegated to him/her
- P. *Engineer of Record (EOR)*
The *Project Engineer* designated by the *Developer* as being responsible in charge of civil engineering work as defined in A.R.S Title 32, Professions and Occupations
- Q. *Excavation*
Any act by which earth, sand, and gravel, rock, or other earthen material is cut into, dug, uncovered, displaced or relocated, and shall include the conditions resulting there from.

- R. *Existing grade*
The grade or elevation of the existing ground surface prior to excavating or filling
- S. *FEMA*
The Federal Emergency Management Agency (FEMA)
- T. *Fill*
Deposits of soil, rock, or other materials placed by humans
- U. *Finish grade*
The final grade or elevation of the ground surface after grading is completed
- V. *General Engineering Standard (GES)*
The Design Standards, Construction Specifications and Standard Details, which comprise this volume, along with all other standard references incorporated herein
- W. *Grading*
Any excavating, filling, moving of natural earthen ground surface or combination thereof, and shall include the conditions resulting from any excavation, fill or movement of the natural earthen ground surface, including the natural drainage pattern
- X. *Grading, Mass*
Grading of more than two acres or sixty (60%) percent of a given lot or parcel, whichever is less.
- Y. *Impermeable Cover or Coverage*
Those portions of a lot that are covered by principal and accessory buildings or structures, and by surfaces that prevent the passage or absorption of storm water such as paving and driveways.
- Z. *Laboratory*
Any testing agency or testing firm, which has been licensed by the State of Arizona to act in such capacity, and meeting the requirements of the City Engineer
- AA. *Multi-family Residential*
A building, or portion thereof, having three (3) or more dwelling units on a single lot, used, or designed or intended for use or occupancy as living quarters by three (3) or more families living independently of each other, including all

necessary domestic employees of each family, and having both kitchen or cooking facilities, private, indoor toilet within each such housekeeping unit, and conforming to the adopted building code of the City of Prescott.

- BB. *Natural Grade*
The ground surface (or grade) which is undisturbed by man.
- CC. *NPDES/AZPDES Program* means the Federal National Pollution Discharge Elimination System (NPDES) Program administered nationally pursuant to 33 U.S.C. Section 1342 and the Arizona Pollution Discharge Elimination System (AZPDES) Program administered in Arizona pursuant to Arizona Revised Statutes, Title 49, Chapter 2, Article 3.1, requiring the control or elimination of erosion and other forms of water pollution from a site
- DD. *Pre-Cast Walls*
Concrete walls poured on the ground or manufactured off-site and then raised in place to create a wall.
- EE. *Public Improvement*
Improvements, which are accepted for operation and maintenance by the City of Prescott, including any subdivision or land use permit where the improvement is determined to be of sufficient public benefit that compliance with these standards is required by the conditions of approval
- FF. *Retaining wall*
A wall, that is engineered or designed for the purpose of retaining (or holding back) soil, earth, or rocks to prevent erosion.
- GG. *Right-of-Way*
Any public street, highway, alley, sidewalk or other platted or written easement which has been dedicated to, or accepted by, or acquired by, the City for public purposes.
- HH. *Right-of-Way Permit*
Authorization by the Department to conduct work, reconstruct or place improvement in established or proposed City rights-of-way or easements.
- II. *Rough grade*
An approximate elevation of the ground surface conforming to the proposed design
- JJ. *Service Area*
Those parcels located within an area identified by ADWR and ADEQ that receive utility services (i.e. water and/or wastewater) by the City of Prescott

KK. *Shall*
Mandatory.

LL. *Soil*
All earth material of whatever origin that overlies bedrock

MM. *State*
When the State Specifications are applicable, the word "State" as used in the State Specifications shall mean the State of Arizona and its political subdivision, City of Prescott

NN. *State Standard Specifications*
The latest edition of the ADOT Standard Specifications for Road and Bridge Construction, and of the associated Standard Plans (ADOT Roadway Design Construction Standard Drawings), of the State of Arizona, Department of Transportation(ADOT)

ARTICLE 2/ GRADING

SECTION 2.1/ INTRODUCTION

2.1.1 PURPOSE

- A. The purpose of this Article is to promote health, safety, and public welfare by establishing minimum requirements for regulating grading activities. This Article provides standards, guidelines and design criteria for the design and construction of grading, drainage and site restoration projects within the City of Prescott (COP). Article 2 sets forth rules and regulations to control earthwork construction, including excavation embankments and drainage and establishes the administrative procedure for issuance of permits; and provides for approval of plans, specifications, and inspection of such construction.
- B. This Article is complimentary to the Prescott City Code (PCC), the Land Development Code (LDC) and other Articles of the General Engineering Standards (GES).
- C. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

2.1.2 APPLICABILITY

- A. The standards and guidelines of this Article shall apply to any moving, relocating, cutting or filling of earth within the City of Prescott. A grading and drainage plan and permit are required under the following circumstances:
 - 1. For any cut and/or fill of dirt that exceeds fifty-(50) cubic yards (CY), or clearing and grubbing of more than ten-thousand (10,000) square feet
 - 2. A grading permit may also be required for grading which is less than fifty-(50) CY, if the work impact's drainage on adjacent upstream and/or downstream properties, and/or for a structure
- B. The guidelines of this section shall not apply to the following development activities:
 - 1. Selective thinning of vegetation for fire control as approved by the Fire Department

2. Individual single-family home construction may be exempt, where slopes are less than twenty (20%) percent or where the site will not be subject to mass grading if meeting the requirements above

2.1.3 DESIGN STANDARDS AND GUIDELINES

A civil engineer registered in the State of Arizona shall analyze, design and seal all grading and drainage plans or associated analysis for projects that meet the requirements noted above.

2.1.4 STATE REGULATIONS

- A. Permits issued under the requirements of this Article shall not relieve the permittee of the responsibility for securing required permits for work to be done that is regulated by any other ordinance, department, or division of the City of Prescott or other governing agency.
- B. All grading work is subject to the requirements of National Pollution Discharge Elimination System (NPDES) and Arizona Pollution Discharge Elimination System (AZPDES). Refer to GES, Article 3, Storm Water Management, and PCC, Title 16.
 1. Before a grading permit is issued, the developer shall submit to the City Engineer a Notice of Intent (NOI), issued by the Arizona Department of Environmental Quality (ADEQ).

SECTION 2.2/ GENERAL INFORMATION

- A. The issuance of a permit or the approval of construction documents and specifications shall not be construed to be a permit for, nor an approval of any violation of or deviation from the provisions of this Article or any other ordinance, law, rule or regulation.
- B. If materials are washed or deposited upon dedicated streets or alleys or other public property as a result of improperly controlled grading of higher or adjacent lands, the City Engineer shall notify the owner or his authorized agent, if known, to remove such materials and restore the dedicated street or alley or other public property to their original condition in accordance with the conditions set forth by the City Engineer but in no case shall compliance with said condition exceed ten (10) days from receipt of the notice to comply.

- C. No clearing and/or grubbing of a site shall occur without first receiving a grading permit. When clearing and/or grubbing has commenced, the site shall be continuously worked until all infrastructure is completed and all erosion control measures and slope stabilization have been completed.

SECTION 2.3/ GRADING STANDARDS AND DESIGN CRITERIA

2.3.1 GRADING STANDARDS

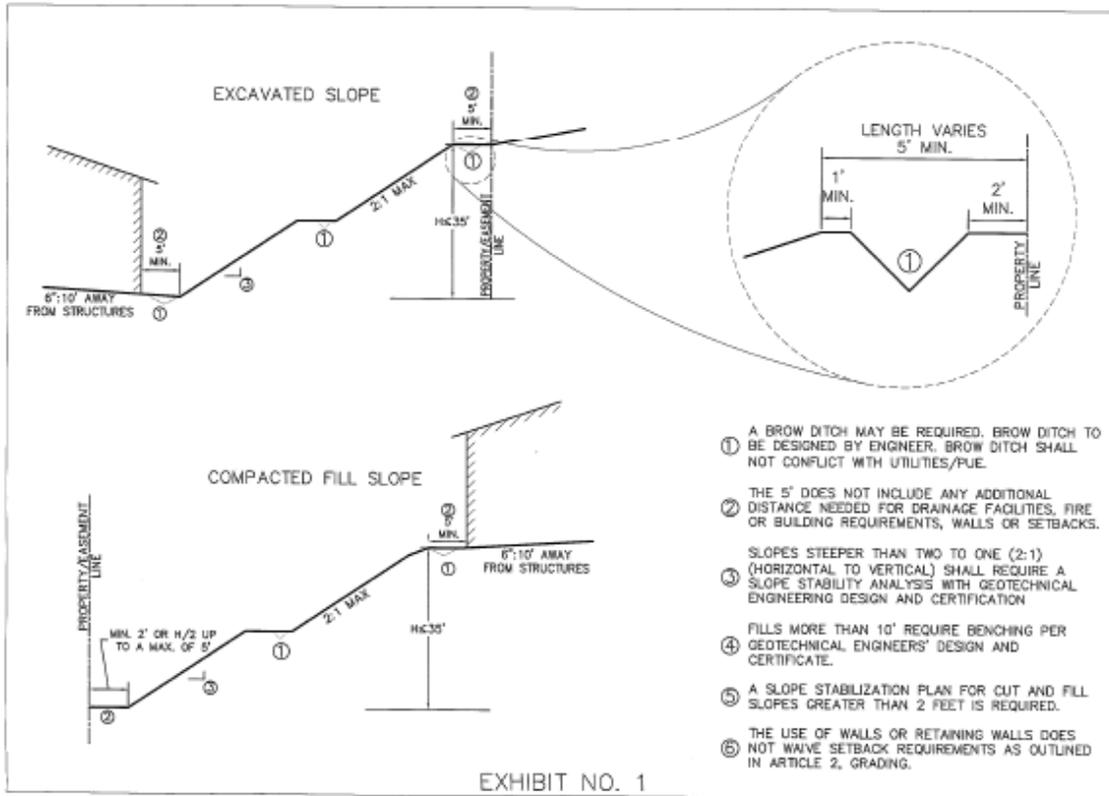
- A. Portions of the site or lot to be disturbed shall be clearly identified on the grading plan as the “disturbed area(s)” or “disturbed area envelope”. Grubbing, grading, and clearing may only occur within the disturbed area(s) identified on the approved grading plan. Portions of the site or lot not identified as disturbed area(s) may not be used for stockpiling materials.
 - 1. All corners of the proposed site shall be clearly staked by a Registered Land Surveyor before any clearing, grubbing or grading begins.
 - 2. For larger areas of development, where grading will not occur within two-hundred (200’) feet of the property boundary, an alteration or modification per GES, Article 9, may be requested.
- B. Drainage shall be directed away from all structures per the International Building Code.
- C. All residential development shall comply with the following requirements:
 - 1. The maximum height of an exposed vertical cut or fill shall not exceed thirty-five (35’) feet and will require a design and certification from a Registered Geotechnical Engineer
 - 2. The maximum length of an exposed vertical cut of fill shall not exceed one-hundred fifty (150’) feet
- D. Commercial development shall not be restricted in terms of height of cut or fill, subject to soil stability analysis and scarring mitigation.
- E. All fills shall be compacted per [Table 2-1](#). Compaction testing frequency shall be every two (2’) feet in elevation (vertical) and every two-hundred (200) feet linearly (horizontally). At the time of the building permit, a geotechnical engineer certification may be required for all structures.

**TABLE 2-1
FILL COMPACTION**

| LOCATION | MINIMUM DENSITY* |
|--------------------------------|-------------------------|
| Within the Building Envelope | 95% |
| Parking Lots, Driveways, Roads | 95% |
| Yards, Open Space | 75% |

*Unless a modification is indicated based on a geotechnical report, seal by a Registered Geotechnical Engineer.

- F. The area over which fills are to be made shall be cleared of all trash, trees, stumps, timber, debris or other material not suitable as a foundation for fill.
- G. Edges of cuts and fills shall conform to the natural topography of the land by reshaping.
- H. Cuts and fills on slopes that encroach into a local drainage tributary (i.e., non-FEMA stream) shall be identified and armored to protect against a one-hundred (100) year storm event.
- I. Excess material shall be handled per the City approved grading plan, or removed to a City approved off-site disposal area.
- J. All site re-vegetation shall be completed within ninety-(90) days of completion of grading work, or prior to release of Financial Assurances or issuance of a certificate of occupancy, whichever occurs first.
- K. Where natural areas or preserved vegetation is designated on a site or lot, temporary fencing shall be installed where they abut construction areas in order to prevent site disturbance of natural areas.
- L. Cut and fill slopes shall be shown on the plans. Slopes steeper than two to one (2:1) (horizontal to vertical) or steeper shall require a slope stability analysis prepared by a Registered Geotechnical Engineer. Additionally, slope stability analysis for slopes flatter than two to one (2:1) may be required at the discretion of the City Engineer.
- M. The top of cut slopes shall be located a minimum of five (5') feet inside the subject property boundary. Buildings located at the bottom of a cut slope shall be located a minimum of five (5') feet away from the hinge point of the cut slope and shall be sloped a minimum of six (6") inches per ten (10') feet (6":10') away from the building. Refer to Exhibit, No 1.



- N. The toe of fill slopes shall not extend into public right-of-way (ROW), adjacent property, proposed easements or existing easements. The toe of fill slopes shall be setback inside the subject property line a minimum of two (2') feet or one-half (1/2) the fill height up to a maximum of five (5') feet.
- O. Buildings located at the top of a fill slope shall be located a minimum of five (5') feet away from the hinge point of the fill slope and shall be sloped a minimum of six (6") inches per ten (10') feet (6":10') away from the building.
- P. Fill placed over existing terrain steeper than four to one (4:1) (horizontal-to-vertical) shall be supported on horizontal benches cut into native competent material. The Registered Geotechnical Engineer shall provide recommendations for the placement of material and bench design.
- Q. Brow ditches shall be provided on the top of slopes to prohibit drainage over and/or down the slopes. The drainage shall be conveyed to a natural or manmade drainage system to minimize erosion.
- R. The natural historical drainage patterns shall be maintained at the property lines. This would include drainage locations, flow rates, velocity, sediment transport and yield, and the direction of flow off the property.

- S. All culverts and backfill materials shall meet minimum H2O loading requirements per the American Association of State Highway and Transportation Officials (AASHTO), including those culverts located at driveways.
- T. Slope stabilization is required for all grading work with cut and fill slopes two (2') feet or higher. Single-family Subdivision, Single-family development on individual lots, commercial site developments, commercial subdivisions, single commercial sites disturbed prior to building permit, public and private rights-of-way shall include slope stabilization and/or landscape plans for all areas disturbed during construction. Plans for the slope stabilization shall be submitted as a part of the civil infrastructure plans or commercial building plans.
 - 1. Bank Stabilization Standards
 - a. A bank stabilization plan, for cut and fill banks two (2') feet high, shall be included with project permit applications (e.g. Building Permit, Site Disturbance and Grading Permit, Paving Permit).
 - b. A landscape Architect or Licensed Landscaper shall prepare stabilization plans for banks greater than eight (8') feet, visible from streets and shall submit such plans for City approval. Where structural designs are required, a registrant shall design and seal the plan.
 - 2. Bank Stabilization Options
 - a. Re-vegetated banks are required where practical and may include combination of reseeding/seed mats, trees, shrubs, groundcover, rock and riprap. Plant type, size and coverage shall be sufficient to stabilize and begin restoring within three (3) years.
 - b. Temporary automatic irrigation is required to establish plants and permanent drip may be required for highly visible planted banks. Planted surfaces shall be amended with topsoil a minimum three to six (3"-6") inches deep, and shall be sufficiently laid back to support plantings.
 - i. A minimum of two to one (2:1) (horizontal-to-vertical) north facing, two and one-half to one (2.5:1) (horizontal-to-vertical) east facing and three to one (3:1) (horizontal-to-vertical) south and west facing to permit terracing with steeper faces between planted platforms.

- c. Manufactured banks greater than eight (8') feet high, visible from nearby streets or neighborhoods, shall be formed to create a more natural appearance (a combination of rough-cut, undulating, and/or rounded toe and top of slope), and terraced where possible to improve plant establishment.
- d. Retaining walls, where visible from streets or nearby neighborhoods, shall be terraced or treated to blend into the natural landscape (e.g. tinted concrete, rock facing, slump block, railroad ties, and/or plantings). Unfinished poured concrete and unfinished concrete block walls are not permitted where visible from streets and nearby neighborhoods.
- e. Un-planted banks are permitted for exposed bedrock cuts, or where the building will effectively screen the cut and/or fill bank in a timely manner. Where bedrock cuts higher than eight (8') feet are visible from streets or neighborhoods, they shall be formed to create a more natural appearance (a combination of rough-cut, undulating, and/or rounded toe and top of slope). Additional treatment of cut faces may be required to stabilize soil and reduce scarring effects, based on proven effective technologies and products.
 - i. All bedrock cuts higher than eight (8') feet must be included in a geotechnical report, including soil-loading calculations, and sealed by a Professional Geotechnical Engineer.
- U. All material placed, as fill shall be bladed and graded to a uniform surface.
- V. Fills in non-hillside areas that are made for yard purposes and which do not exceed eighteen (18") inches in depth are not required to be compacted.
- W. The material to be used for fill shall be approved by a Registered Geotechnical Engineer prior to use when the fill is to support the foundations of structures. No organic material shall be permitted in fills. When the fill material includes large rocky or hard lumps, such as hardpan or cemented gravel, which cannot be broken readily, such material shall be well distributed throughout the fill. Sufficient earth or other fine material shall be placed around the larger material as it is deposited to fill the interstices and produce a dense, compact fill. However, such material shall not be placed within two (2') feet of the finished grade of the fill.
- X. Fills greater than ten (10') feet will require benching, per an analysis and design, performed by Registered Geotechnical Engineer.

SECTION 2.4/ PLAN SUBMITTAL REQUIREMENTS

2.4.1 SUBMITTALS

- A. The applicant shall complete a grading permit application.
- B. Three (3) copies of grading and drainage plan, a minimum of twenty-four by thirty-six (24" X 36") inches in size, on bond paper. The plans shall include the following:
 - 1. Existing contours at a maximum interval of two (2') feet with elevation labels at ten (10') foot intervals, which extend at least one-hundred (100') feet beyond the perimeter of the property
 - a. Spot elevations shall be shown where the contours do not clearly depict the slope or direction of drainage flow
 - b. The City Engineer may modify this requirement per the GES, Article 9, based on the existing topography or scope of the project
 - 2. Location of all existing structures, walls (free standing and retaining), curbs, gutters, sidewalks, streets, all utilities, wells, culverts, channels or other drainage improvements
 - 3. Property lines and all existing recorded easements
 - 4. City/County jurisdictional lines
 - 5. The one-hundred (100) year Federal Emergency Management Agency (FEMA) designated Special Flood Hazard Area (SFHA) limits shall be delineated
 - 6. Proposed culverts, include location and dimensions of detention basin(s), and drainage structures or improvements
 - 7. Finish pad and finish floor elevations, "hinge point" elevations and invert elevations for culverts, channels and/or drainage swales
 - 8. Profile for all culverts, manholes and inlet structures, showing both existing ground and proposed finished ground/grade profile, for the entire length of culvert
 - 9. Inlet and outlet elevations for all culverts, manholes and inlet structures

10. All proposed cut and fill slopes with (horizontal-to-vertical) slope
 11. A profile depicting the existing and proposed grading, with all proposed slopes, clearly labeled
 12. Walls, both free standing and retaining shall have elevations for top of footing and top of wall
 - a. Walls and footings shall be installed entirely within the property on which they are permitted
 - b. Wall and footings on lots located entirely within a subdivision development that is under ownership of the developer, may be on the shared property line of the subject parcel(s)
 13. Estimate the cubic yards of material to be moved on the project including both import and export
- C. Drainage Analysis (Refer to General Engineering Standards (GES), Article 3, Storm Water Management).
- D. A Notice of Intent (NOI) issued by ADEQ.
- E. Geotechnical Report. The report shall identify structural soil properties and recommendations for compaction. A minimum ninety-five (95%) percent of maximum density and structural fill is required. If the site requires import, the Geotechnical Report must provide recommendations for compaction and structural fill of the import material.
- F. Projects, which disturb one (1) acre or more or are part of a larger common plan of development, or subdivision that would result in the accumulated disturbance of one (1) acre or more will require a complete Storm Water Pollution Prevention Plan (SWPPP) binder in conformance with the Arizona Department of Environmental Quality (ADEQ). The SWPPP shall include a Notice of Intent (NOI) from ADEQ and a Sediment and Erosion Control Plan, which includes permanent stabilization and/or landscape plans. These documents shall be submitted to the City prior to a permit being issued.
- G. The Engineer of Record (EOR) shall provide the location of any site, which is providing import to the project, or any site to which export is taken. A grading plan or stockpile plan including a SWPPP to control erosion at the off-site location shall be provided for all off-site import/export locations. The grading contractor shall provide this information prior to a permit being issued.

- H. Any other items deemed necessary by the City Engineer.

SECTION 2.5/ PRELIMINARY GRADING PERMIT

2.5.1 PRELIMINARY GRADING PERMIT

- A. A “Preliminary Grading Permit” is defined as a grading permit that is issued:
 - 1. Prior to recording of the Final Plat
 - 2. Prior to issuance of a permit for approved civil improvement plans
 - 3. Prior to issuance of a permit for approved building permit
- B. The City of Prescott may consider requests for a “Preliminary” grading permit, on an individual project basis, under specific conditions as follows:
 - 1. Each request will be considered independently on its own merit. In no case shall a preliminary grading permit be guaranteed for every project
 - 2. If the applicant has met the minimum requirements of this code and the issuance of a preliminary grading permit will not be detrimental to surrounding property owners the City Engineer may, at their discretion, issue a preliminary grading permit.
- C. Application for a preliminary grading permit shall include all items outlined in the GES, [Section 2.4](#).

SECTION 2.6/ GRADING PERMITS

- A. The Grading Permit may be issued when a complete submittal per GES, [Section 2.3](#) and [2.4](#) have been submitted, reviewed and approved by the City of Prescott Public Works Department. The issuance of a grading permit shall constitute an authorization to do only that work which is described or illustrated on the permit application, plans and specifications.
- B. Financial Assurances are required at the time of permit issuance in an amount equal to one-hundred and ten (110%) percent of the engineer’s estimate of cost for grading and one-hundred and twenty (120%) percent of the engineer’s

estimate of cost for slope stabilization, as stated in the City of Prescott Land Development Code (LDC).

- C. Grading Permits are valid for a period of one (1) year from the date they are issued. If the work has not commenced within one (1) year of the issued date, or the work has been discontinued for more than one (1) year from the last “Approved” inspection, the permit will expire.
 - 1. If the permittee is unable to complete the work within the specified time, a request may be made prior to the expiration of the permit in writing, to the City Engineer for an extension of time. The letter shall set forth the reasons for the requested extension. If, in the opinion of the City Engineer, such an extension is warranted, he/she may grant additional time for the completion of the work.
 - 2. If a permit expires, the permit must be re-submitted for review and approval prior to any work commencing. Expired plans and permits shall adhere to the most currently adopted Prescott City Code (PCC), General Engineering Standard (GES), Land Development Code (LDC), Standard Details, Specifications and fees.

- D. In granting any permit under this Article, the City Engineer may attach such conditions as may be reasonably necessary to prevent creation of a nuisance or hazard to public or private property. Such conditions may include, but shall not be limited to:
 - 1. Requirements for fencing of excavations or fills, which would otherwise be hazardous
 - 2. Improvement of any existing site condition to bring it up to the standards of this Article
 - 3. Temporary fencing of protected hillside and desert preservation areas, prior to commencing grading operations

- E. Neither the issuance of a permit under the provisions of this Article nor the compliance with the provisions hereof, or with any conditions imposed in the permit issued hereunder, shall relieve any person from responsibility for damage to other persons or property, nor impose any liability upon the City for damage to other persons or property.

SECTION 2.7/ FLOODPLAIN

If the site is impacted by a FEMA designated floodplain or floodway, refer to Prescott City Code (PCC), Title 13.

SECTION 2.8/ BLASTING

No blasting shall occur within the City Limits without a Permit first being issued from the City of Prescott Fire Department. Copies of the 2008 City of Prescott Explosives/Blasting Standards, a supplement to the 2006 International Fire Code can be obtained by contacting the City of Prescott Fire Department at 928-777-1714. This standard outlines the requirements for issuance of a Blasting Permit. Prescott City Code (PCC), Title 5-4-2 (E), also regulates blasting activities. Additional restrictions and City Council approval are required to extend blasting hours.

SECTION 2.9/ RETAINING WALLS

- A. Retaining walls, of any type shall require a site plan approval and/or a permit from the City of Prescott Building Division per [Table 2-2](#)

**TABLE 2-2
PERMIT REQUIREMENTS FOR RETAINING WALLS**

| | Site Plan Only Approval | Building Permit Required | Design/Sealed by Structural Engineer |
|--------------------------|--------------------------------|---------------------------------|---|
| Retaining Wall | 2 feet high or less | More than 2 feet high | More than 2 feet high |
| Dry Stack Retaining Wall | 4 feet high or less | More than 4 feet high | More than 4 feet high |
| Boulder Stacking | 2 feet high or less | More than 2 feet high | More than 2 feet high |
| Pre-Cast Walls | All – regardless of height | All – regardless of height | All – regardless of height |

- B. The height of the retaining wall shall be measured from the finish grade at the bottom of footing to the top of the wall.
- C. Retaining walls less than two (2') feet that have a surcharge loading from structures and/or vehicles require a permit. All retaining walls shall require a design from a Registered Professional Engineer.

- D. Retaining walls used in conjunction with or adjacent to stormwater conveyance or detention facilities, and subject to inundation or flow, shall be designed for such conditions.

2.9.1 PERMIT DOCUMENTS

- A. Completed grading permit application.
- B. A site plan showing property lines, existing structures, easements, streets, right-of-ways, required setbacks, and both natural and proposed drainage features.
- C. Retaining Walls:
 - 1. Show location(s), cross section(s), footing, height, finish grade elevations at base and top of wall, backfill and drainage provisions
 - 2. Structural analysis and design prepared by a licensed engineer
- D. Segmented “dry stacked” Walls:
 - 1. Show location(s), cross section(s), footing, height, finish grade elevations at base and top of wall, backfill and drainage provisions
 - 2. Include manufacturer’s specifications, design, structural analysis, and construction recommendations
 - 3. Structural analysis and design prepared by an Arizona Registered Professional Engineer
- E. Stacked Boulder Wall:
 - 1. Location(s) and cross section of the boulder wall showing boulder size, embedment, height, batter, backfill, drainage, slope of the ground, and any surcharge loading
 - 2. Structural analysis and design prepared by an Arizona registered Professional Engineer

2.9.2 INSPECTION AND TESTING

- A. The Design Engineer and Geotechnical Engineer shall perform special inspection of the retaining wall construction and compaction testing of the backfill material.

- B. Upon completion of the retaining wall, the Structural Engineer and the Geotechnical Engineer shall submit a stamped and sealed certification stating the wall was constructed per the approved civil plans and specifications. The certification shall include the compaction results.

2.9.3 EXCEPTIONS

- A. Retaining Walls less than two (2') feet in height that are used for landscaping or decorative purposes with no surcharge shall not require a Structural Analysis or Geotechnical Report.
- B. Slopes that are steeper than two to one (2:1) (horizontal-to-vertical) shall require stabilization to control erosion using approved methods a soil analysis prepared by a licensed Geotechnical Engineer.

ARTICLE 3/ DRAINAGE

SECTION 3.1/ INTRODUCTION

3.1.1 PURPOSE

- A. The purpose of this Article is to regulate the analysis and design of all stormwater management facilities within the corporate limits of the City of Prescott, Arizona. In this context, the term stormwater includes consideration of all aspects, including capture, conveyance, and quality of such waters. Although we have attempted to place all stormwater related requirements into one section, the Prescott City Code (PCC) includes other sections where stormwater requirements are stated. In cases of conflict with other City ordinances, the more restrictive or conservative of the two will apply.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

3.1.2 APPLICABILITY

The standards and guidelines of this Article shall apply to the analysis and design of all stormwater related facilities for both public improvement and private development and redevelopment projects, within the City of Prescott.

3.1.3 DESIGN STANDARDS AND GUIDELINES

- A. Use of this Article does not supersede the need for acquiring various permits or authorizations required by the Federal Emergency Management Agency, Environmental Protection Agency, U.S. Army Corps of Engineers, State of Arizona, Yavapai County, other City of Prescott departments, or any other governing agency having jurisdiction. It is the responsibility of the project owner and project professionals and contractors, to comply fully, with all applicable regulations.
- B. No person shall alter any natural drainage course or existing drainage facility, or commit any act, which would result in the alteration of natural drainage courses or existing drainage facilities, in such manner as might damage or endanger by flooding erosion, or any other means, any public right-of-way, public easement, or public property or improvements.

3.1.4 GENERAL INFORMATION

- A. This Article is intended to act as a guide for implementing the policies and design criteria specific to the City of Prescott. It represents a minimum standard that the City has determined to be appropriate for a wide variety of projects within the City that takes into account the unique terrain, weather, and experiences of the Prescott area. The full responsibility for adequacy of the design remains with the Engineer of Record (EOR). Approval by the City does not alter that professional liability.

- B. This Article refers to hydrologic and hydraulic analysis methods and design procedures contained in reference manuals published by various Federal, State, and County government agencies. The intent is to utilize those public resources where design methodologies are thoroughly presented, and widely accepted.

SECTION 3.2/ DRAINAGE REPORTS & PLANS

3.2.1 PURPOSE

- A. The purpose of this section is to present criteria for submittals of drainage reports, floodplain studies, grading and drainage plans, and public drainage improvement plans to the City of Prescott for review and approval.

- B. Drainage reports and engineered grading plans are required to:
 - 1) Analyze the impact that the proposed project will have on stormwater discharges

 - 2) Provide adequate data to ensure that the project is designed to be protected from flooding and conforms to applicable floodplain and stormwater management regulations

 - 3) Provide data for the design of public and private drainage facilities

- C. Drainage reports shall be of sufficient detail to demonstrate that the development or project will not create drainage or flooding problems and that any on-site drainage facilities are properly sized to detain and/or convey the design storm flows.

3.2.2 DRAINAGE REPORTS AND PLANS REQUIRED

- A. Drainage reports and construction plans will be required for the following land development activities:

1. All subdivisions (residential and non-residential)
 2. Any commercial, industrial, or multi-family site improvements
 3. Public improvements involving new streets, culverts, stormdrains, open channels, private/public detention facilities, or other drainage infrastructure
 4. Other projects at the discretion of the City Engineer.
- B. A drainage report for subdivision activity shall be submitted with all Preliminary Plat and Final Plat submittals, per the Land Development Code (LDC).
- C. A drainage report is not required for individual single-family residential sites, unless there are significant off-site flows that enter the site.
- D. For phased developments, drainage reports and plans shall clearly identify each phase, the intended sequence, and coordination of the work to ensure that each phase is adequately served by drainage and detention improvements.
- E. A drainage report is not required for minor site modifications that have no impact to the existing drainage paths and/or systems, at the discretion of the City Engineer.
- F. A drainage report may be required for any building permit, floodplain use permit, or grading permit if the City Engineer determines site conditions warrant its necessity.
- G. Drainage reports and construction plans submitted for review, shall be prepared and sealed, by an Arizona Registered Professional Civil Engineer.
- H. Final drainage reports shall include a data disk (CD) containing all calculational spreadsheets and models, as well as a pdf version of the final sealed report.

3.2.3 PRELIMINARY DRAINAGE REPORT

- A. A preliminary drainage report shall be submitted at the preliminary plat phase for subdivisions, when required by the LDC, or when requested by the City Engineer.
- B. The purpose of the Preliminary Drainage Report is to conceptually define the nature of the proposed development or project, describe all existing drainage conditions and anticipated proposed conceptual drainage facilities needed to conform to the requirements of Prescott City Code (PCC).

- C. The preliminary drainage report shall be written as a stand-alone document.
 - 1. When references are made or assumptions are based on previously approved reports, the preliminary drainage report must include the appropriate excerpts, pages, tables, and maps containing the referenced information.
 - 2. Assumptions made in previous drainage reports must be verified and substantiated.
 - 3. All previously, approved reports submitted should be clearly and cleanly reproduced. Photocopies of charts, tables, nomographs, calculations, or any other referenced material must be legible.

- D. The following shall be included in a Preliminary Drainage Report:
 - 1. Cover Page
 - a. Project name, address, and COP permit number (once assigned)
 - b. Date of report and any revision dates
 - c. Name, address, and phone number of developer/owner
 - d. Name, address, and phone number of engineering firm
 - e. Seal/Signature of Arizona Registered Professional Engineer responsible for preparing the report
 - 2. Table of Contents
 - a. List Section Headings and their Respective Page Numbers
 - b. List of Tables with Page Numbers
 - c. List of Figures with Page Numbers
 - d. List of Attachments, Numbered
 - 3. Location of Project
 - a. Township, range, section, $\frac{1}{4}$ section, subdivision, and parcel number(s)

- b. City, county, state highway and local streets within and adjacent to the site or the area to be impacted by the drainage improvements
 - c. Names of surrounding developments, land use, and zoning classifications
4. Description of Property
- a. General project description, including existing and proposed land use
 - b. Topographic features, slopes, and acreage
 - c. Existing drainage and irrigation facilities such as ditches, canals, channels, culverts, pipes, ponds, headwalls, and other improvements
 - d. Existing drainage easements within and adjacent to the site
 - e. Identify the Federal Emergency Management Agency (FEMA) Special Flood Hazard Area (SFHA) Zone Designations that affect the property
 - f. Identify and discuss previous studies or reports that pertain to the project area.
5. Major Drainage Basins and Sub-basins (onsite and off-site)
- a. Describe and discuss all tributary drainage basins and sub-basins affecting or being affected by the development, both onsite and offsite.
 - b. Existing and planned land uses within the basins.
 - c. Discuss soil types and hydrologic group information for all soils within the tributary areas, and the source of the information.
 - d. Identification of all existing drainage facilities and/or detention basins that contribute to, or will be affected by, the proposed project within one-hundred (100') feet of the property.
 - e. Historic drainage patterns of the property in question

6. Drainage Design Criteria

a. Hydrologic Analysis and Design

- 1) Identify runoff calculation methodology
- 2) Identify, discuss, and summarize all modeling assumptions and input parameters, including rainfall, soil types, vegetative cover, and other variables pertinent to the calculations.
- 3) Compute existing and developed peak flows for all sub-basins for the two (2)-year, ten (10)-year, twenty-five (25)-year, and one-hundred (100)-year design storm events
- 4) Summary table of pre and post-development peak discharges for all sub-basins, concentration points, and locations where flows enter and exit the development site.

b. Hydraulic Analysis and Design

- 1) Provide methodology and approximate one-hundred (100)-year water surface limits for all natural or man-made watercourses within the property wherever the tributary drainage area is greater than ten (10) acres.
- 2) Provide a description of the methodology and assumptions for analysis and design of anticipated stormwater conveyance facilities

7. Drainage Facility Design

- a. Provide a description of the types and locations of anticipated stormwater conveyance facilities to be utilized or constructed in the proposed project and a general discussion of how the preliminary calculations support the proposed design.
- b. Provide stormwater detention basin size, depth, and volume estimates and a discussion of anticipated locations.
- c. Identify drainage easement or tract locations internal to the project boundary

- d. Identify the anticipated parties responsible for maintenance of all proposed drainage facilities

8. References

Reference all criteria, master plan, design software and technical information used in support of the preliminary drainage design and concept.

9. Appendices

- a. Hydrologic Computations
- b. Hydraulic Computations

10. Required Maps

- a. General Location Map
- b. Hydrologic Soils Map, with site location and watershed boundaries clearly shown and labeled. This information can be shown as a layer on the pre-development drainage area map.
- c. Effective FEMA FIRM Map(s), with project boundary clearly shown and labeled. The floodplain can be shown as a layer on the pre-development drainage area map.
- d. Pre-development Drainage Area Map:
 - 1) Existing topographic contours
 - 2) Existing drainage flow paths entering, within, and leaving the site wherever the tributary drainage area is greater than one (1) acre.
 - 3) Major basin and sub-basin boundaries shown and labeled per summary table nomenclature
 - 4) Project boundary shown and labeled
 - 5) Pre-development runoff summary table

- 6) For offsite areas greater than () acres, a separate or inset map for offsite areas may be used in order to clearly see the onsite sub-basins at a reasonable map scale.
- e. Preliminary Post-development Drainage Area Map:
- 1) Twenty-four by thirty-six (24" X 36") inches in size at a scale of one inch equals twenty (1" = 20') feet to one inch equals one-hundred (1"= 100') feet
 - 2) Existing (dashed) and proposed (solid) contours at two (2) foot intervals to show details of the drainage flow patterns both on and off-site, extended a minimum of one-hundred (100') feet beyond the property boundary
 - 3) Existing and proposed property lines, right-of-way (ROW), and roadways
 - 4) Existing buildings within one-hundred (100') feet of the property and proposed building locations, if applicable
 - 5) Preliminary post-development drainage basin and sub-basin boundaries shown and labeled, per runoff summary table nomenclature
 - 6) Pre and post-development runoff summary table
 - 7) Hydrologic design point locations, matching summary table nomenclature
 - 8) Location of all existing and proposed utilities affected by, or affecting the drainage design
 - 9) All existing and proposed conceptual drainage facility locations, including culverts, headwalls, inlets, stormdrain systems, and detention basins
 - 10) Existing one-hundred (100)-year floodplain boundary for all FEMA designated floodplains and all natural watercourses wherever the tributary drainage area is greater than ten (10) acres.
 - 11) Any off-site feature or basin affected by the development

- 12) Include North arrow, scale, benchmark, and flow arrows
- 13) Legend to define map symbols and line types
- 14) Project name, address, engineering firm and seal, and date in title block

3.2.4 FINAL DRAINAGE REPORT

- A. A final drainage report shall be submitted with final construction plans for all project types as noted in [Section 3.2.2](#), or when required by other portions of the Prescott City Code (PCC), or at the request of the City Engineer.
- B. The purpose of the Final Drainage Report is to finalize the hydrologic analysis and hydraulic design computations needed to conform to the requirements of the PCC.
- C. Where final reports and construction plans deviate significantly from preliminary reports, the nature and impacts of those changes must be fully discussed in the final report, and will necessitate updates to the master planning documents for phased developments.
- D. The final drainage report shall be written as a stand-alone document. It shall contain, at a minimum, all the elements of a Preliminary Drainage Report with the following changes and additional elements:
 1. Drainage Facility Hydraulics and Design
 - a. Provide a detailed description of the proposed stormwater conveyance systems to be constructed, including methods used for hydraulic calculations and a detailed discussion of how those calculations support compliance of the proposed design with all City requirements.
 - b. Hydraulic Calculations for Proposed Drainage Conveyance Infrastructure:
 - 1) Street conveyance capacity
 - 2) Channel conveyance capacity
 - 3) Culvert capacity, conveyance, and overtopping
 - 4) Stormdrain capacity and conveyance

- 5) Inlet and catch basin locations and sizing calculations
- 6) Outlet and Energy Dissipation Device Design (e.g., riprap sizing)
- 7) Hydraulic gradeline calculations
- 8) Hydraulic loss calculations
- 9) Detailed water surface calculations for all watercourses with contributing watersheds with flows greater than fifty (50) cubic feet per second (cfs)

c. Proposed Stormwater Detention

- 1) Pond location(s), geometry, and freeboard
- 2) Detention volume and stage/storage calculations
- 3) Detention pond inflow/outflow summary table
- 4) Detention pond discharge and outfall design and considerations

d. Other Project Specific Details

- 1) Identify and describe other state/federal permits that will be required to construct the proposed project (e.g., CLOMR/LOMR, CWA 401, CWA 404, AZPDES)
- 2) Identify drainage easement or tract locations internal to the project boundary
- 3) Identify the parties responsible for maintenance of all proposed drainage facilities (in coordination with plat language)
- 4) Provide Geotechnical Engineering Data supporting pertinent channel side slopes or other germane issues.

2. Required Maps

- a. Post Development Drainage Area Map:

- 1) Final post development drainage basins and sub-basins
- 2) Existing and proposed stormwater capture and conveyance facility locations, sizes, and naming designations that correspond to summary tables contained in the report and/or appendix
- 3) Detention basins, with dimensions, volumes, and release rates noted
- 4) Proposed one-hundred (100)-year floodplain boundary for all FEMA designated floodplains and all natural watercourses wherever the tributary drainage area is greater than ten (10) acres
- 5) Proposed minimum finished floor elevations for lots subject to inundation by a watercourse or other points of drainage concentration.

3.2.5 DRAINAGE CONSTRUCTION PLANS

- A. All drainage construction plans shall be submitted on twenty-four by thirty-six (24" x 36") inch bond paper drawn to a readable scale.
- B. All drainage construction plans shall, at a minimum, contain the following information:
 1. Location Map
 2. Legend to define map symbols and line types
 3. City of Prescott (COP) General Notes
 4. COP Standard Construction Notes
 5. COP Standard Details
 6. COP survey datum and ties to COP monuments
 7. Special details, notes, and specifications
 8. Street names and widths

9. Parcel boundaries and Right-of-Way lines
10. Floodplain and floodway boundaries, with Base Flood Elevations
11. Existing and proposed topographic contours at two (2') foot intervals (interval may be increased in steep areas to improve readability)
12. Drainage boundaries and sub-boundaries (optional)
13. Existing and proposed utilities
14. All existing and proposed drainage facilities, with labels or designations that correlate directly with report and model identifiers
15. Stormwater flow directions
16. Stations and offsets for all utility lines and drainage facilities referenced to street stationing
17. Stormdrain pipe system stationing
18. Detention facility construction dimensions and details with plan and profile, cross-sections and tabulated volumes
19. Finished Floor Elevations of all existing and proposed structures
20. Finished pad elevations for all building sites
21. Roof and lot drainage directions
22. Paved and landscaped areas
23. Scour protection locations, (e.g., rock riprap areas dimensions with d50 size and depth)
24. Limits of grading and disturbance
25. Cut and fill areas identified with slopes labeled and quantities tabulated
26. Survey benchmarks and control points
27. Existing on-site features and improvements, and off-site features that may affect or be affected

28. Storm Water Pollution Prevention Plan (SWPPP) and/or erosion and sediment control (ESC) and permanent stabilization plan sheet(s)
29. Additional information as appropriate to the project

3.2.6 FEMA DESIGNATED FLOOD HAZARD AREAS

- A. If the proposed project contains an area that is designated on the currently effective Flood Insurance Rate Map (FIRM) as being in a Special Flood Hazard Area (SFHA) and the applicant would like to change, modify, or remove the SFHA, then detailed floodplain study is required. The applicant is responsible for preparing and submitting all required supporting documents and Letter of Map Change (LOMC) applications to Federal Emergency Management Agency (FEMA) in conformance with all local, State, and Federal requirements.
- B. The City of Prescott will review all floodplain studies for technical compliance and completeness per applicable Arizona Department of Water Resources (ADWR) State Standards and FEMA guidelines. All local and federal review fees associated with map revision requests are the responsibility of the applicant.

SECTION 3.3/ HYDROLOGY

3.3.1 PURPOSE

This section provides an overview of hydrologic methods and procedures acceptable for use in the City of Prescott.

3.3.2 GENERAL POLICIES

- A. Pre-development calculational parameters and results shall be consistent with the City of Prescott Area Drainage Master Plan, assumptions and results.
- B. Runoff calculations shall be performed, at a minimum, for the two (2)-year, ten (10)-year, twenty-five (25)-year and the one-hundred (100)-year return periods, in conjunction with all drainage reports for private developments and public works projects.

3.3.3 RAINFALL DATA

The rainfall values to be used for development in the City of Prescott are shown in [Table 3-1](#) or [3-2](#) depending on site location. These values were derived from the rainfall depth-duration frequency statistics for Arizona from NOAA Atlas 14, Precipitation-Frequency Atlas of the United States, Volume 1 Version 4: Semiarid Southwest. The

SCS Type-II 24-hour storm distribution with Antecedent Moisture Condition II is applicable to the Prescott area. Other temporal distributions or durations may be accepted.

3.3.4 TIME OF CONCENTRATION

The Time of Concentration (Tc) may be determined by a wide variety of commonly accepted calculation procedures. While no method is specifically preferred or disallowed, any method used shall be appropriate to the watershed being analyzed and compatible with the rainfall loss and hydrograph methods selected. Whichever method is used, it shall be fully documented in the drainage report. Different methods can be used for post-development conditions for on-site developed sub-basins. If the calculated Tc is less than five (5) minutes, the rainfall intensity for the five (5) minute duration shall be used (i.e., the minimum Tc is five (5) minutes).

**TABLE 3-1
RAINFALL DATA FOR CITY OF PRESCOTT
(FOR ALL AREAS DRAINING TO WILLOW AND WATSON LAKES)**

| Duration | Storm Frequency (years) | | | | | |
|----------|---|-------------|-------------|-------------|-------------|--------------|
| | Rainfall Depth (inches) / Intensity (inches/hour) | | | | | |
| | 2 | 5 | 10 | 25 | 50 | 100 |
| 5-min | 0.32 / 3.85 | 0.43 / 5.21 | 0.53 / 6.31 | 0.66 / 7.88 | 0.76 / 9.16 | 0.88 / 10.51 |
| 10-min | 0.49 / 2.94 | 0.66 / 3.97 | 0.80 / 4.81 | 1.00 / 5.99 | 1.16 / 6.97 | 1.33 / 8.00 |
| 15-min | 0.61 / 2.42 | 0.82 / 3.28 | 0.99 / 3.97 | 1.24 / 4.96 | 1.44 / 5.76 | 1.65 / 6.61 |
| 30-min | 0.82 / 1.63 | 1.11 / 2.21 | 1.34 / 2.67 | 1.67 / 3.34 | 1.94 / 3.88 | 2.23 / 4.45 |
| 1-hr | 1.01 / 1.01 | 1.36 / 1.36 | 1.65 / 1.65 | 2.06 / 2.06 | 2.40 / 2.40 | 2.75 / 2.75 |
| 2-hr | 1.14 / 0.57 | 1.52 / 0.76 | 1.84 / 0.92 | 2.28 / 1.14 | 2.64 / 1.32 | 3.04 / 1.52 |
| 3-hr | 1.23 / 0.41 | 1.59 / 0.53 | 1.89 / 0.63 | 2.31 / 0.77 | 2.67 / 0.89 | 3.09 / 1.03 |
| 6-hr | 1.44 / 0.24 | 1.80 / 0.30 | 2.10 / 0.35 | 2.58 / 0.43 | 2.94 / 0.49 | 3.30 / 0.55 |
| 12-hr | 1.80 / 0.15 | 2.16 / 0.18 | 2.52 / 0.21 | 3.00 / 0.25 | 3.36 / 0.28 | 3.72 / 0.31 |
| 24-hr | 2.16 / 0.09 | 2.88 / 0.12 | 3.12 / 0.13 | 3.84 / 0.16 | 4.32 / 0.18 | 4.80 / 0.20 |

**TABLE 3-2
RAINFALL DATA FOR CITY OF PRESCOTT
(FOR ALL AREAS DOWNSTREAM OF WILLOW AND WATSON LAKES)**

| Duration | Storm Frequency (years) | | | | | |
|----------|---|-------------|-------------|-------------|-------------|-------------|
| | Rainfall Depth (inches) / Intensity (inches/hour) | | | | | |
| | 2 | 5 | 10 | 25 | 50 | 100 |
| 5-min | 0.29 / 3.47 | 0.39 / 4.70 | 0.48 / 5.72 | 0.60 / 7.18 | 0.70 / 8.36 | .080 / 9.64 |
| 10-min | 0.44 / 2.64 | 0.60 / 3.58 | 0.73 / 4.36 | 0.91 / 5.47 | 1.06 / 6.37 | 1.22 / 7.34 |
| 15-min | 0.55 / 2.18 | 0.74 / 2.96 | 0.90 / 3.60 | 1.13 / 4.52 | 1.32 / 5.26 | 1.52 / 6.06 |
| 30-min | 0.74 / 1.47 | 1.00 / 1.99 | 1.21 / 2.42 | 1.52 / 3.04 | 1.77 / 3.54 | 2.04 / 4.08 |
| 1-hr | 0.91 / 0.91 | 1.23 / 1.23 | 1.50 / 1.50 | 1.88 / 1.88 | 2.19 / 2.19 | 2.53 / 2.53 |
| 2-hr | 1.04 / 0.52 | 1.36 / 0.68 | 1.64 / 0.82 | 2.04 / 1.02 | 2.38 / 1.19 | 2.74 / 1.37 |
| 3-hr | 1.11 / 0.37 | 1.41 / 0.47 | 1.68 / 0.56 | 2.07 / 0.69 | 2.40 / 0.80 | 2.79 / 0.93 |
| 6-hr | 1.32 / 0.22 | 1.62 / 0.27 | 1.92 / 0.32 | 2.28 / 0.38 | 2.64 / 0.44 | 3.00 / 0.50 |
| 12-hour | 1.56 / 0.13 | 1.92 / 0.16 | 2.28 / 0.19 | 3.00 / 0.25 | 2.88 / 0.24 | 3.24 / 0.27 |
| 24-hour | 1.92 / 0.08 | 2.40 / 0.10 | 2.64 / 0.11 | 3.12 / 0.13 | 3.60 / 0.15 | 3.84 / 0.16 |

3.3.5 DETAILED HYDROLOGIC METHODS

A. Hydrologic analysis for predevelopment conditions shall be performed using a detailed hydrologic analysis method. Tabular and graphical approximation methods are no longer acceptable except for preliminary work. Other methods may be approved on an individual project basis.

1. Rainfall Losses

a. Pre-development condition rainfall losses shall be calculated using a method such as the Soil Conservation Service (SCS), or Green & Ampt. These methods are implemented within commonly available modeling software such as the U.S. Army Corps of Engineers Hydrologic Modeling System (HEC-HMS), and are compatible with routing methods used for analyzing detention facilities. (Please note that HEC-1 is no longer an acceptable version). The results of any method used should be consistent with the City of Prescott, Area Drainage Master Plan. The SCS method is preferred.

b. Input Curve Numbers (CN) for the SCS method for predevelopment conditions shall also be consistent with the City's Master Plan, hydrology inputs. Soil mapping data is available through the Natural Resources Conservation Service (NRCS) website. Adjustment based on geotechnical testing of undisturbed

native site soils is acceptable. Further information on selection of CN and implementation of the SCS method can be found in the SCS, TR-55 manual.

2. Hydrographs

Synthetic unit hydrograph procedures such as the Clark, SCS, or Kinematic Wave are acceptable.

3.3.6 RATIONAL METHOD

The Rational Method is acceptable for design of on-site stormwater conveyance facilities with contributing drainage areas less than one-hundred and sixty (160) acres and a Tc less than sixty-(60) minutes. Use of the Rational Method, is fully presented in widely available engineering publications. The variation used in the Arizona Department of Transportation (ADOT) Hydrology Manual, is applicable to the Prescott area, as modified by criteria presented herein.

SECTION 3.4/ OPEN CHANNELS

3.4.1 PURPOSE

- A. This section applies to all private and public open channels. Requirements for roadside ditches are presented in [Section 3.6](#).
- B. An open channel is defined as a conveyance in which water flows with a free surface. Open channels will be classified as either natural or man-made. A channel is considered natural if the cross section geometry, bed slopes, bed and bank materials, vegetative growth, and overbank floodplain areas have not been altered by previous human activity. Any prior or planned alterations to natural channels may trigger any or all of the policies and requirements of this Article.

3.4.2 GENERAL POLICIES FOR NATURAL CHANNELS

- A. Developers are encouraged to leave natural channels, including the overbank areas within the one-hundred (100)-year floodplain, in their natural state.
- B. Natural channels with a tributary drainage area greater than ten (10) acres shall be analyzed using HEC-RAS to delineate the floodplain boundary.
- C. Drainage easements shall be created for all natural channels, and shall include the entire floodplain.

- D. Natural channels within development boundaries that will be subject to increased stormwater discharges will require permanent erosion protection measures, particularly in steeper terrain, or when supercritical flow regimes occur, or where increases in velocity exceed the erosive velocity of the natural bank soils.
- E. Assessment of the one-hundred (100)-year lateral erosion hazard shall be performed for all proposed development adjacent to alluvial natural channels (i.e., channels formed within unconsolidated material). Channels with bed and bank materials other than alluvial deposits shall be assessed on an individual project basis.
- F. Adequate building setbacks shall be established or bank stabilization measures shall be provided for all developments along natural channels. The minimum setback for all channels having tributary drainage areas greater than ten (10) acres shall be twenty (20') feet unless bank stabilization is provided for the one-hundred (100)-year discharge.

3.4.3 GENERAL POLICIES FOR MAN-MADE CHANNELS

- A. Safety of the general public shall be considered in selecting the location and design of man-made channels.
- B. All drainage channels shall include maintenance access and easements to allow periodic maintenance, cleaning and repair.
- C. Any channels, which will be maintained by the City of Prescott, must be within public drainage easements dedicated to, and accepted by, the City of Prescott. The City does not maintain channels or drainage facilities on private property, within private drainage easements.
- D. Concentrated drainage flows (for all storm frequencies) shall enter and depart from the property to be developed in substantially the same manner and location as under predevelopment conditions, including consideration of depths, widths, and velocities. Substantial deviation from pre-development flow patterns will require written acceptance and drainage easements from affected adjacent property owners and approval by the City Engineer.
- E. Drainage channels conveying substantial off-site flows may not be constructed within the right-of-way parallel to public roadways without prior written permission of the City Engineer. (NOTE: This does not apply to borrow ditches intended to intercept local surface runoff from entering the roadway)

- F. No valves, enclosures, transformers, standpipes, poles, mailboxes or other obstructions will be permitted in any open channel, with the possible exception of sanitary sewer manholes, where allowed by the City Engineer.
- G. Channel bank soil stabilization and scour protection is required for all channels. Unlined earthen channels are not permitted.
- H. The one-hundred (100)-year peak discharge shall be contained in the defined channel cross-section, with freeboard, per this Article.
- I. Minimum finished floor elevations for structures adjacent to channels shall be at least one (1') foot above the one-hundred (100)-year freeboard elevation.
- J. Construction activities, which will disturb or alter a natural channel may require permits from other government agencies, such as the U.S. Army Corps of Engineers or Arizona Department of Environmental Quality, to comply with requirements of the Clean Water Act and similar regulatory programs. Approval by the City of Prescott does not supersede or waive compliance with other applicable Federal and State laws.
- K. Existing man-made channels within a development or redevelopment area must be modified if they do not meet current design criteria.

3.4.4 CHANNEL DESIGN

- A. Well established and widely accepted design procedures for opens channels can be found in the current editions of various Federal, State, and County publications. The design engineer shall thoroughly reference and document the procedures and assumptions used and shall adequately demonstrate that the procedure and resultant design is appropriate to the project.
- B. The channel cross section geometry, alignment, and bed slope should result in a stable channel with subcritical flow. Grade control structures in combination with energy dissipation measures may be required to limit slopes and velocities.
- C. Channel side slopes shall be no steeper than three to one (3H:1V) for vegetated channels and two to one (2H:1V) for lined channels. Slope stability analysis may be required in some instances.
- D. The minimum allowable channel slope shall be one-half (0.5%) percent.
- E. Channel stabilization measures (including consideration of bank erosion protection, scour protection, and energy dissipation measures) shall be designed for the expected depths, velocities, and shear forces for the one-hundred (100)-

year event. The most common channel stabilization methods include turf reinforcement mats, rock riprap, grouted riprap, gabion baskets and mattresses, revetments, concrete, and other biotechnical or synthetic measures. These types of stabilization can be used alone or in combination. Re-vegetation may be acceptable where channel side slopes are three horizontal to one vertical (3H:1V) or less steep and the underlying soil type is stable for the maximum velocities and forces anticipated.

- F. Levee style embankments are not acceptable for creating channels, however, they may be used to satisfy the freeboard requirement.
- G. Retaining walls used in conjunction with or adjacent to channels and subject to inundation or flow shall be designed for such conditions.

3.4.5 FREEBOARD

- A. Freeboard is the minimum additional channel depth required between the calculated one-hundred (100)-year water surface elevation and the top of the lowest channel bank.
- B. Freeboard shall be calculated by a method that includes consideration of depth, velocity and flow regime. The minimum freeboard is one (1') foot.
- C. Additional freeboard may be required at junctions, channel curves, changes in cross section or slope, culvert outlets, where backwater effects may occur, and at locations where supercritical flow or hydraulic jumps occur.
- D. Channel linings of protected open channels shall extend to the elevation necessary to include the freeboard requirement.
- E. Freeboard is not required for minor drainage conveyances such as roadside ditches, yard drainage swales on residential lots, and internal surface drainage for commercial or multi-family developments, where finished floors would not be impacted by overflows.

SECTION 3.5/ CULVERTS AND BRIDGES

3.5.1 PURPOSE

- A. The purpose of this section is to present policies and criteria for the design and construction of roadway culverts and bridges. Culverts consist of a relatively short length of pipe or multiple conduits used to convey stormwater through a roadway embankment, and are aligned with open channels. Nationally accepted

culvert design procedures are presented in publications such as the (Federal Highway Administration (FHWA) Hydraulic Design Series No. 5 (HDS-5), Hydraulic Design of Highway Culverts.

- B. Bridges are structures that provide a clear span over a watercourse where the full one-hundred (100)-year discharge is conveyed beneath the lowest structural member (low chord) under the deck. Bridges may be required where other City policies and criteria cannot be met with culverts.

3.5.2 GENERAL POLICIES

- A. All roadway culverts for local streets shall be designed to convey at least the twenty-five (25)-year frequency flows without roadway overtopping. Culverts under collector and arterial streets shall convey the fifty (50)-year flow without overtopping.
- B. The allowable depth of weir flow over the roadway for the 100-year storm shall be limited to one (1') foot or less for Local and Collector Streets and one-half (0.5') foot for Arterial Streets. This approach is acceptable for crossings where the roadway sags at that point and the overtopping flows can be controlled and redirected into the channel immediately downstream of the crossing. Where overtopping flows cannot be controlled in such a manner, the full 100-year discharge must be conveyed by the culvert pipes, without roadway overtopping.
- C. All culvert designs shall provide a controlled path for overtopping flows. The overflow path shall be designed to withstand erosion and scour for the calculated overflow depths and velocities for the one-hundred (100)-year event.
- D. Culvert sizes shall be increased by a clogging factor to account for a fifty (50%) percent loss of conveyance capacity during the design event. Alternatively, trash racks may be used with the approval of the City Engineer when the potential for large natural debris in the watershed is relatively high.
- E. Culvert pipes shall not be laid on a curved alignment, nor have horizontal or vertical bends except at a manhole or junction box.
- F. The minimum slope for any culvert shall be one-half (0.5%) percent, but may need to be steeper to prevent siltation for minor storm events such as the two (2)-year event.
- G. Culverts shall not be located underneath structures (such as habitable buildings) except where approved under the Article 9 process, and only when a one-hundred (100)-year overflow path is provided around the structure with one (1') foot of freeboard to the finished floor elevation of the structure.

- H. At-grade (ford) crossings for any street classification in new developments are not permitted. They are permissible for private non-commercial driveways.
- I. In new subdivisions where roadside ditches or behind-the-curb drainage is proposed, the design engineer shall determine driveway culvert sizes and shall indicate all such sizes on the subdivision infrastructure plans.
- J. All driveway culverts that are part of an overall residential drainage system shall be designed to convey stormwater runoff for the twenty-five (25)-year event.
- K. The minimum diameter for all roadway-crossing culverts shall be eighteen (18") inches. The minimum driveway culvert size is eighteen (18") inches.
- L. Inundation areas on the upstream side of all culvert crossings for the one-hundred (100)-year event shall be contained within drainage easements. Allowable inundation areas and water surface elevations may also be limited by the potential impacts to existing improvements. Pondered water surface elevations shall be one (1') foot below the finished floor of existing structures.
- M. The minimum easement width for culvert pipes forty-eight (48") inches in diameter or less shall be twenty (20') feet. For larger diameter or multiple pipe installations, the easement width shall extend at least eight (8') feet beyond the outside edges of the pipe(s) on each side.
- N. Bridge designs shall be evaluated on an individual project basis using nationally accepted design standards consistent with the functional classification of the roadway, and in accordance with City of Prescott, General Engineering Standards (GES).

3.5.3 INLET AND OUTLET TREATMENT

- A. All culvert pipes shall have an appropriate end treatment.
 - 1. Commercial prefabricated end sections are permitted on culverts thirty-six (36") inches in diameter or less. Riprap protection shall extend around and over the top of the end section a minimum of two (2') feet.
 - 2. Concrete headwalls are required on all culverts greater than thirty-six (36") inches in diameter within drainage easements and public right-of-way (ROW).
- B. All culvert outlets shall include erosion and scour protection measures that protect the embankment as well as the receiving channel.

1. If the discharge velocity at the end of the pipe is consistent with the normal depth velocity in the receiving channel, then a short riprap apron is adequate to prevent scour at the pipe end that could undermine the headwall or pipe end treatment.
2. For discharge velocities up to ten (10) feet per second (fps), a rock riprap apron can be used to provide scour protection. The length and width of the apron shall be designed to transition the discharge back to the normal depth and velocity of flow in the channel.
3. For discharge velocities in excess of ten (10) fps, grouted or wire tied rock in combination with a riprap apron may be necessary to withstand the calculated forces.
4. For discharge velocities in excess of fifteen (15) fps, energy dissipation structures shall be required in conjunction with additional apron downstream of the energy dissipater to transition the flows back into the receiving channel.
5. The maximum velocity in any culvert shall be twenty (20) fps.
6. Roadways designed for overtopping will require additional slope protection on the downstream side of the roadway, since the roadway acts as a spillway.

3.5.4 CULVERT MATERIAL AND INSTALLATION

- A. The minimum service life for culverts shall be fifty (50) years for privately maintained facilities.
- B. The following materials may be used for privately maintained drainage facilities:
 1. Rubber Gasket Reinforced Concrete Pipe (RGRCP)
 2. Corrugated Metal Pipe (CMP)
 3. High Density Polyethylene (HDPE)
- C. Pipe Materials for publicly maintained drainage facilities:
 1. Preferred Material: Rubber Gasket Reinforced Concrete Pipe (RGRCP)

2. The following materials will be considered on an individual project basis with approval of the City Engineer:

Alternative Material: High Density Polyethylene Pipe (HDPE)

- D. The material selection process for privately maintained culverts shall include consideration of factors such as hydraulic efficiency, depth of bury, bedding conditions, soil types, sediment loads, abrasion, and corrosion resistance.
- E. Watertight joints are required for all installations where the calculated hydraulic gradeline is above the top of pipe at any point along the pipe.

SECTION 3.6/ STREET DRAINAGE

3.6.1 PURPOSE

When designing streets, the amount of stormwater allowed to flow on the surface of the street is governed by consideration of the need for the road to continue functioning under storm conditions and the safety of all roadway users, including vehicles and pedestrians. When the flow in the street exceeds allowable depth, spread, or velocity criteria, the stormwater shall be intercepted, removed from the roadway into an underground pipe system or roadside ditch, and conveyed to an appropriate, open channel. This section presents the criteria for surface drainage on public and private streets. Publications such as the Federal Highway Administration (FHWA) Urban Drainage Design Manual (HEC-22) present detailed methods for performing calculations related to street drainage.

3.6.2 GENERAL POLICIES

- A. Street drainage and roadways shall be designed to maintain the natural overall drainage patterns existing prior to development, whenever possible. Significant cross-basin diversions that could increase flow in downstream receiving watercourses where capacity and erosion would be adversely impacted are not allowed.
- B. Street drainage design shall consider both the twenty-five (25)-year and one-hundred (100)- year storms.
- C. When physical conditions do not allow for a sub-surface stormdrain system on new curb and gutter street sections, a paralleling surface drainage system may be permitted through the GES, Article 9 process.
- D. New inverted crown public streets, are prohibited.

- E. Where concentrated flows from adjacent properties enter the road right-of-way, drainage facilities shall be placed to intercept the runoff at a point outside the street section to prevent those flows from going over sidewalks or curbs, especially in areas where sediment loads are carried.
- F. It is acceptable for adjacent properties to sheet drain onto the streets. However, slopes and grading outside the roadway template shall be designed to intercept and reduce the potential for sediments to be deposited on sidewalks and in the streets.
- G. In all cases, once stormwater runoff enters the street, it shall be confined to the public right-of-way (on the surface or underground) until it can be discharged in a controlled manner to the appropriate receiving watercourse or other approved conveyance system.
- H. Flow on a local street that has significant depth or velocity approaching an intersection with a collector or arterial shall be captured prior to the intersection. It is unacceptable for the local street flow to "shoot out" onto the major roadway. Minor and bypass flows that can "turn the corner" may be permitted.

3.6.3 STREET AND GUTTER

- A. Design Frequency and Allowable Spread
 - 1. For Local curbed street sections, runoff from the twenty-five (25)-year design storm must be contained between the curbs of the street. The one-hundred (100)-year flow must be contained within the right-of-way with a maximum depth at the crown of one (1') foot.
 - 2. For Collector and Arterial curbed street sections, at least one (1) twelve (12') foot travel lane in each direction must remain free from flooding for the twenty-five (25)-year design storm. The one-hundred (100)-year flow must be contained within the right-of-way, with a maximum depth at the crown of six (6") inches.
 - 3. If either of the above two criteria are exceeded, the excess flow must be captured and removed from the street surface. The subsurface system capacity may need to be increased beyond the Twenty-five (25)-year discharge to handle the balance of the one-hundred (100)-year event, which cannot be contained within the right-of-way, such as in a hillside situation.
- B. Gutter Flow

1. The term gutter flow may include the portion of flow that spreads out into the travel lane.
2. Valley gutters perpendicular to the flow of traffic on arterial or collector streets are not permitted. Valley gutters may be used at intersections parallel to the major road. In cases where a valley gutter is not permitted or is inadequate, the water must be removed from the street by stormdrains or other approved methods.

C. Longitudinal and Transverse Slopes

1. The geometric and cross section requirements for all streets are specified in the General Engineering Standard (GES), Article 6, Transportation and Traffic.
2. In locations where super-elevated roadway sections are used, all gutter flow approaching the transition from the normal crown section shall be removed from the street to minimize the amount that sheets across the street.
3. Where longitudinal slopes are relatively steep and surface flow velocities are high, allowable gutter capacities shall be reduced to keep depths and velocities at safe levels for pedestrians.

D. Curb and Gutter Terminations

Adequate erosion protection and conveyance measures are required where the curb and gutter section terminates at transitions to uncurbed streets to prevent undermining of the pavement edge and prevent head cutting of roadway fill slopes. Such protection will consist of a riprap or concrete spillway that extends to the adjacent roadside ditch or other conveyance.

E. Rural Roads and Roadside Ditches

1. Roadside ditches or channels for rural, uncurbed street sections shall be designed for the twenty-five (25)-year design storm, with provision for the one-hundred(100)-year storm to be contained within the right-of-way.
2. The underlying soil conditions, flow depths and velocities, and maintenance shall be considered in the roadside channel design. Unlined ditches may be acceptable where sedimentation is expected, but are unacceptable where long term erosion is more likely.

3.6.4 CATCH BASINS

A. Locations

1. Catch basins shall be placed where depth, spread, and velocity criteria dictate. Bypass flow is permissible on continuous grades.
2. In addition, basins shall also be located in the following locations, with one-hundred (100%) percent capture of the design storm:
 - a. Where the road warps or super elevates, to prevent gutter flow from sheeting across the lanes
 - b. Immediately upgrade of Arterial intersections
 - c. Immediately upgrade of bridges
 - d. Behind curbs, shoulders, or sidewalks to drain low areas to prevent ponding

B. Types

3. Curb opening, grate, and combination catch basin types are all acceptable for use, depending on the location, debris potential, and flow rates.
4. Combination inlets (on-grade) are preferred on public streets due to the reduced clogging tendency when the curb opening is placed upstream of the grate. Calculation of clogging factors for combination inlets can be waived in areas where the likelihood of significant debris is low.
5. Curved vane grates are preferred for on-grade locations, but are not allowed for sump locations.
6. All grates within the street shall be bicycle friendly.
7. To promote flow capture at catch basins on grade, a two (2") inch gutter depression at the inlet is recommended.
8. Scuppers are acceptable where a receiving ditch or channel is adjacent, and require a concrete or riprap chute to prevent erosion.

C. Clogging Factors

1. Clogging factors shall be applied as follows:

- a. Grate only (on-grade): 2.0
 - b. Grate only (sump): not allowed
 - c. Curb opening only (on-grade): 1.25
 - d. Curb opening only (sump): 1.5
 - e. Combination inlet (on-grade): 1.0
 - f. Combination inlet (sump): 1.25
2. Computationally, clogging factors increase the required length or area, or the reciprocal value can be used to calculate the reduced capture capacity.

SECTION 3.7/ STORMDRAINS

3.7.1 PURPOSE

This section presents the criteria for the design of underground stormdrain pipe systems that convey street flows once they are intercepted by catch basins. They differ from culverts in that the stormdrain system hydraulics is controlled by friction and junction losses whereas culvert hydraulics is controlled predominantly by inlet or outlet conditions.

3.7.2 GENERAL POLICIES

- A. Stormdrains shall be designed in conjunction with the street drainage criteria. Accordingly, the minimum design frequency for all stormdrain systems shall be the twenty-five (25)-year storm. Stormdrain capacity may need to be increased to control the one-hundred (100)-year storm per the street criteria.
- B. Stormdrain system outfall points, shall be constructed with end treatments, energy dissipation, aprons, scour protection, and other erosion control measures in a manner similar to the requirements for culvert outlets.
- C. If the outlet is perpendicular to the direction of flow in the receiving channel, erosion of the opposite channel bank must be considered and a channel bank lining of riprap or other appropriate material, will be required.

- D. The horizontal placement of stormdrains and manholes within roadways shall be in general accordance with the standard roadway cross section templates in the GES, Article 6, Transportation and Traffic.
- E. Where stormdrain systems are placed within easements, the minimum easement width for pipes forty-eight (48") inches in diameter or less shall be twenty (20') feet. For larger diameter or multiple pipe installations, the easement width shall extend at least eight (8') feet beyond the outside edges of the pipe(s) on each side.
- F. The minimum service life for public storm drains shall be fifty (50)-years. Pipe materials for publicly maintained drainage facilities:
 - 1. Preferred Material: Rubber Gasket Reinforced Concrete Pipe (RGRCP)
 - 2. The following materials will be considered for public stormdrains on an individual project basis, with approval of the City Engineer:

Alternative Material: High Density Polyethylene Pipe (HDPE)
- G. The minimum service life for private storm drains shall be fifty (50)-years. The following materials may be used for private stormdrains:
 - 1. Rubber Gasket Reinforced Concrete Pipe (RGRCP)
 - 2. Corrugated Steel Pipe (CSP) or Corrugated Metal Pipe (CMP)
 - 3. High Density Polyethylene (HDPE)

3.7.3 DESIGN CRITERIA

- A. The minimum allowable slope for any stormdrain shall be one-half (0.5%) percent.
- B. The minimum velocity for pipes in the system when flowing half full shall be three (3) feet per second (fps).
- C. The maximum velocity in any portion of a stormdrain system shall not exceed twenty (20) fps for the design flow unless the forces of such velocity are factored into the design.
- D. In most cases, stormdrain pipes shall be on straight runs with uniform slope between manholes or junctions. Horizontally curved stormdrains may be permitted, where it would be beneficial to follow a curved street alignment. The

radius of curvature shall not be less than the pipe manufacturer's recommendation. Pulled joints are not an acceptable method.

- E. Minor vertical bends without manholes are discouraged, but may be used where necessary to resolve conflicts with other utilities.
- F. Horizontal bends without manholes are acceptable for deflection angles less than twenty-five (25°) degrees.
- G. The minimum pipe diameter for all laterals shall be eighteen (18") inches, and the minimum diameter for all main lines shall be twenty-four (24") inches.
- H. Stormdrain pipe sizes shall increase in the downstream direction.
- I. Watertight joints are required for all installations where the calculated hydraulic gradeline is above the top of pipe at any point along the pipe.

3.7.4 MANHOLES AND JUNCTION STRUCTURES

- A. The primary function of a stormdrain manhole is to provide access to the stormdrain system for inspection and maintenance. Junction structures are used where multiple flows converge and are outfitted with a manhole. The terms manhole and junction are used somewhat interchangeably.
- B. At a minimum, manholes are required for the following locations:
 - 1. At junctions where two (2) or more stormdrains converge. (Lateral connections from adjacent catch basins do not require a manhole since maintenance access is available through the inlet
 - 2. At intermediate points along stormdrains in accordance with [Table 3-3](#)
 - 3. At vertical deflections greater than ten (10°) degrees at a single joint
 - 4. At horizontal alignment changes
 - 5. Changes in pipe size
 - 6. Manholes may also be required by the City Engineer at other locations to facilitate maintenance

**TABLE 3-3
MANHOLD SPACING CRITERIA**

| PIPE DIAMETER (INCHES) | MAXIMUM DISTANCE (FEET) |
|------------------------|-------------------------|
|------------------------|-------------------------|

| | |
|---------------------------------|-----|
| Less than or equal to 30 inches | 300 |
| Between 30 and 42 inches | 400 |
| 42 inches and up | 500 |

- C. Manholes at vertical deflections shall be at or as close as practical to the point of deflection, with allowance for manufactured bends. If the manhole is not at the point of deflection, it shall be located immediately upstream of the deflection.
- D. Manhole covers should be transversely located within the right-of-way per COP Standard Details. Every effort shall be made to avoid locating manhole covers within the vehicle wheel path.
- E. A pressure manhole shaft and pressure frame and cover is required whenever the hydraulic grade line elevation for the design storm at the manhole is less than twelve (12") inches below the manhole cover.
- F. Where the manhole depth is greater than twelve (12') feet the shaft diameter shall be five (5') feet and must be designed to withstand soil pressures. If a manhole will extend below the water table, it must also be designed to withstand hydrostatic pressure and/or seepage.
- G. Manhole shafts shall be five (5') feet in diameter for stormdrain pipes thirty-six (36") inches in diameter or greater.
- H. A positive grade to the outflow pipe within the structure is required. A minimum drop of one-tenth (0.10') foot is required through all stormdrain manholes.

3.7.5 HYDRAULICS

- A. The Hydraulic Grade Line (HGL) and Energy Grade Line (EGL) shall be calculated for all systems and laterals.
- B. In addition to friction losses, HGL calculations shall include losses at all junctions, manholes, bends, transitions, inlets, and outlets.
- C. Pressure flow is acceptable, but the HGL shall not extend above the ground surface at any point, and shall be at least one (1') foot below the opening or grate of a catch basin.
- D. When connecting into an existing stormdrain system, the existing system shall be analyzed to determine available capacity. Improvements to the existing system may be required.

SECTION 3.8/ STORMWATER DETENTION

3.8.1 PURPOSE

This section presents the criteria for the design of stormwater detention facilities in the City of Prescott.

3.8.2 GENERAL POLICIES

- A. Stormwater detention is required for all new subdivisions, commercial and industrial developments, re-development of previously un-detained sites and other developments that increase impervious area or identifiably impacts adjoining properties.
- B. Detention may be waived by the City Engineer for sites less than one (1) acre where there is no adverse impact to downstream property or undersized facilities.
- C. Detention may also be waived by the City Engineer where permissible under State or County criteria, and where it can be clearly demonstrated that the downstream peak discharge will not be increased.
- D. Detention is not required for a single-family residential structure on a residential lot.
- E. The post-development discharge from a development cannot exceed the total pre-development peak at each discharge point for the two (2)-year, ten (10)-year, and one-hundred (100)-year storms.
- F. The same hydrologic methodology shall be used for both pre-development and post-development analyses.
- G. Off-site flows shall not be routed through on-site detention.
- H. The use of pavement parking lot storage as the primary detention facility is not permitted unless other sites or detention alternatives are not available. Some inundation of parking lots may be permissible for the major storm event.
- I. Rooftop storage is not permitted for meeting City of Prescott detention requirements.

- J. Stormwater retention is not permitted unless used in conjunction with water quality elements and subsurface infiltration enhancements, which will prevent ponding on the surface for more than seventy-two (72) hours after a storm event.
- K. Individual on-lot storage systems within single-family residential developments are not permitted for meeting City of Prescott detention requirements.
- L. The minimum finished floor elevation of any structure adjacent to a detention facility shall be a minimum of one (1') foot above the emergency overflow water surface level for the one-hundred (100)-year storm.
- M. Developments, which are phased, shall prepare a master stormwater detention plan for the entire development. Either, the master stormwater detention plan must be implemented with the first phase in full, if possible, or detention must stand alone for each phase. Changes to any phase may require an update of the master plan to insure that all phases remain coordinated.
- N. Discharges from detention facilities shall be designed to enter established downstream drainage systems, (e.g., drainage channels, natural watercourses, public streets, or stormdrain systems) whenever possible.
- O. The City of Prescott, may accept regional detention basins for operation and maintenance on an individual project basis. The City will not accept local on-site detention basins for operation, maintenance, or liability.
- P. Maintenance of detention facilities shall be the responsibility of the property owner or the homeowner's association. Final Plats, Covenants, Conditions, Restrictions (CC&R's), and/or Development Plans shall include a statement as to the responsibility for the operation and maintenance of all detention facilities.
- Q. A maintenance plan shall be prepared in conjunction with the detention/retention facility design that includes both scheduled and unscheduled maintenance activities. Scheduled maintenance includes items such as mowing, vegetation control, and sediment and trash removal. Unscheduled maintenance includes repairs to damage discovered during regular inspections or after a flooding event.
- R. An Operation and Maintenance Plan per [Section 3.10.4](#) of this Article is required, if any portion of the detention system is used to meet post construction stormwater quality requirements.
- S. No part of a private detention basin shall be constructed in a public right-of-way or public utility easement.

- T. The Engineer of Record must certify detention basin dimensions and volumes upon substantial completion of construction as part of the as-built certification process.

3.8.3 DESIGN CRITERIA

- A. Hydrologic analysis for detention design shall be performed using a detailed method that is capable of routing a hydrograph based on stage-storage-discharge characteristics and multi-frequency outlet structures. Tabular and graphical approximation methods such as TR-55 or manipulations of the Rational Method are no longer acceptable except for preliminary work.
- B. The following minimum slope and depth criteria are required for multi-use basins and basins that have unrestricted access:
 - 1. A maximum of two horizontal to one vertical (2H:1V) for protected side slopes and three horizontal to one vertical (3H:1V) for unprotected side slopes where depths of ponding are less than three (3') feet.
 - 2. A maximum of four horizontal to one vertical (4H:1V) for side slopes where depths of ponding exceed three (3') feet.
- C. Security barriers, with maintenance access gates, are required along the top of all basin side slopes steeper than three to one (3H:1V) and where water depths exceed three (3') feet.
- D. A minimum freeboard of one (1') foot above the one-hundred (100)-year high water elevation is required for all detention facilities. Freeboard may include adjacent parking lot areas.
- E. Vegetated embankments shall be less than twenty (20') feet in height and shall have side slopes no steeper than three horizontal to one vertical (3H:1V). Embankments protected with riprap or other approved erosion control measure shall be no steeper than two horizontal to one vertical (2H:1V).
- F. A geotechnical engineering study and slope stability analysis is required for embankments exceeding ten (10') feet in height or for embankment slopes exceeding those given above.
- G. Top width of the embankment shall be a minimum of one-half (1/2) the height of the embankment.
- H. All detention facilities shall include an emergency overflow spillway or bypass sufficient to convey the post development one-hundred (100)-year peak

discharge without damage or overtopping of the impoundment berm. The emergency spillway shall not be placed on the impoundment berm itself. All detention facilities shall incorporate provisions for emergency overflow.

- I. Riprap aprons or other energy dissipating measures should be used at all inflow points (side slope and basin floor) to reduce velocities and encourage sedimentation.
- J. Low flow channels are required on the bottom of basins, which serve as multi-use areas and are recommended on all basins. Low flow channels should be designed with a minimum longitudinal slope of one-half (0.5%) percent. Concrete lined low flow channels can be designed with a minimum longitudinal slope of two-tenths (0.2%) percent.
- K. Outlet structure designs shall take clogging potential into account. Outlet structures incorporating orifice plates shall include a trash rack to minimize clogging.
- L. Outlet pipes projecting from a basin side slope or embankment are not permitted. Outlet points shall be constructed with end treatments, energy dissipation, aprons, scour protection, and other erosion control measures in a manner similar to the requirements for culvert outlets.
- M. Where basins outfall near to adjoining private properties, additional measures may be necessary to reduce velocities and allow flows to return to natural conditions, to as great an extent as possible, prior to exiting onto the downstream property.
- N. Drainage crossings or culverts shall be provided whenever runoff entering or exiting a basin crosses pedestrian paths or sidewalks.
- O. The minimum outlet culvert size for detention facilities is twelve (12") inches. Orifice plates are permitted provided the orifice plate is permanent, tamper-proof, and connects to a twelve (12") inch minimum diameter outlet pipe.
- P. Maintenance ramps and access shall be provided to facilitate maintenance activities such as debris and sediment removal.
- Q. Retaining walls used in conjunction with or adjacent to detention/retention facilities and subject to inundation or flow shall be designed for such conditions.

SECTION 3.9/ EROSION AND SEDIMENT CONTROL

3.9.1 PURPOSE

- A. The purpose of this section is to control or eliminate soil erosion and sedimentation within the City of Prescott. The City has established standards and specifications for conservation practices and planning activities which minimize soil erosion and sedimentation in order to accomplish all of the following goals:
 - 1. Protect public health, welfare, and the environment
 - 2. Enable the City to comply with stormwater pollution prevention provisions of the Arizona Pollutant Discharge Elimination System (AZPDES) Small MS4 General Permit
 - 3. Properly manage and reduce erosion and the discharge of pollutants from construction sites to the City's stormwater system, and in turn, to surface waters and groundwater

- B. This section presents general guidelines and requirements for erosion and sediment control or Best Management Practices (BMPs) associated with construction activities as defined in 40 CFR 122.26(b)(14)(x). These requirements are outlined in the City of Prescott Stormwater Runoff Regulations, Prescott City Code (PCC), Title XVI, in compliance with the AZPDES General Permit for Stormwater Discharges from Small Municipal Separate Storm Sewer Systems (MS4s) to Waters of the United States.

3.9.2 APPLICABILITY

Erosion and Sediment Control (ESC) Site Plans and/or Stormwater Pollution Prevention Plans (SWPPPs) are required for construction activities that result in the land disturbance of greater than or equal to one (1) acre, including projects that are less than one (1) acre but are part of a larger common plan or development of sale.

3.9.3 GENERAL GUIDELINES

- A. If the project disturbs one (1) acre or greater or is less than one (1) acre but is part of a common plan of development, the owner or operator must apply for coverage under the AZPDES for Stormwater Discharges Associated with Construction Activities. A copy of the Notice of Intent (NOI) to be bound by the State's general construction permit must be filed with the City during permitting.

- B. The permittee shall implement BMPs as described in the ESC Plan or SWPPP and shall periodically conduct site inspections to ensure that BMPs are operating correctly and have not been damaged and/or altered.
- C. Sediment control shall be used whenever possible or necessary, to prevent off-site damage or sediment deposition to public streets, ROWs, and stormwater facilities.
- D. All disturbed soils shall be permanently stabilized.
- E. All temporary and permanent erosion and sediment control practices shall be maintained and repaired as needed to assure continued performance of their intended function.

3.9.4 DESIGN CRITERIA

- A. SWPPPs, BMPs, ESC design methods and facilities shall be in accordance with the Arizona Department of Transportation Erosion and Pollution Control Manual, December 2012, as amended. The SWPPP shall be a separate plan sheet or shall be incorporated into the grading & drainage plans. The SWPPP does not supersede the need for AZPDES or NPDES permits.
- B. For specific design criteria and guidelines, see the Prescott City Code (PCC), Title XVI, Stormwater Runoff Regulations.

SECTION 3.10/ POST CONSTRUCTION STORMWATER TREATMENT

3.10.1 PURPOSE

This section will present general guidelines and requirements for post construction stormwater management or Best Management Practices (BMPs) for new development and redevelopment projects to prevent or minimize water quality impacts. These requirements are outlined in the Prescott City Code (PCC), Title XVI, Stormwater Runoff Regulations in compliance with the Arizona Pollutant Discharge Elimination System (AZPDES) General Permit for Stormwater Discharges from Small Municipal Separate Storm Sewer Systems (MS4s) to Waters of the United States.

3.10.2 APPLICABILITY

New development or redevelopment projects that disturb greater than or equal to one (1) acre, including projects that are less than one (1) acre but are part of a larger

common plan or development of sale, must treat the “first flush” (defined as the first half-inch) runoff from a site.

3.10.3 GUIDELINES

A. The following categories of structural treatment practices for post-construction stormwater runoff are approved for use within the City of Prescott.

1. Inlet Filtration Systems

Inlet filtration systems are accepted for use on project sites with underground storm infrastructure. Inlet filters must be sized and located appropriately to treat the required WQV for the site. Filter systems shall provide removal of sediments, debris and other nonpoint source pollutants from the site. For sites with a high degree of impervious surface (i.e., parking lots), filters shall provide for the removal of petrochemicals and hydrocarbons.

2. Underground Separators

Underground separators, such as oil-water separators or sand-oil separators, are accepted for use on project sites with underground storm infrastructure. Underground separators may be required for land uses or activities with higher potential pollutant loadings, known as “hotspots,” or for sites that discharge directly to a waterway.

3. Surface Treatment/Biofiltration

For developments with surface flows, stormwater may be collected and treated in engineered vegetated basins on site. Traditional detention basin designs may be modified to allow for a longer holding time in the basin and include the installation of a sand/soil filter bed and underlying gravel storage zone. For design specifications, reference the City of Flagstaff Low Impact Development Guidance Manual for Site Design and Implementation, [Section 3.8](#) Extended Detention Basin.

3.10.4 OPERATION AND MAINTENANCE

A. The owner of permanent stormwater management facilities shall be responsible for the proper operation and maintenance of those facilities during and after construction. All permanent on-site BMPs shall be operational prior to the use by any development or phase of development dependent on those BMPs. An Operation and Maintenance Plan shall be prepared for review and approval by

the City Engineer and shall be executed and signed by the Department and the owner.

- B. Development project permits shall be conditioned to require the property owner to execute a Standard Operation and Maintenance Agreement for Post Construction Stormwater Quality Best Management Practices (BMPs) Facilities prior to the issuance of any construction permits.
- C. The City of Prescott does not endorse specific commercial brands or products, but may provide examples of brands or products currently used in the City to meet Code requirements.

ARTICLE 4/ WATER

SECTION 4.1/ INTRODUCTION

4.1.1 PURPOSE

- A. The goal of the Water System Planning and Design Article is to provide technical management, comprehensive planning, and sound engineering to expand and maintain a reliable and safe water supply, distribution, and storage system. Engineering standards, guidelines, and geometric requirements for the design and construction of public water improvements within the City of Prescott will result in high-quality customer service and fire protection capabilities.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

4.1.2 APPLICABILITY

- A. The standards and guidelines of this Article shall apply to the construction, modification and extension of all water distribution mains, pump stations and other appurtenances as well as any modification of the public water system to be owned, operated and maintained, by the City of Prescott (COP). Privately funded water mains that are proposed to be connected to the City of Prescott's system are reviewed for compliance with City of Prescott standards. Private water mains shall be extended to new users in a logical and cost effective manner without adversely affecting existing customers and system performance. This Article is intended for use in plan design, preparation, and review process.
- B. Developers are required to install all improvements necessary, including off-site improvements to provide water service for domestic and fire fighting purposes to their development in accordance with Prescott City Code (PCC), this Article, and the most current water and wastewater model study.

4.1.3 DESIGN STANDARDS AND GUIDELINES

- A. New public and private water supply distribution and storage facilities shall be designed in accordance with Article 1.
- B. A civil engineer registered in the State of Arizona shall analyze, design, and seal all proposed water system project plans and specifications that are determined by the City Engineer to have an impact on the water distribution system. The effects of peak and fire flow demands shall be examined to ensure proper sizing

and layout of proposed water system facilities, including impacts to existing off-site facilities where deemed appropriate by the City Engineer.

- C. The operation, maintenance, and repair of private water systems are the responsibility of the Water Service Provider (Provider). The Provider shall pay the cost for the operation, maintenance, repair, and replacement of private systems. These private systems are connected to main lines operated by the City of Prescott, as identified on the final approved plans. The City of Prescott shall have no obligation or requirement to assist or provide labor or equipment to perform or supplement any responsibilities pertaining to said private water systems, all of which are those of the Provider. The Provider shall provide on a yearly basis the firm, contact name, business phone number, and emergency phone number for the private system operator, who shall be certified by the Arizona Department of Environmental Quality at the required level.
- D. All components and related appurtenances for construction and repair of water infrastructure shall be domestically manufactured, certified and submitted to the City for review and approval prior to installation.

4.1.4 STATE REGULATIONS

- A. Arizona Department of Water Resources (ADWR)

ADWR regulations are contained within A.R.S. § 45 and they encompass requirements for water supplies and the management thereof. In 1999, the Prescott Active Management Area was declared to be out of “safe-yield”, this enacted more regulations including the Assured Water Supply (AWS) rules. For the City, this resulted in a Decision and Order of AWS. All new water connections made to the City’s system require the confirmation that the supply exists and that there is entitlement to it.

- B. Arizona Department of Environmental Quality (ADEQ)

ADEQ’s Engineering Manual Bulletin No. 8 “Disinfection of Water Systems” and No. 10, “Guidelines for the Construction of Water Systems” and the Arizona Administrative Code, “Title 18-5-505.B - Approval to Construct”, contain specific requirements for submittals, approvals, and notifications when extension of a public water main is proposed. The developer and the Engineer of Record are expected to be aware of and comply with the above referenced regulations. Additional information can be found on the ADEQ website at www.azdeq.gov/

- 1. Before civil infrastructure construction permits are issued, the developer shall submit to the City Engineer an Approval to Construct (ATC) from ADEQ or the local delegated authority.

2. Prior to the City Engineer issuing an Approval to Operate (ATO) and release from the City of Prescott, the developer shall deliver to the City a signed Approval of Construction (AOC) from ADEQ or the local delegated authority.
- C. Yavapai County Environmental Services Department (YCESD)
1. Before civil infrastructure construction permits are issued, the developer shall submit to the City Engineer an Approval to Construct (ATC) from ADEQ through the YCESD delegation.
 2. Prior to the City Engineer issuing an Approval to Operate (ATO) and release from the City of Prescott, the developer shall deliver to the City a signed Approval of Construction (AOC) from ADEQ through the YCESD delegation.
- D. City of Prescott (COP)
1. Issuance of an Approval to Construct by YCESD or ADEQ does not constitute an approval of plans and reports by the City of Prescott. No construction shall commence prior to the City of Prescott City Engineer issuing a permit.
 2. An as-built plan submittal is required prior to receiving an “Approval to Operate and Release” from the City of Prescott.

SECTION 4.2/ GENERAL INFORMATION

4.2.1 WATER AVAILABILITY

A. Prescott Active Management Area (PrAMA)

The City of Prescott is located within the Prescott Active Management Area (PrAMA), established under the Arizona Groundwater Code of 1980 (A.R.S. § 45). The City and other water providers in the AMA are required to follow state laws and the water management plans (MP) specific to the AMA. State statute outlines five MPs, they are issued for ten (10) year periods starting in 1980; the fifth and final MP will be 2020 to 2025.

B. Water Service Area

The City of Prescott water service area is located within the PrAMA. The City of Prescott water service area accounts for approximately ten (10%) percent of the

land within the PrAMA and includes the City limits of Prescott, certain surrounding areas of the unincorporated part of Yavapai County, portions of Chino Valley, and the Yavapai-Prescott Indian Tribe Reservation. All water service is subject to water contracts or settlements and known under a general term of water service agreements.

C. Private Water Companies

1. Within the City's water service area, there are private water companies using water from their own sources or master metered by the City of Prescott (all are documented with ADWR). These include Home Owners Associations (HOA), Limited Liability Companies (LLC) or other entities established to provide ownership, operation and maintenance responsibilities.
2. Proposed private water mains located within the City of Prescott's rights-of-ways or easements will require an agreement between the City of Prescott and the private water company delineating liability and maintenance responsibilities. Private water line design and materials shall comply with City of Prescott, General Engineering Standards.
3. Private water companies shall review and approve the construction of, and modification to, water systems within their franchise areas. The developer will submit to the City Engineer written documentation that the private water company has approved facilities shown on the final plans before the City of Prescott grants approval of the project.
4. The City Engineer shall review development plans, which contemplate new water demands, prior to construction. The City of Prescott cannot provide water service within private water company, franchise areas. If the work will occur within the City of Prescott's rights-of-way, easements, or the private system connects to City of Prescott public water system, then plans shall be reviewed by the City and meet City standards as outlined in Prescott City Code (PCC) and this Article. In cases where the City of Prescott reviews private water systems, the applicable review fees shall be paid. A note will be placed on the final construction and as-built plans stating that the operation and maintenance of private main lines and appurtenances is not the responsibility of the City of Prescott. Additionally, the establishment of Conditions, Covenants, and Restrictions (CCR) and/or an LLC or HOA shall be required in accordance with Prescott City Code (PCC). The responsibility to blue stake water mains within a private district shall lie with the private entity.

D. Water Service Agreement

1. All new water connections made to the City's system require the confirmation that the supply exists and that there is entitlement to it. The City's Decision and Order is the document that outlines the supplies while Prescott City Code (PCC)s and Water Management Policy outline the rules and policies for water service. Developers making application for a Water Service Agreement (WSA) are encouraged to meet with staff prior to application due to the complexity of the issues related to the allocation of water.
2. An application and fee shall be required for all water service agreement requests. These may be found on the City of Prescott website at www.prescott-az.gov/documents.

4.2.2 AGREEMENTS

Developers and property owners who install improvements to the public water system may be eligible to request the formation of a reimbursement district allowing for partial reimbursement of costs to design and construct the improvements. The City of Prescott has specific reimbursement district criteria for developers or property owners that allow them to collect main reimbursement charges and compensation for the cost of over sizing or extending water mains, refer to Prescott City Code (PCC), Section 2-1-10.

SECTION 4.3/ PLAN SUBMITTAL REQUIREMENTS

Improvement plans submitted to the City of Prescott for approval shall adhere to professional engineering standards governed by the State of Arizona Board of Technical Registration. Improvement plans shall be complete and shall comply with City of Prescott and State Board requirements. Improvement plans that do not meet industry standards or are incomplete shall be returned by the City of Prescott without review comments and shall be determined to be an incomplete project submittal. Repetitive submittals of non-compliant and incomplete improvement documents may be subject to State of Arizona Board of Technical Registration review.

SECTION 4.4/ WATER SYSTEM REPORTS

4.4.1 GENERAL INFORMATION

- A. All projects shall be required to submit a Water Master Plan Report and/or a Water System Design Report. The purpose of these reports is to provide the City of Prescott with information regarding the potential water demands of the project

and verify the capability of the City of Prescott to provide the domestic water use and fire flow demands that will be required.

- B. All Water System Reports shall be prepared under the direction of a licensed civil engineer in the State of Arizona. The report shall be sealed and signed by the Engineer of Record in accordance with the requirements of the State of Arizona Board of Technical Registration, and submitted to the City Engineer.
- C. Reports shall provide an analysis of the impact that a development will have on the City of Prescott's water system. The objectives of the Water Reports are to verify the water demand, the system's ability to supply and maintain sufficient fire flows, and the available system flows, pressures, and proposed hydraulic regimes. All reports submitted to the City of Prescott for review shall be prepared in accordance with the following guidelines:
 - 1. Format:
 - a. The report shall be on letter-sized paper eight and a half inches by eleven inches (8 ½" X 11")
 - b. All reports shall have a table of contents, including page numbers
 - c. Maps and other supporting materials larger than folded ledger size paper eleven inches by seventeen inches (11" X 17") shall be placed into sleeves as an appendix to the report
 - d. A civil engineer licensed to practice in the State of Arizona shall seal each report
 - e. The project name shall be located on the cover
 - f. The name, address, and phone number of the developer/owner and Engineer of Record shall be stated on the cover
 - g. The original submittal date and all subsequent revision dates shall be located on the cover
 - h. Common spreadsheet formats shall be compatible with MS Excel
 - 2. Existing Conditions:
 - a. Reference any existing master plans or design reports applicable to adjacent development

- b. Include excerpts from existing plans or reports as applicable
 - c. Indicate the dates, times and results of certified flow testing of the existing water system
 - d. Include a description based on all parts or reference within a platted development
 - e. Describe the existing topography, and landform features
 - f. Include the location and description of existing utilities in the vicinity
 - g. Describe the existing and proposed site zoning and land uses
 - h. Include reference to elements of the City of Prescott's General Plan and identify any designated character area or studies that will affect the project's design
3. Proposed Conditions:
- a. Summarize the proposed development. Include a site plan that indicates the layout of the proposed development with a survey a minimum of one-hundred (100') feet beyond the project limits or as determined by the City Engineer
 - b. Identify the location and size of all proposed connection(s) to the City of Prescott's water system
 - 1) Show all looping and/or extension of water mains into the site
 - 2) Indicate the location of all pressure zone boundary lines
 - c. Provide a looped system for all water supply demands needed to meet the minimum requirements of the Fire Department, maintain water quality standards, and maintain system redundancy for operational flexibility
 - d. Reference which water zone the site is within and address all required fire flows and system pressures
 - e. Address any maintenance responsibilities of the proposed water system

4. Mapping:
 - a. All mapping shall conform to City of Prescott adopted CADD standards and the GES Article 8, Survey Controls
 - b. The project name shall be located on each sheet
 - c. The name, address, and phone number of the developer/owner and Engineer of Record shall be stated on each sheet
 - d. The original submittal date and all subsequent revision dates shall be located on the exhibits
 - e. All maps shall be prepared to an appropriate scale that is legible
 - f. All proposed or existing on-site and off-site facilities including, but not limited to, pump stations, transmission and distribution mains, and reservoirs
 - g. All proposed or existing street locations, parcel boundaries, and proposed lots within each parcel
 - h. Label contour lines at two (2') foot intervals
 - i. Indicate pressure zone boundaries, pressure-reducing valves (PRV'S), and corresponding zone valves
 - j. Include size and pressure settings for all pressure reducing valves (PRV).
 - k. Include a vicinity map showing existing and proposed streets to a distance of one (1) mile from the exterior boundaries of the project

4.4.2 WATER MASTER PLAN REPORT CONTENT

- A. When phasing is contemplated or when required by the City Engineer, a Water Master Plan Report shall be prepared in accordance with the requirements of the General Engineering Standards (GES), [Section 4.4](#). The report shall also address, but not be limited to the following:
 1. The report shall specify the terms and requirements for water service to the development, including any off-site improvements

2. All development project owners shall be responsible for determining their specific water system needs between interim and buildout
 - a. Include the projections for future surrounding developments based on the adopted General Plan Land Use Map to ensure there is no adverse impact on the system

3. A computer water network model, which is compliant with the GES [Section 4.4.4](#) shall be used for the analysis of pressure and flow within the distribution system, verifying that adequate pressures, domestic and fire flows will be available within the proposed development based on projected demands and shall include the following:
 - a. If certified flow tests performed on the system during peak demand to which the project is to be connected do not show that sufficient capacity exists, the computer model will be used to determine the required on-site and off-site facilities, such as pump stations and pipelines, necessary to serve the project
 - b. If the proposed development requires a change in zoning that increases density or proposes a water system different from the City of Prescott's Water Master Plan, then additional upstream and downstream off-site analysis will be required
 - c. Modification of existing or creation of new pressure zones shall require approval of the City Engineer

4. Master plan mapping shall demonstrate the following:
 - a. Demonstrate compliance with the City of Prescott's Water Model, which encompasses the respective area
 - b. Master planned developments that design a distribution system that will be phased shall provide a synopsis of the phasing with modeling backup to the Public Works Department prior to acceptance of the developments Water Master Plan
 - c. Each phase of the master plan must be a standalone document when submitted to the City of Prescott for review of that specific phase

4.4.3 WATER DESIGN REPORT CONTENT

- A. A Water Design Report shall be prepared in accordance with the requirements of the General Engineering Standards (GES) [Section 4.4.1](#).
- B. The system design shall comply with the Arizona Department of Environmental Quality (ADEQ), Engineering Manual Bulletin No. 10 and the Arizona Administrative Code R18-5 requirements for system performance and redundancy. The developer shall coordinate water system improvements with the City of Prescott water model and the Capital Improvement Plan.
- C. In addition to the general requirements in the GES [Section 4.3.1](#) and [4.4.1](#), the Water Design Report shall at a minimum, include the following:
 - 1. Design Documentation
 - a. Note the design compliance with the latest revision of this manual and all other applicable design standards and codes
 - b. Include a narrative of which design procedures, policies and methodologies will be incorporated into the design engineering of the water system
 - c. List the title and version of any software used in the design analysis
 - 2. A summary of the anticipated water demands for the project
 - a. The priority of obtaining water design flows for a non-residential facility is, 1) flow measurements from the actual facility; 2) the use of flows from similar facilities in the immediate area; 3) the use of similar flows from similar facilities in other areas; and 4) flow tables as listed in the GES, Article 4, [Table 4-1](#)
 - b. Flows may be calculated on a sub-area basis, however, total flows for the entire development shall also be provided
 - c. Flow rates shall include average day, maximum day, peak hour flows, and fire flow demands
 - d. Include fire flows and fire sprinkler system demands that will be required for the development in accordance with the adopted Fire Code, City of Prescott Amendments, and site-specific criteria as may be established by the City of Prescott Fire Marshal
 - 3. All projects shall be required to provide a fire analysis that shall demonstrate that there are adequate fire flows available from the City of Prescott's existing

system to meet the required fire demands of the proposed development, considering the building construction type, height, layout, etc.

- a. The Engineer of Record shall use a hydraulic model to evaluate the ability of the water distribution system to deliver fire flows to the development
- b. The developer is responsible for understanding the fire flow requirements of the structures that are to be built and shall assure that the water distribution system within the development is capable of delivering the required fire flows
- c. Improvements, including off-site improvements may be required to meet these criteria
- d. The fire flow demands shall be included in the overall water demand and summarized in the water design report

4. Site Specific Analysis

- a. The fire flow demand shall comply with the International Fire Code (IFC) and the National Fire Protection Association (NFPA), Standard 13, 13R and 13D, *Installation of Sprinkler Systems* as adopted by the City of Prescott and the Board of Technical Registration Substantive Policy Statement for fire sprinkler systems. The report shall list the applicable codes and standards and the appropriate engineering practices
- b. The “fire flow demand” section shall provide the following information:
 - 1) Engineer of Records name, company, address, City, State, zip code, and phone number
 - 2) Project name, limits, and address, general location & proposed uses
 - 3) Detail physical aspects of the proposed on-site structures including:
 - i. Building height (feet)
 - ii. Number of stories above finished grade

- iii. International Building Code (IBC) construction type
- iv. Building area (square feet)
- 4) The “fire flow” design report shall provide:
 - i. Revised International Fire Code (IFC) and most recent City of Prescott Amendments for required fire flow
 - ii. Percentage of reduction allowed for the installation of a fire sprinkler system and resultant system demand as approved by the Fire Marshal
- 5) Projects that install fire sprinkler systems shall be in accordance with the Arizona Board of Technical Registration Substantive Policy Statement regarding fire sprinkler systems. The following information shall be included:
 - i. Range of the fire hazards of the project
 - ii. The hazard classification of the intended occupancy, including any special hazards
 - iii. The appropriate engineering practices
 - iv. The availability and adequacy of the water supply
 - v. Based on a hazard analysis for the proposed use of each building (including special hazards), the appropriate fire sprinkler design density and area of operation shall be provided for each hazard area
 - vi. Anticipated fire sprinkler demand (psi and gpm)
 - vii. Required fire flow and anticipated fire sprinkler demand shall be provided for the project in accordance with the IFC and most current City of Prescott Amendments
- c. In order to ascertain the availability and adequacy of the water supply for the project, the report shall provide field fire hydrant flow and pressure test results performed in accordance with AWWA M-17 based on a certified test of the existing water system within the

previous sixty-(60) days. A flow test shall be witnessed by the design engineer and valves operated by City of Prescott Water Operations by calling 928-777-1118. The flow test information shall include:

- 1) Test date
- 2) Test time
- 3) Test locations
- 4) Test and residual hydrants
- 5) Orifice size
- 6) Orifice coefficient
- 7) Flow test data:
 - i. Static pressure (psi)
 - ii. Residual pressure (psi)
 - iii. Pitot measurement (psi)
 - iv. Recorded flow rate (gpm)
 - v. Flow rate (gpm) converted to twenty (20) (psi)
- 8) Testing technician
- 9) A copy of the written City of Prescott Water Operations or Fire Department report showing the flow test was witnessed by City personnel

4.4.4 WATER MODELING

A. Water System Model Analysis

1. The Engineer of Record shall provide a model of the planned Water System Master Plan Report or Water System Design Report as delineated in the GES [Sections 4.4.1](#) and [4.4.2](#). This modeling shall be required where new public water mains are being added, modified or extended in the City of Prescott system. The model shall provide the data necessary

to validate that water demands are met to support the project. A water model is also required to demonstrate the required fire flow is adequate in accordance with the adopted Fire Code and City of Prescott Amendments.

2. Infill projects may not require a model. Infill projects include single lot development where fire and domestic flows are taken directly from existing City of Prescott mains and where the zoning is in conformance with the City of Prescott General Plan. If a model is not required, include narrative of the proposed connections to the City of Prescott.

B. When water hydraulic modeling information is needed, the following requirements shall be met to deliver model data to the City of Prescott.

1. Model Software

The City of Prescott currently uses the H₂OMAP Water software. The modeling software used to evaluate a proposed development does not need to be the same software, but the data shall be provided to the City of Prescott in EPA-NET 2.0 format.

2. Model Development

The developer or developer's engineer shall create and utilize a hydraulic model to demonstrate that the proposed infrastructure size, hydraulics, etc. are adequate and satisfy the City of Prescott's requirements for domestic and fire flow demands. The City of Prescott may provide specific boundary conditions upon request. The model information shall be delivered to the City of Prescott to be added to the City of Prescott's model, where modeling evaluations may be performed to verify that the proposed infrastructure and demands will not negatively affect the existing COP system. Proposed water systems shall evaluate the City of Prescott's overall water supply, pumping, and storage requirements.

C. Hydraulic Model Data

1. All drawings and model data shall use the City of Prescott coordinate system and datum in the GES, Article 8.
2. Model data shall be free of Topology errors in accordance with commonly accepted engineering practices prior to submission to the City of Prescott.
3. Water Distribution System Attributes:

- a. Mains – Diameter, length, material, location, connectivity with other entities
 - b. Reservoirs – Base elevation, height, volume, location, connectivity with other entities
 - c. Pump Stations – Layout, number of pumps, pump curve or design point, elevation, location, connectivity with other entities, pump control scheme
 - d. Nodes (Connection Points) – Location, elevation, demand
 - e. Pressure Reducing Valves and Tank Fill Valves – Location, connectivity with other entities, elevation, number of valves, diameter and valve set points
 - f. Wells – Location, connectivity, design flow, control scheme
 - g. Polygons - Pressure zone boundaries, zoning/density
4. All model data shall include the following:
- a. Demands shall be calculated per GES [Section 4.4.3.C](#) using average day water demands in gallons per day
 - b. The system shall be capable of providing maximum day demand plus required fire flow
 - c. Verification of the ability of the proposed system to provide adequate flow and pressures under the various demand combinations shall be provided
 - d. Verification that the minimum required pressure throughout the water distribution system is achieved at the highest finished floor elevation or service point in the fire sprinkler line. If the minimum residual pressure under fire flow conditions cannot be achieved, additional private improvements may be required
 - e. Pipes and nodes – Include item identification, demands, elevation, hydraulic grades, length, status, diameter, velocity, headloss/1000 ft

- f. Reservoirs and pumps – Provide identification, diameter, height, elevation, upstream and downstream hydraulic grade, number of pumps, pump capacity and proposed pump curves
 - g. Pressure Reducing Valves (PRV's) – Provide identification, size, elevation, upstream and downstream hydraulic grade settings
5. Hydraulic Evaluation
- a. New developments that do not constitute infill shall be modeled to determine if the infrastructure is adequate to serve the development and provide the level of service as defined by the City of Prescott's performance criteria.
 - b. The City will use the City of Prescott's water model to verify the ability of the existing water distribution system to deliver adequate water to the proposed development per the design Engineer's proposal. Supplemental fire hydrant tests shall also be used to make this determination. The developer is responsible to make certain that all fire flow requirements (hydrants and/or sprinklers) within the development satisfies the City of Prescott General Engineering Standards and adopted Fire Code requirements.
 - c. The following scenarios shall be modeled:
 - 1) Average Day Demands
 - 2) Maximum Day Demands
 - 3) Fire Flow Demands (Which are defined as the maximum daily demand plus the appropriate fire flow demand)
 - d. Model simulations shall be documented in a graphical and tabular format to demonstrate that the water distribution system shall provide the required flow at suitable pressures and water main velocities.
 - e. Fire flow modeling results will be documented as follows:
 - 1) Junction Report
 - i. Static Pressures
 - ii. Residual Pressures

- iii. Demands
 - iv. Calculated Minimum Pressure
 - v. Elevation
 - vi. Pressure Head
 - vii. Hydraulic grade
- 2) Pipe Report
- i. Length
 - ii. Type
 - iii. Diameter
 - iv. Velocities
 - v. C-Factor
 - vi. Control Status
 - vii. Discharge
 - viii. Head Loss
 - ix. Head Loss Gradient
- f. Boundary conditions that represent the interface between the development and the rest of the distribution system, shall be explained clearly.

**TABLE 4-1
UNIT DESIGN FLOWS FOR WATER**

| TYPE OF FACILITY SERVED | APPLICABLE UNIT | WATER FLOW PER APPLICABLE UNIT GALLONS PER DAY (GPD) | DESIGN PER PER |
|---|--|---|-----------------------|
| Airport | Passenger (average daily number) Employee | 5 18 | |
| Auto Wash | Facility | Per manufacturer, if consistent with this Article | |
| Bar/Lounge | Seat | 36 | |
| Barber Shop | Chair | 42 | |
| Beauty Parlor | Chair | 120 | |
| Bowling Alley (snack bar only) | Lane | 90 | |
| Camp | | | |
| Day camp, no cooking facilities | Camping Unit | 36 | |
| Campground, overnight, flush toilets | Camping Unit | 90 | |
| Campground, overnight, flush toilets and shower | Camping Unit | 180 | |
| Campground, luxury | | | |
| Camp, youth, summer, or seasonal | Person Person | 120-180 60 | |
| Church, Without Kitchen | Person (Maximum attendance) | 7 | |
| Church, With Kitchen | Person (Maximum attendance) | 8 | |
| Country Club | Resident member Non-resident member | 120 12 | |
| Dance Hall | Patron | 6 | |
| Dental Office | Chair | 600 | |
| Dog Kennel | Animal, Max occupancy | 18 | |
| Dwellings | Person | 96 | |
| Fire Station | Employee | 54 | |
| Hospital, AllFlows | Bed | 300 | |
| Hospital, Kitchen Waste Only | Bed | 30 | |
| Hospital, Laundry Waste Only | Bed | 48 | |
| Hotel/Motel, Without Kitchen | Bed (2 person) | 60 | |
| Hotel/Motel, With Kitchen | Bed(2 person) | 72 | |
| Industrial Facility, without Showers | Employee | 30 | |
| Industrial Facility, with Showers | Employee | 42 | |

| | | |
|---|-----------------------------|---|
| Industrial Facility, Cafeteria added | Employee | 6 |
| Institutions, Resident | Person | 90 |
| Institutions, Nursing Home | Person | 150 |
| Institutions, Rest Home | Person | 150 |
| Institutions, Laundry, Self Service | Wash Cycle | 60 |
| Institutions, Laundry, Commercial | Washing Machine | Per Manufacturer, if consistent with this Article |
| Office Building | Employee | 24 |
| Park (Temporary Use) | | |
| Picnic, with showers, flush toilets | Parking Space | 48 |
| Picnic, with flush toilets only | Parking Space | 24 |
| Recreational Vehicle, with no water and sewer connections | Vehicle Space | 90 |
| Recreational Vehicle, with water and sewer connections | Vehicle Space | 120 |
| Mobile Home/Trailer | Space | 300 |
| Restaurant/Cafeteria | Employee | 24 |
| With toilet, add | Customer | 8 |
| Kitchen Waste, add | Meal | 7 |
| Garbage Disposal, add | Meal | 1 |
| Cocktail Lounge, add | Customer | 2 |
| Kitchen Waste Disposal Service, add | Meal | 2 |
| Restroom, Public | Toilet | 240 |
| School, Staff and Office | Person | 24 |
| Elementary, add | Student | 18 |
| Middle and High, add | Student | 24 |
| With Gym & Showers, add | Student | 6 |
| With Cafeteria, add | Student | 4 |
| Boarding, Total Flow | Person | 120 |
| Service Station, with Toilets | First Bay | 1200 |
| | Each Additional Bay | 600 |
| Shopping Center, no Food or Laundry | Square Foot of Retail Space | 0.1 |
| Store | Employee | 24 |
| Store, Public Restroom, add | Square Foot of Retail Space | 0.1 |
| Swimming Pool, Public | Person | 12 |
| Theater, Indoor | Seat | 6 |
| Theater, Drive-In | Car Space | 12 |

SECTION 4.5/ WATER SYSTEM DEMAND

4.5.1 SYSTEM DEMAND

The water system demand describes the standards against which the water infrastructure is measured to determine the criteria of the proposed infrastructure. This water system demand criteria is based on the Arizona Administrative Code (AAC) and other applicable regulatory standards.

4.5.2 INFRASTRUCTURE SIZING

Infrastructure shall be sized to supply the maximum day demand plus fire flow. This will be determined by using the average daily demands listed in [Table 4-1](#) and adding a maximum day peaking factor of one point eight (1.8). Fire flows shall be determined by the most currently adopted International Fire Code and identified by the City Fire Marshal. [Table 4-1](#) lists water usage in categories that are correlated to sewage collection Unit Design flows as shown in the (AAC).

SECTION 4.6/ PLANNING

4.6.1 COMPREHENSIVE PLANNING

- A. The City of Prescott has developed a Water System Model to ensure that future improvement requirements are achieved through an orderly development of the City of Prescott planning areas. Any engineer working on the design of public or private water infrastructure shall develop plans consistent with the appropriate sections of the City of Prescott's Model.
- B. Water mains shall be installed along the entire length of the property line frontage of the property being developed. The property line frontage is defined as that portion of a parcel of property that abuts a street, easement, or public rights-of-way. If a parcel to be developed has more than one frontage, improvements may be required to be installed along all frontages.

4.6.2 WATER FACILITIES

- A. Water facilities (wells, reservoirs, booster pump stations, etc.) are designed and constructed by the City of Prescott through its capital improvement program. Developers needing to construct water facilities shall contact the Public Works Department and request a meeting to coordinate the design of these facilities. The developer shall be prepared to address how the proposed system will conform to the City of Prescott's Water Master Plan. The City of Prescott will address design issues and the review process for these facilities.

B. Unless otherwise agreed to in writing by the City Engineer, water facilities shall be located on a tract or lot dedicated to the City of Prescott (conveyed by a general warranty deed) and accompanied by a title policy in favor of the City of Prescott.

C. Wells

1. ADWR issues the authority to drill groundwater wells both exempt and non-exempt. Before drilling a well, a Notice of Intent to Drill shall be filed with ADWR and a “drill card” must be obtained from ADWR. Non-exempt wells require greater ADWR review and public noticing. Post drilling, and well and pump installation, required well logs, and pump completion report must be filed with ADWR.
2. In the event a property has an existing exempt well, and the use continues while a connection is made to the City’s potable water system, a backflow prevention device is require by Prescott City Code (PCC), Section 3-6-2 (International Plumbing Code, Amendments).
3. A property owner currently using an exempt well that seeks to discontinue its use for which the City is able to provide water supplies, then the well owner shall complete ADWR well abandonment requirements. Water service to the property will require a water service agreement to document the volume to be served and conditions of the service. Refer to the adopted City of Prescott Water Management Policy.
4. Refer to GES [Section 4.7.1](#) for information regarding the drilling of new exempt wells.

D. Tanks

Storage facilities shall provide operational storage for peak demands above maximum daily demands, emergency storage for power outages and line breaks, and fire protection storage. Facilities shall be designed to maximize the efficient use of water production wells and pumping facilities. Therefore, storage in each designated service area shall exceed the volume of the sum of 1, 2 and 3 below or the average day demand for the area(s) served, whichever is greater.

1. Operational storage equaling twenty (20%) percent of maximum daily demand
2. Fire storage as determined by the fire department ranges from four (4) to ten (10) hours for multiple fire occurrences within a pressure zone. Fire demand rates and storage volumes are determined using criteria

established by the American Insurance Association according to the following equation: $G=1,020 \sqrt{P(1-.01\sqrt{P})}$

P = Population of the Area

G = Storage in Gallons

3. Emergency storage equaling ten (10%) percent of maximum daily demand
4. The City Engineer shall determine final sizing and dimensions.

E. Booster Pump Stations

1. Booster pumps shall be designed to maintain adequate pressure for domestic and fire protection water supply. All stations shall provide at a minimum; variable frequency drives, backup power supply, separate electrical room and supervisory control and data acquisition (SCADA) compatible with the City of Prescott's current system. Designers shall refer to Engineering Bulletin No. 10, Arizona Department of Environmental Quality, for additional design criteria. The City Engineer shall determine final sizing, number of pumps, pump capacity, SCADA requirements, and building dimensions.
2. A preliminary design report shall be prepared and submitted to the City Engineer for review by the Public Works Department prior to submittal of final plans for review. This report shall outline the type of equipment and controls proposed Piping and Instrumentation Diagram (PI&D) and SCADA for the station along with the proposed hydraulics. A final design report prepared by a registered civil engineer licensed in the State of Arizona shall accompany the construction drawings and specifications.

SECTION 4.7/ TRANSMISSION AND DISTRIBUTION SYSTEMS

4.7.1 GENERAL INFORMATION

- A. Effective January 1, 2006 A.R.S 45§ 45-454 (C) prohibits the drilling of an exempt well (max. pump capacity of not more than 35 gpm and that is used only for non-irrigation purposes) on land if any part of the land is within 100 feet of an operating distribution system with a Decision and Order of AWS. For additional detail, see the statute referenced and ADWR Substantive Policy Statement GW43.
- B. The City of Prescott requires water mains to be installed along the entire length of the property line frontage of that property being developed. The property line

frontage is defined as, that portion of a parcel of property that abuts a street, easement, or public rights-of-way. If a parcel to be developed has more than one frontage, improvements may be required to be installed along all frontages if required flow, system redundancy, water quality or paving issues conflict.

C. Design Policy

1. The City of Prescott will require the extension of water mains along a frontage, or through a subdivided parcel, to the boundary where future extension of the water main is possible, providing a point of service to adjacent properties, or as determined necessary by the City Engineer. Reconstruction of residential or commercial structures requires compliance with all current ordinances and design guidelines relating to water main extensions, main sizing, and service lines.
2. Each lot shall have safe, reliable, and potable water in sufficient volume and pressure for domestic use and fire protection. The Engineer of Record shall verify this by performing a flow and pressure test during peak demand period, of that part of the potable system to be extended or connected onto. The flows and pressure shall meet minimum requirements for domestic and fire flow per applicable regulatory standards. The Engineer of Record shall place a statement verifying this within the design report along with copies of the tests.
3. The City of Prescott maintains over eighty-(80) pressure zones and care shall be taken to identify boundary conditions when designing near a zone line. The Engineer of Record shall contact the Public Works Department for verification of water pressure zone boundaries. Static and dynamic water pressure and flow tests shall be performed on fire hydrants located on each leg of the existing water system between zone boundaries where connections are proposed.

D. Design Standards

1. The Engineer of Record shall design proposed water infrastructure in accordance with the design standards listed in the GES. These documents contain construction related specifications and details that impact the design of water systems including trenching, bedding, backfill and pavement replacement, etc.
2. Water system expansion and extension requirements to serve both new and existing development are outlined in the Prescott City Code (PCC), Article 2, Chapter, 2-1.

4.7.2 WATER MAIN PIPES AND MATERIALS

A. Pipe Sizing

1. The minimum size of a water main for providing fire protection and serving fire hydrants shall be six (6") inches in diameter when service is provided from two (2) directions, or where the maximum length of six (6") inch pipe serving the hydrant from one (1) direction does not exceed two-hundred fifty (250') feet. A minimum eight (8") inch water main shall be required where service is provided from one (1) direction only. Larger size mains shall be provided as necessary to meet all other design criteria, the development's design report, specific water demands, or the City of Prescott Water System Master Plan.
2. Refer to the adopted Fire Code and City of Prescott Amendments for the maximum length of dead end fire lines that may be used for fire protection. Fire lines shall be calculated from the street tap to the building for determining sprinkler design.
3. One (1") inch, two (2") inch or three (3") inch water mains are not permitted for new construction as public water mains. Four (4") inch mains may be considered with approval from the City Engineer.

B. Pipe Materials

1. Preferred Material: Class 350 (CL350) Ductile Iron Pipe (DIP) cement mortar lined and seal coated is preferred for all water main installations.
2. The following material will be considered on an individual project basis, with approval of the City Engineer.

Alternate Material: The use of Polyvinyl Chloride (PVC) pipe when installed in accordance with COP Standard Detail 200P-2.
3. Service connections for meters shall be one (1") inch minimum, Type K copper without splices.
4. Remote Fire Department connection piping shall be CL350 DIP. Below the vertical section at the street, an approved drip valve shall be installed over a bed of gravel as per Fire Department COP Standard Detail "Remote Fire Department Connection". The line shall include tracer wire per COP Standard Detail 319Q-1.

5. Fire line services three (3") inches and smaller shall be connected to the main with a minimum four (4") inch DIP, CL350 with a resilient seat valve. The DIP pipe shall be extended beyond the resilient seat valve to the property or easement boundary.
6. Design analysis for wall thickness shall be performed for external loads. These include, but are not limited to, pipelines crossing under stormdrain lines greater than thirty-six (36") inches in diameter, pipelines in the roadway alignment that would be exposed to construction vehicle loads prior to paving, and installations exceeding the pipe manufacturer's maximum depth of bury.
7. All aggregate material within the trench envelope of water mains and/or fire lines shall be noncorrosive as determined by the Arizona Department of Transportation (ADOT) Standard Specification Manual 501-3.02 for pH and resistivity. Test results shall be submitted to the City Engineer prior to construction.

4.7.3 SYSTEM LAYOUT

- A. To provide appropriate water pressure, water circulation and redundancy, all new water mains shall be designed in a looped configuration where feasible, as determined by the City Engineer, providing a minimum of two sources that can be isolated by gate valves.
- B. Water mains shall not be located within ten (10') feet of a building or retaining wall.
- C. For purposes of horizontal and vertical separation, stormdrains, non-potable water systems, and sewer service lines shall be treated as sewer mains. Separation shall be required pursuant COP Standard Detail 404P-1 and 404P-2.
- D. Existing water main stubs and service lines adjacent to a proposed development that are not used shall be removed. For mains, this includes removing all valves, capping the tee and removing the pipe at the main. For services, this includes removing the saddle and its appurtenances and installing a full circle stainless steel clamp.
- E. Main Locations:
 1. Public water mains and appurtenances are required to be located within dedicated public right-of-way or easements

2. Horizontal Location: All water mains shall be aligned parallel to property lines or street center lines to the maximum extent possible and shall not cross and re-cross the centerline
 - a. Water mains located beneath streets within the public right-of-way shall refer to COP Standard Details 601P thru 606P
 - b. Public water mains in commercial, multifamily, and industrial developments shall be located under driveways, or drive aisles and a minimum of ten (10') feet from any structure, including overhangs
 - c. A minimum twenty (20') foot easement shall be provided and the water main shall be located in the center of the water main easement
 - d. Horizontal alignment changes shall be made with eleven and one-quarter ($11 \frac{1}{4}$), twenty-two and one-half ($22 \frac{1}{2}$), forty-five (45), or ninety (90) degree bends. Joint deflection shall not exceed seventy-five (75%) percent of the pipe manufactures specification
 - e. Horizontal and Vertical locations shall be consistent with COP Standard Detail 601P thru 606P and 404P-1
3. Vertical Location:
 - a. New water mains, fire lines, and water service lines are not allowed to pass under detention basins
 - 1) This does not apply to landscape irrigation lines downstream of proposed backflow prevention devices or buildings
 - b. Hydrants, meters, blow-offs and valves shall not be located in washes, detention areas, private driveways, sidewalks, curbs or valley gutters
 - 1) Hydrants shall have a maximum bury depth of six (6') feet.
 - c. Hydrants that require adjustment as a result of improvements shall be adjusted using a "Gradlok" or approved equal when vertical adjustment is in excess of six (6") inches, refer to COP Standard Detail 363Q

4.7.4 DEAD-END MAINS

- A. The maximum length for a dead-end water distribution main shall be governed by water quality standards and shall be sufficient to provided minimum pressures and fire flows.
- B. Capped dead-end mains shall be fitted with a flushing device per COP Standard Detail 318P. Fire hydrants may be used as a flushing device. Flushing devices shall not be located in washes, detention areas, retention areas, sidewalks, driveways or paved areas.

4.7.5 DESIGN FLOWS

- A. The ultimate design flow within the City of Prescott’s water transmission and distribution system will be based on the City of Prescott’s current water master plan. Water demand for each development will be calculated using the average day demands with appropriate peaking factors applied, as shown in [Table 4-1](#) to ensure that the existing distribution supply is sufficient. Designs will include all necessary improvements, including booster-pumping stations, reservoirs, mains and appurtenances to meet the system’s ultimate demand. Design flow C factors shall be applied using [Table 4-2](#).

**TABLE 4-2
C-FACTORS**

| DIAMETER | C-FACTOR |
|----------|----------|
| 6 | 90 |
| 8 | 110 |
| 10 | 115 |
| 12 | 120 |
| 14 | 120 |
| 16 | 130 |
| 18 | 130 |
| 20 | 130 |
| 24 | 130 |
| 30 | 130 |
| 36 | 130 |

- 1. Hydraulic analysis will demonstrate that the system will provide average day, maximum day demand, and maximum-day demand plus fire flow. The average day to maximum day peaking factor is one point seven one (1.71), and the max day to peak hour factor is one point eight (1.8).

2. A minimum of twenty (20) psi shall be maintained at the point of maximum fire flow demand during max day plus fire flow conditions. Impacts to surrounding areas shall also be considered.
 - a. Water velocity criteria under maximum day demand conditions are as follows:
 - 1) Velocity \leq 5 feet per second (fps) for pipes < 36 inches diameter (Head loss, HL= 2-7 feet/1,000 feet)
 - 2) Velocity \leq 6 fps for pipes \geq 36 inches diameter (Head loss, HL= 1-2.5 feet/1,000 feet)
 - b. Velocity criteria under maximum day plus fire demand conditions is as follows:
 - 1) Velocity \leq 10 fps
3. Design flows for all distribution systems will be based upon flow and pressure of the existing system as documented by the Engineer of Record.
4. Prior to acceptance by the City Engineer, all platted subdivisions will conduct an additional flow test at the fire hydrants or flushing devices having the lowest and highest ground elevation within the subdivision.
5. Developments that cross pressure zone boundaries shall conduct a flow test within each pressure zone as outlined above. The results of this test, along with a copy of the final plans, shall be submitted to the City Engineer for review and acceptance.

4.7.6 FIRE HYDRANT FLOW TEST REQUIREMENTS

- A. Pressure and available flow information for existing water mains shall be obtained by having a flow test performed on the system at the developer's expense. Flow tests are required for all commercial projects, multi-family residential projects and public extensions of the City of Prescott's water distribution system. A private fire protection company shall perform the tests in accordance with AWWA manual M-17 and certify the results. The design engineer shall witness all tests and valves operated by City of Prescott Water Operation's personnel. The certified flow test results shall be included in all master plans or design reports submitted to the Public Works Department. A

copy of the flow test report shall be included when fire sprinkler plans are submitted.

- B. Flow tests may be scheduled through Water Operations at 928-777-1118 a minimum of 48 hours prior to the test.

4.7.7 PRESSURE REQUIREMENTS FOR NEW DEVELOPMENT

- A. Pressure extremes in water systems result in the potential for contaminants to enter the network. Low pressures in the water system may allow polluted fluids to be forced into the system, and high pressures may cause ruptures or breaks in the network.
- B. The static pressure in the distribution system shall not exceed one-hundred twenty (120) pounds per square inch (psi), and the system shall be designed to maintain a minimum residual pressure of forty-(40) psi at the point of service (box and setter or meter) to be served by system pressure at maximum day demand conditions. The system will be designed to maintain twenty (20) psi minimum pressure under design maximum day plus fire flow requirements. The twenty (20) psi minimum pressure design requirement accounts for the safety factor included in conservative “C” factors to account for aging infrastructure and flexibility in locating pressure zone boundaries.
- C. All distribution water mains, appurtenances and service lines will be designed for a minimum normal internal working pressure of one-hundred fifty (150) psi. Working pressures for transmission mains will be verified with the Public Works Department.
- D. Provisions shall be made to protect the system from water hammer pressures. The occurrence and severity of water hammer can be reduced by using pressure-release valves, surge tanks, variable frequency drives, soft start motor controllers and air chambers.

4.7.8 PRESSURE REDUCING VALVES (PRV’S)

- A. Approximate pressure zone boundaries and their respective elevations are shown in the City of Prescott’s water model. Within the distribution system or to interconnect to other zones, PRV’s shall be required to maintain pressure zones within the distribution system or to interconnect to other zones. Distribution systems will not be designed to operate at pressures in excess of one-hundred twenty (120) psi. PRV’s shall be built in accordance with COP Standard Detail 323Q. A minimum of one (1) PRV in each pressure zone will be designed with a high-pressure relief valve.

- B. Vaults shall be located outside of paved areas adjacent to the back of curb or sidewalk. PRV's shall be located within the rights-of-way, an easement, or an easement within a private street tract, and shall be provided with unobstructed vehicular access, including parking. Curbs adjacent to PRV vaults shall be Type C or Type D in accordance with COP Standard Detail 220Q-1, unless other access is provided that is acceptable to the City. Site grading shall route storm water and discharge water from relief valves away from the vault. Site design shall consider and mitigate any impacts of discharge water on downstream improvements. The location of pressure relief risers shall be shown on the final plans.
- C. The Engineer of Record shall specify in the design report and on final plans, the size of the main and low flow pressure reducing valves, the upstream system pressure, and the design downstream pressure setting.
- D. The City of Prescott requires all metered services to have a pressure-regulating valve installed on the private service line.
- E. The City of Prescott requires the installation of isolation valves on all PRV stations to facilitate the operation, maintenance, and expansion of the water distribution system.

4.7.9 VALVES

- A. The City of Prescott requires the installation of isolation valves to facilitate the operation, maintenance, and expansion of the water distribution system. Water main valves shall meet or exceed the pressure classification of the water main.
- B. Gate valves, required to control the operation of the water system, shall be installed per COP Standard Detail 301Q.
- C. All valves shall be resilient seat/wedge gate valves, epoxy-coated interior in accordance with American Water Works Association (AWWA) C-550, and all supplemental requirements of MAG Specification 630.3.

4.7.10 FITTINGS

- A. Joint deflection shall be evaluated and may be allowed for specific conditions, shall not exceed seventy-five (75%) percent of the pipe manufactures specification. Fittings may be required where more than two (2) pipe lengths are deflected.
- B. A minimum distance between fittings shall be specified on the final plans for constructability. All fittings shall be identified on both the plan and profile view

with station and elevations. The Engineer of Record is responsible for verifying the minimum distance necessary for the type and diameter of pipe and related fittings specified for the project.

4.7.11 VALVE LOCATIONS

- A. Shutoff valves shall be installed on water mains at locations within the distribution system that allow sections of the system to be taken out of service for repairs or maintenance without significantly curtailing service in other areas. Special consideration shall be given to the number of fire hydrants taken out of service. A sufficient number of valves shall be provided on water mains so that inconvenience and sanitary hazards will be minimized during repairs. Valves will be located such that closing no more than four (4) valves can isolate any section of the system.

- B. Valve Location
 - 1. A valve shall be located on each side of a vertical alignment per COP Standard Detail 370Q, creek, wash, railroad, or highway crossing.
 - 2. Valves shall not be located in curbs, sidewalks, driveways, valley gutters, bike lanes, and vehicle wheel paths.
 - 3. Provide a valve on each fire hydrant lateral and flange the valve to the tee. Provide a valve for all fire protection water supply connections in accordance with adopted Fire Code and City of Prescott Amendments. Refer to COP Standard Detail 360Q.
 - 4. The preferred locations of valves shall be set at the intersecting street adjacent to other valves, or at a fire hydrant location.

- C. Spacing
 - 1. Maximum spacing of water distribution main valves shall be as follows:
 - a. In Commercial, multi-family, and industrial areas, valve spacing will not exceed five-hundred (500') foot intervals
 - b. In single family residential, valve spacing cannot exceed eight-hundred (800') foot intervals or one (1) per block, whichever is less
 - 2. Maximum spacing of water transmission main valves will be as follows:

- a. Valves spacing will not exceed one-thousand three-hundred-twenty (1,320') feet
 - b. Intermediate access points shall be provided between valves for leak detection purposes
- D. Install all tees with a valve on the lateral main, so that the lateral can be taken out of service without interrupting the supply to other locations. At intersections of distribution mains valves shall be required on each leg.
- E. All valves require valve boxes installed per COP Standard Detail 391Q. All valves shall be installed with a debris cap. The debris cap handle shall be the appropriate color to indicate valve type. Refer to COP Standard Detail 391Q.
- F. All valve frame and cover adjustments will be per COP Standard Detail 391Q.

4.7.12 AIR RELEASE VALVES

- A. Air release valves shall be installed at all local high points of water mains four (4") inches or larger in diameter, as follows:
- 1. When water main changes from a positive slope to a zero slope or a negative slope
 - 2. Vertical alignment changes that cross under or over another facility, such as other utilities, drainage washes, etc
 - 3. All air release valves shall be per COP Standard Detail 317Q or 322P, as applicable for size

4.7.13 THRUST RESTRAINT

- A. Thrust blocks shall not be allowed for new construction on the City of Prescott's water system unless approved in writing by the City Engineer. Thrust restraint shall be met by:
- 1. Welded joints in steel pipelines
 - 2. Mechanical joints in concrete and Ductile Iron Pipelines
 - 3. Locking gasket and ring systems acceptable to the City Engineer
- B. All valves shall be considered dead ends for thrust restraint.

- C. COP Standard Details 303Q-1 and 303Q-2 include acceptable means of joint restraint for both horizontal and vertical deflections. Vertical slopes that exceed twenty (20%) percent require restrained joints, slopes exceeding twenty-five (25%), require additional anchoring, in addition to restrained joints to prevent pipe movement and minimize settling. The Engineer of Record shall verify the water pressures and earth bearing pressures assumed by the details. Where joint restraint is not proposed per COP Standard Details 303Q-1 and 303Q-2, the Engineer of Record shall submit joint restraint analysis with the plans for review and comments. All restrained pipe lengths shall be specified on the plans, or referenced to a COP Standard Detail.

- D. Couplings, Joints, Gaskets, and Flanges
 - 1. Couplings, joints, gaskets, and flanges shall conform to Section 610.10 of the MAG Uniform Standard Specifications or equal, unless otherwise approved.
 - a. Thrust Restraint: Joint restraint shall be used at all bends, elbows, tees, crosses, dead ends, stubs, curb stops, fire hydrants, taps, and valve locations on water mains where water flow changes direction or is stopped. The joint restraint limits shall be shown on the plan and profile view. Restrained joint analysis shall be prepared and submitted when necessary keeping in mind that concrete thrust blocks are not to be considered in the analysis.
 - b. Acceptable restrained joint systems include the following Manufactures:
 - EBA Iron Inc
 - Ford Meter Box Company Inc.
 - Romac Industries
 - Tyler Union
 - US Pipe
 - c. Ductile Iron Pipe (DIP) joint restraint shall comply with COP Standard Detail 303Q-1 and 303Q-2.

4.7.14 ELECTRONIC MARKERS

- A. Tracer wire shall be placed above all public water mains, service connections and fire lines. All splices of trace wire shall be Copperhead Snakebite or 3M DBR moisture displacement connectors.

- B. A continuity test will be performed at the completion of installation. Passing test results shall be provided for all pipe segments within the Engineer of Record's as-built data and plan set.
- C. Trace wire stations shall be required on all mains without sufficient valves and other points necessary to access the wire to maintain accurate signal strength.
- D. Maximum distance between test stations shall not exceed five-hundred (500') feet. Refer to COP Standard Detail 319Q-2.

4.7.15 PIPE COVER

- A. Water mains shall be installed to minimum and maximum depths measured from the proposed finished grade to top of pipe as follows:
 - 1. Water main shall have a minimum cover of four (4') feet
 - 2. Public water mains that are installed through undeveloped property (i.e., locations where the final finished grade elevation is not known, particularly along future street alignments), shall have a minimum cover of five (5') feet from the existing grades.
 - 3. Subsequent fills that produce a cover depth in excess of six (6') feet shall require the main to be vertically realigned within the four to six (4'-6') foot envelope
 - 4. Maximum cover over water mains shall be six (6') feet, except at vertical re-alignments necessary to avoid conflicts with other utilities and stormdrains
- B. If a water main is installed within an area to be filled at a later time, adequate pipe protection shall be provided. This may include a temporary berm or constructing the water main to a minimum cover below existing grade. The Engineer of Record shall notify the Public Works Department of such occurrences and address them in the design report or master plan and civil plans.
- C. Concrete encasement of water mains is prohibited to achieve separation from sewer.

4.7.16 WASH CROSSINGS

- A. All wash crossings will be constructed using restrained joint Class 350 (CL350) Ductile Iron Pipe (DIP). Bury requirements to place water mains under washes

or channels shall be based upon the scour depth of the one-hundred (100)-year peak design discharge (Q100) in the channel or wash. The minimum depth of bury below the design flow line of the channel or wash shall be per GES, [Table 5-7](#), unless site-specific geotechnical and hydraulic data is provided.

- B. Wash crossings with a one-hundred (100) year flow above five hundred (500) cubic feet per second (cfs), shall have the scour depth estimated using Arizona State Standard Attachment SSA 5-96, Guideline 2, Level I, as published by the Arizona Department of Water Resources. The engineer shall estimate the depth of scour and design the top of pipe to be three (3') feet below the estimated scour depth. The engineer shall provide a detailed analysis of the scour depth with final plans for review and approval.
- C. All pipelines that must be located within the scour zone or do not meet the minimum required depth of bury, as indicated above, shall be protected by installing a cut-off wall, and energy dissipation downstream of the pipeline to stabilize the scour depth to a minimum of three (3') feet above the pipeline. The engineer shall design the cut-off wall energy dissipation and include details on the improvement plans. The City Engineer shall review pipe protection and scour stabilization requirements on an individual project basis.

4.7.17 SEPARATIONS AND VERTICAL REALIGNMENTS

- A. For the protection of the public water supply from contamination the Engineer of Record shall maintain separation distances in accordance with the Arizona Department of Environmental Quality Engineering Bulletin 10, MAG Specification Section 610.5 and COP Standard Details 404P-1 and 404P-2 Water and Sanitary Sewer Separation/Protection.
- B. Vertical separation of water and sanitary sewer mains shall comply with COP Standard Detail 404P-1. Where conditions prevent adequate horizontal and vertical separation:
 - 1. Both the water and sewer main will be constructed of Class 350 (CL 350) Ductile Iron Pipe (DIP) with restrained joints or
 - 2. Where the existing water main is other than restrained ductile iron, the water mains shall be replaced with restrained DIP per COP Standard Detail 404P-2, and/or
 - 3. Where the existing or proposed sewer main is other than restrained Ductile Iron Pipe, the sewer main shall be replaced with Protecto 401 epoxy lined, restrained DIP per COP Standard Detail 404P-2

- C. Separation of water from all utilities will conform to COP Standard Detail 402P.
- D. Water mains crossing over culverts and stormdrains shall maintain both a minimum of twenty-four (24") inches vertical separation and the minimum depth of bury. If the design cannot provide these clearances, a vertical realignment may be required.
- E. Separation from Stormdrains and Culverts: Water mains shall maintain four (4') feet horizontal and two (2') feet vertical separations from stormdrains and culverts. Water mains crossing less than two (2') feet below a stormdrain or culvert shall require additional protection such as the use of restrained joint DIP or placing the water main in a casing pipe. Air release valves and isolation valves shall be installed per the following:
 - 1. Isolation valves will be installed on each side of the vertical realignment
 - 2. The Public Works Department may consider the location of adjacent valves, fire hydrants, and water service mains to help minimize valves at vertical realignments
 - 3. Install air release valves at localized high points where air entrapment or cavitation may occur
 - 4. Do not place tees, fire hydrants, service lines, and other appurtenances within any portion of the vertical realignment

4.7.18 SERVICE LINES AND METERS

- A. The water service line and meter shall be sized based upon the total daily demands for the development and the recommended maximum capacity. The minimum size service tap, saddle, line and meter set shall be one (1") inch.
- B. That portion of the water service from the water main up to, and including the brass nipple downstream of the meter is public and shall be maintained by the City of Prescott as shown in COP Standard Detail 316P. That portion of the water service beginning at the curb stop after the meter is private and shall be maintained by the property owner. Design of the private on-site portion of the water service shall comply with the current plumbing code and shall include a customer shutoff and pressure-regulating valve. Refer to COP Standard Detail 316P.
- C. Due to the City of Prescott's water billing rate structure, meter sizes shall not exceed the size of the service. (E.g. a one and one-half (1 ½") inch meter shall not be allowed on a one (1") inch service). Extra attention is required when

sizing services for custom home lots where demands occasionally necessitate meter sizes exceeding one (1") inch.

- D. Service lines are necessary to meet domestic, fire and irrigation demands. Residential fire sprinkler and irrigation demand is normally supplied through the domestic service line and meter. Commercial developments may use separate meters for building and landscape service but will provide separate lines for fire protection.
- E. Installation of metered one (1") inch to two (2") inch water services shall be in accordance with COP Standard Detail 316P. Installation of three (3") inch to six (6") inch metered services shall be in accordance with COP Standard Detail 321Q.
- F. Final plans shall show locations of service lines and meters to each unit referenced with stations and dimensions, or offsets, from the street centerline or monument line. Service lines and meter boxes shall be located within a public right-of-way or a utility easement per COP Standard Detail 316P.
- G. Service taps are not permitted on a fire line, which is designed to service private fire sprinkler systems and/or a fire hydrant.
- H. Water services connections shall be designed perpendicular to the main. Lines shall be continuous from the main to the meter with no splices. Water service lines shall have six (6') feet minimum horizontal separation from sanitary sewer service lines.
- I. No service connections or fire protection systems shall be made directly to water transmission mains.
- J. All non-copper water service lines in sizes three-quarter ($\frac{3}{4}$ ") inch through two (2") inch which are exposed during construction, shall be replaced in their entirety with one (1") inch minimum Type "K" copper tubing. This includes the replacement of service saddles, box and setter and corporation stop in all cases. Refer to COP Standard Detail 316P.
- K. The Developer shall install all water services, meter setter and meter boxes in new development projects.
 - 1. Water meter boxes shall be installed out of traffic areas and behind back of curb and sidewalk. Refer to COP Standard Detail 316P.
 - 2. Metered Services:

- a. Subdivision plans shall provide a typical water service location detail
 - b. Where water services are being designed for isolated locations, provide stationing and offset to property line
 - c. Minimum water service size is one (1") inch with a five eighths (5/8") by three-quarter (3/4") inch meter
- L. Existing water services not used will be noted on the approved civil plans and the as-built plan as abandoned at the main with full circle stainless steel repair clamp. The services shall be properly abandoned during construction.

4.7.19 BACKFLOW PREVENTION & CROSS CONNECTION CONTROL

- A. The installation of Backflow Prevention Assemblies is required for all developments with the exception of single family residential, unless the property has a groundwater well. The following types of service connections shall require backflow prevention to be installed: commercial meters, master metered systems, irrigation meters, construction meters, hydrant meters, multifamily residential meters, and fire protection connections.
- B. Residential uses are required to have a Backflow Prevention Assembly when a well, pond, submerged make up line, automatic irrigation system or fire sprinkler is present, which may raise the degree of hazard for the site.
- 1. The type of backflow assembly required is determined by the degree of hazard. Reduced Pressure Zone Assemblies (RPZA) shall be required for all commercial, multifamily residential, master metered systems, private potable water systems and irrigation meters. A testable Double Check Valve Assembly (DCVA) shall be required for all fire protection connections.
 - 2. The City of Prescott Backflow Prevention Program, Prescott City Code (PCC), Section 2-1-55, requires that Backflow Prevention Assemblies be installed according to COP Standard Details 324Q-1 and 324Q-2, as applicable for size and in accordance with the adopted plumbing code.
 - 3. All backflow prevention assemblies shall be approved by the "Foundation for Cross Connection Control and Hydraulic Research of the University of Southern California", and the City of Prescott Public Works Department.
 - 4. Initial certification testing is required upon establishment or restoration of water service to the meter. A certified backflow prevention device tester,

approved by the Public Works Department, shall make all testing, maintenance and repairs to the backflow prevention assembly at the water service customer's expense. The initial testing and the required annual testing are the responsibility of the water service customer.

5. Fire protection systems require the installation of a City of Prescott approved testable double check valve backflow prevention assembly (DCVA).
 - a. Fire lines that feed building fire sprinkler systems may install a DCVA in an NFPA approved fire riser room if the distance from the public/private interface to the riser room is less than one-hundred and fifty (150') feet. In this case, the DCVA shall be mounted vertically on fire riser and prior to any piping that consists of material not IPC approved for potable water piping. The DCVA shall be installed to allow for clearances per the adopted Fire Code.
 - b. Fire Lines that do not meet the criteria in [4.7.19.e.1](#) require that a DCVA shall be installed in a vault as close to the public/private interface as practicable but not within the utility easement or ROW. The vault shall not be installed in any drainage, retention or detention area, driveway, or parking lot entrance/exit. If the vault is located within a paved area, the vault and lid shall be traffic rated. The DCVA shall be installed in the vault to provide at least minimum clearances per the adopted Fire Code.
 - c. The location of the backflow prevention assembly shall be indicated on the approved civil plans.
- C. Every effort shall be made to locate the water meter and vault in an area that can accommodate a properly installed backflow assembly. When the location of a backflow preventer cannot be accommodated adjacent to the meter, the designer shall:
 1. Request in writing to separate the backflow assembly from the meter with a detailed explanation why the applicable standard cannot be met as well as the proposed alternative location and measures that will be taken to ensure no taps can be made between the meter and the device
 2. Backfill the water line between the meter box and the backflow preventer with 1-sack slurry Controlled Low Strength Material (CLSM) per MAG Section 728. CLSM shall be placed to the full width of the trench and to six (6") inches above top of pipe

3. Note on the plans that inspection of the water piping connecting the meter to the backflow preventer be inspected by a City of Prescott Water Protection Specialist prior to CLSM and backfill
- D. The City of Prescott requires backflow prevention on temporary hydrant meters used to supply construction water. For additional information, contact Water Operations at 928-777-1118 for “Hydrant Meter Policy”.

4.7.20 TRACT AND EASEMENT REQUIREMENTS

- A. All public water mains and appurtenances located outside of a public right-of-way or a private street tract with easement overlay shall be placed in the center of a minimum twenty (20') foot wide easement. The water main shall be located a minimum of ten (10') feet from the edge of easement. The easement shall have legal access from a public right-of-way, shall be free of obstructions, shall not be located in a fenced area, and will be accessible at all times to City of Prescott service equipment such as trucks, backhoes, etc. Water main easements located outside of paved areas shall have a twelve (12') foot wide all weather access road per COP Standard Detail 612P.
- B. The City Engineer may require additional easement width if in their opinion excessive laying depth of the pipe would require the additional width for maintenance purposes.
- C. Any re-vegetation within the easement shall consist of low growing shrubs or plant material acceptable to the City Engineer. Trees may be located along the edge of the easement but not within ten (10') feet of the water main as measured to the trunk of the tree.

SECTION 4.8/ FIRE PROTECTION

4.8.1 GENERAL INFORMATION

- A. It is the intent of the COP Fire Department to establish requirements consistent with nationally recognized practices for safeguarding life and property from hazards of fire and explosion arising from the storage, handling and use of hazardous substances, materials and devices, and from conditions hazardous to life and property arising from the use or occupancy of buildings or premises.
- B. For information related to the most currently adopted plumbing and fire codes, see the Prescott City Code (PCC) and the International Fire Code (IFC), with amendments.

- C. If the property is to be supplied with domestic service and with fire flows from a storage tank or facility, the Engineer of Record shall provide a report indicating that the required pressures and volumes exist, as required by the Fire and Public Works Departments, and are available to meet the required fire demands.
- D. Particular attention shall be given to the fire hydrant locations on final plans for infrastructure where future building locations are not identified. Final building location and elevation may necessitate the addition of water mains, fire hydrants, and/or a fire pump to serve the structure. Compliance with the fire hydrant spacing and pressure requirements are the responsibility of the party requesting a building permit.

4.8.2 FIRE FLOW REQUIREMENTS

- A. Water distribution facilities shall be sized to deliver the following minimum fire flows:
 - 1. One-thousand five-hundred (1,500) gallons, per minute (gpm) minimum for commercial, industrial, multi-family residential properties and single-family dwellings three-thousand six-hundred (3,600) square feet or greater
 - 2. One-thousand (1,000) gallons, per minute (gpm) minimum, for single-family residential properties under three-thousand six-hundred (3,600) square feet.
 - 3. Larger structures over three-thousand six hundred (3,600) square feet and non-sprinkled structures may require fire flow above one-thousand five-hundred (1,500) gpm depending on construction type.
 - 4. The fire flow requirements are subject to change. All fire flows and structural calculations as stated in the currently adopted International Fire Code shall be met.

4.8.3 HYDRANT LOCATIONS

- A. The spacing of fire hydrants is to be measured along the street or roadway in which a fire hose would be laid. This spacing is measured along the curb line and shall be inclusive of the distance along a private driveway to the proposed structure.
- B. The Fire Department will stipulate fire hydrant locations during the site planning process or during plans review. The following standards shall be used as a guide:

1. The Fire Department will stipulate fire hydrant final locations during the site planning process or during the plan review
2. Fire hydrants shall be provided along required fire apparatus access roads and adjacent public streets
3. The minimum number of fire hydrants available to a building complex or subdivision shall not be less than that determined by spacing requirements listed in the adopted Fire Code. Exception: The fire chief is authorized to accept a deficiency of up to ten (10%) percent where existing fire hydrants provide all or a portion of the required fire hydrant service
4. Existing fire hydrants on public streets may be considered for adjacent development. Existing fire hydrants on adjacent properties shall not be considered available unless fire apparatus access roads extend between properties and easements are established to prevent obstruction of such roads
5. Regardless of the average spacing, fire hydrants shall be located such that all points on streets and access roads adjacent to a building are within the distances listed in the adopted Fire Code

4.8.4 FIRE LINES AND BUILDING SPRINKLER LINES

- A. The location of on-site fire lines and taps shall be determined by the relative location of the fire department connection, riser location, emergency access, and fire hydrant locations.
- B. The size of fire lines shall be determined by the Engineer of Record for design of the project based on required fire flow demands. Fire systems shall include a backflow preventer in accordance with [4.7.19.A.2.e](#).
- C. Show all fire lines on the approved civil site plan.
- D. Fire lines shall not be connected to transmission mains.
- E. Installation of Fire Service Lines
 1. Installation of two (2") inch and smaller fire service lines shall use a saddle connection per COP Standard Detail 316P.
 2. Installation of three (3") inch and larger fire service lines use a tee and valve per COP Standard Detail 340Q-1.

3. Meters are not required on services used solely for fire sprinkler systems.
 4. All private fire sprinkler lines shall be installed with trace wire per COP Standard Detail 319Q-1.
- F. Fire service lines shall be installed perpendicular to the main within the right-of-way or easement.
- G. Excessive lengths of line, as determined by the City Engineer, may require additional backflow protection near the main to prevent stagnant water flowing into the active portions of the City's system.
- H. No service laterals or irrigation laterals shall be connected to fire lines.

4.8.5 FIRE DEPARTMENT CONNECTION

- A. If a remote Fire Department connection for a sprinkler system is required, it shall be installed between four (4') feet and eight (8') feet from the back of curb of a public or private roadway, on-site driveway or sidewalk. The location of the sprinkler system connection shall be unobstructed and readily accessible to the Fire Department. This connection shall also be within an appropriate distance of a fire hydrant as determined by the Fire Department.
- B. Fire Department connections, whether remote or wall mounted, need to be identified and coordinated on the improvement plans and on the building site plan for relationship to fire lanes and fire hydrants. All Fire Department connections shall be appropriately clear of glazing and other hazards and protected from vehicular damage.
- C. Pavement markers for Fire Department sprinkler system connections shall be provide per the International Fire Code (IFC).

4.8.6 AUXILIARY STORAGE TANKS

Minimum water pressures and discharge flow shall be in accordance with the adopted Fire Code. A fire pump package installation may be required when the building's construction type, occupancy fire load commodities classification, volumetric building areas, building height and individual square footage areas per floor level produce a pressurized fire flow supply in excess of the water transmission mains capabilities.

ARTICLE 5/ WASTEWATER

SECTION 5.1/ INTRODUCTION

5.1.1 PURPOSE

- A. The purpose of this Article is to provide engineering standards, guidelines, and geometric requirements for the design and construction of the public wastewater collection system and private sewer systems that discharge into the public collection system within the City of Prescott. This Article is complimentary and supplemental to the Prescott City Code (PCC): Title 2, Chapter 1/ Public Works Department, Title 10/ Land Development Code (LDC), and Title 16/ Street and Utility and Drainage Requirements, and all other Articles of the General Engineering Standards (GES). An electronic copy of the Prescott City Code (PCC) can be found on the City of Prescott website at www.prescott-az.gov.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

5.1.2 APPLICABILITY

- A. The standards and guidelines of this Article shall apply to the construction and modification of the public wastewater collection system and any private sewer system that discharges into the public collection system. This Article is intended for use in plan design, plan preparation, and the plan review process. The information provided in this Article is not intended to cover all situations that arise, nor may it be a substitute for sound engineering principles and judgment.
- B. Developers are required to install all improvements necessary to provide wastewater service to their development. This includes any sanitary sewer main, lift station, force main, manhole, vault or other facilities and all required appurtenances, including the payment of all applicable development fees.
- C. Developers shall also adhere to the City of Prescott's standards for extension of the City of Prescott's wastewater system to newly developed areas, subdivisions, and in-fill areas inside the City of Prescott's service area in accordance with the City of Prescott's Wastewater Collection Model.

5.1.3 DESIGN STANDARDS AND GUIDELINES

- A. All public sanitary sewer systems are to be of a gravity flow design, unless other factors dictate the use of a force main and lift station. For lift station and force main requirements, refer to GES, [Section 5.7.2](#) (Wastewater Lift Station) and

GES, [Section 5.7.3](#) (Force Main). Developments that require the construction of lift station facilities and force mains shall address the facilities compatibility with the City of Prescott's Wastewater Collection Model.

- B. A civil engineer registered in the State of Arizona shall analyze, design and seal all proposed wastewater system project plans, reports and specifications that are determined by the City Engineer to have an impact on the wastewater collection system. The effects of peak flow shall be examined to ensure proper sizing and layout of proposed wastewater system facilities, including impacts to existing off-site facilities.

5.1.4 FEDERAL, STATE AND COUNTY REGULATIONS

A. Environmental Protection Agency (EPA) Regulations

1. The U.S. Environmental Protection Agency (EPA) requires the City of Prescott to develop and implement a program to control discharges that might harm the Publicly Owned Treatment Works (POTW). The program establishes local discharge limits for non-residential users, and provides a permitting process based on the users' discharges and types of businesses.
2. The Environmental Protection Agency (EPA) wastewater flow report (*EPA 625/R-00/0008 – Chapter 3*), states “great care shall be exercised in predicting wastewater characteristics so as not to accumulate multiple factors of safety that would yield unreasonably high design flows...”. Further, “...actual measurement of wastewater flows and quality from a residential dwelling or nonresidential establishment always provides the most accurate estimate for sizing and designing...”. Therefore, the priority of obtaining water and wastewater design flows is 1) flow measurements from the actual facility, 2) the use of flows from similar facilities in the immediate area, 3) the use of similar flows from similar facilities in other areas and, 4) flow tables.

B. Arizona Department of Environmental Quality (ADEQ)

Engineering Bulletin No. 11, "Minimum Requirements for Design, Submission of Plans and Specifications of Sewage Works" published by the Arizona Department of Environmental Quality (ADEQ), and Arizona Administrative Code (AAC), "Title18 - Environmental Quality" (R18-9), contain specific requirements for submittals, approvals, and notifications when extension of a public sanitary sewer main is proposed. Additional information can be found on the ADEQ website at www.adeq.gov/function/permits/index.html.

1. Before a permit is issued for the construction of civil infrastructure, the developer shall submit to the City Engineer, Construction Authorization from ADEQ.
 2. Before the City Engineer will issue an Approval to Operate (ATO) and Release from the City of Prescott, the developer shall deliver to the City Engineer, a signed Discharge Authorization from ADEQ.
- C. Yavapai County Environmental Services Department (YCESD)
1. Before a permit is issued for the construction of civil infrastructure, the developer shall submit to the City Engineer, Construction Authorization from ADEQ through the YCESD delegation.
 2. Before the City Engineer will issue an Approval to Operate (ATO) and release from the City of Prescott, the developer shall deliver to the City Engineer, a signed Discharge Authorization from ADEQ through the YCESD delegation.
- D. City of Prescott (COP)
1. Issuance of an Approval to Construct by YCESD or ADEQ does not constitute an approval of plans and reports by the City of Prescott. No construction shall commence prior to the City of Prescott City Engineer issuing a permit.
 2. An as-built submittal is required prior to receiving an Approval to Operate and Release from the City of Prescott.

5.2/ GENERAL INFORMATION

5.2.1 AVAILABILITY OF PUBLIC CITY SEWER

- A. The Engineer of Record shall submit the required forms for ADEQ approval to the Public Works Department.
1. ADEQ approval is delegated to Yavapai County Environmental Services. Refer to Yavapai County website www.azdeq.gov for further information and applicable forms.
- B. Average daily design flows shall be used for the Treatment Capacity Assurance, while appropriate peaking factors per (GES) [Table 5-3](#) and [Table 5-4](#) shall be applied to flows for the Collection Capacity Assurance.

- C. In order to prevent the possibility of differing ADEQ and City of Prescott approved plans, the City of Prescott will **NOT** sign and release the Capacity Assurance Forms until all design and review comments have been resolved. Design and construction scheduling shall account for this requirement. Design issues relative to capacity shall be resolved prior to forms being signed.

5.2.2 PRIVATE WASTEWATER AREAS

- A. Private wastewater collection providers serve portions of Prescott's municipal service area. Placing private sanitary sewer mains within City of Prescott rights-of-way (ROW) shall require a license agreement between the private wastewater company (e.g. LLC or HOA) and the City of Prescott.
- B. The private wastewater service provider shall review modifications or construction of wastewater systems within their service areas. When submitting final plans, the developer shall provide written documentation that the private wastewater service provider has reviewed and approved the sanitary sewer mains within its jurisdiction.
- C. The City of Prescott shall review all private wastewater systems as part of the plan review process. A note shall be placed on the recorded plat or lot split survey and on the construction drawings stating the sewer collection system is private or public as well as operation and maintenance responsibilities on each specific portion of the system constructed. The plan cover sheet shall include a signature block for the party responsible for maintenance.
- D. Private wastewater system connections to the City system shall be built to City of Prescott standards for public sewer force main discharge as identified in GES, [Section 5.7.3](#). Required testing of the system shall be performed after backfill and compaction have been completed.

5.2.3 PRIVATE ON-SITE SYSTEMS/ALTERNATIVE TREATMENT FACILITIES

The City of Prescott does not issue approval for the installation of on-site treatment and disposal systems (i.e. septic or alternative systems). Approval for on-site systems, in lieu of connection to the City of Prescott public sanitary sewer system is reviewed and permitted by Yavapai County Environmental Services Department (YCESD), under Delegation Agreement from, Arizona Department of Environmental Quality (ADEQ). The City will also review the site plan. The property owner is responsible for the design, permitting, construction, operation, and maintenance of these systems in accordance with all permit conditions.

5.2.4 PRIVATE GRAVITY MAINS/LOW PRESSURE SEWER SYSTEMS

- A. The City of Prescott does not own, operate or maintain Low Pressure Sewer (LPS) Systems as these systems are considered private by the City of Prescott. For connection of an LPS to the City of Prescott system for more than one home, the language from [5.2.4.B.1](#) is to be added to the project plans, plat, CCR's and the deeds. Formation of a Limited Liability Company (LLC), or other entity approved by the City of Prescott Legal Department for perpetual maintenance responsibilities shall be required.
- B. Operation, Maintenance and Repair of Common Elements and Units
1. Operation, Maintenance and Repair of the Private Sewer System. The individual sewer customers are served by private sewer collection systems, located as shown on the final as-built project plans, the operation and maintenance of which are the responsibility of the Wastewater Service Provider (Provider) in perpetuity. The Provider shall pay the cost of all operation, maintenance, repair and replacement, of private systems. These private systems are connected to main lines operated by the City of Prescott, as identified on the final approved plans. The City of Prescott shall have no obligation or requirement to assist or provide manpower or equipment to perform or supplement any responsibilities pertaining to said private sewer systems, all of which are those of the Provider. The Provider shall provide on a yearly basis the firm, contact name, business phone number and emergency phone number for the private system operator, who shall be certified by the Arizona Department of Environmental Quality at the required level.
 2. Identification of Private Sewer System. All sewer clean out covers associated with the private system, per COP Standard Detail 270Q, shall be clearly marked "Private Sewer" with cast lettering on the cover.
 3. Homeowner Responsibilities. Homes within the private LPS area that have sewer ejector pumps shall be owned, operated and maintained by the individual homeowner. The City of Prescott shall have no obligation or requirement to assist or provide manpower, equipment or assist the homeowner in said duties.
 4. Arizona Bluestake Responsibilities. The Provider shall, as the owner of underground facilities and as required by State law, join and maintain membership in the Arizona Bluestake Association at a membership level determined by the Bluestake Association, with all the rights and obligations set forth by law.

- C. Acceptance of Private Sewer Systems for City of Prescott Ownership, Operation, and Maintenance
 - 1. The legal entity shall not petition the City of Prescott to accept the private system or any portion thereof for ownership, operation or maintenance until the systems are reconstructed to meet City of Prescott standards in effect at the time of said petition.

5.3/ AGREEMENTS

Developers and property owners who install improvements to the public wastewater system may be eligible to request a credit, oversize or reimbursement agreement with the City of Prescott allowing for partial reimbursement of costs to design and construct those improvements. Only those system components identified in the City's then current Master Plan shall be eligible for such consideration.

5.4/ PLAN SUBMITTAL REQUIREMENTS

- A. The improvement plans submitted to the City of Prescott for approval shall adhere to professional engineering standards governed by the State of Arizona Board of Technical Registration. Improvement plans shall be complete and shall comply with City of Prescott and State Board requirements. Improvement plans that do not meet industry standards, or are incomplete, shall be returned by the City of Prescott without review comments and shall be determined to be an inadequate project submittal. Additional plan review fees may be required. Repetitive submittals of non-compliant and inadequate improvement documents may be subject to State of Arizona Board of Technical Registration review.
- B. Plans for new sewer mains and extensions of existing sewer mains shall comply with the requirements of the Prescott City Code (PCC), Article 2, Chapter 2-1-11 and 2-1-37.
- C. A civil engineer registered in the State of Arizona is required to analyze the wastewater generation from a proposed development while determining the closest and best connection point through discussion with the City Engineer. The Engineer of Record shall also determine its impact on the City of Prescott's wastewater collection system. This analysis is required from the development to the downstream system. The City of Prescott is responsible for analysis of sanitary sewer mains shown in the City of Prescott's most recently adopted Water & Wastewater Models.

5.5/ WASTEWATER SYSTEM REPORTS

5.5.1 GENERAL INFORMATION

- A. The developments wastewater master plan and design reports provide an analysis of the impact that a development will have on the City of Prescott's wastewater system. These reports are reviewed and accepted by the City Engineer.
- B. Analysis of all proposed development determined by the City of Prescott to have an impact on the wastewater system shall be performed by a civil engineer, registered in the State of Arizona. The analysis shall include the effects of peak flow to ensure proper sizing and layout of the proposed wastewater system facilities.
- C. The objectives of the Wastewater Reports are to determine the development's wastewater generation rate, analyze the hydraulics of the proposed sanitary sewer system to a point of connection within the City of Prescott's Wastewater System Model and demonstrate conformance for each phase of the development with the accepted master plan for that development. All reports submitted to the City of Prescott for review shall be prepared in accordance with the following guidelines:
 - 1. Format
 - a. The report shall be on letter-sized paper eight and one-half inches by eleven inches (8 ½" X 11")
 - b. All reports shall have a table of contents
 - c. Maps and other supporting materials larger than folded ledger size paper eleven inches by seventeen inches (11" X 17") shall be placed into sleeves as an appendix to the report
 - d. A civil engineer licensed to practice in the State of Arizona shall seal each report
 - e. The project name shall be located on the cover
 - f. The name, address, and phone number of the developer/owner and engineer shall be on the cover

- g. The original submittal date and all subsequent revision dates shall be located on the cover
 - h. Common spreadsheet formats shall be compatible with MS Excel
2. Existing Conditions
- a. Provide a site description, size, addresses, major streets, township, range and section, relationship to other developments or significant sewer features
 - b. State the existing zoning and land use
 - c. Describe the existing, topography, and landform features
 - d. Describe the existing wastewater collection system infrastructure affected by the development. Identify lift stations and downstream trunk mains that will be used to convey wastewater from the development
 - e. Reference any existing master plan(s) or design report(s), applicable from adjacent development. Include excerpts from existing plans or reports, as applicable
3. Proposed Conditions
- a. Summarize the proposed development, including project name, size and type of development. Include a site plan that indicates the layout of the proposed development with a survey one-hundred (100') feet beyond the project limits
 - b. Describe infrastructure required to convey wastewater from the development to the wastewater treatment plant. Show all proposed connection(s) to the City of Prescott's wastewater system. Show extension of sanitary sewer mains into and within the site
 - c. Summarize the ADEQ and/or City of Prescott's design standard requirements that were applied to this project
 - d. Address ownership, operation and maintenance responsibilities of the proposed wastewater system
 - e. Design Methodology

- 1) Sanitary sewer main design shall provide gravity collection and buildout flows from the ultimate service area and shall allow for future extension of service to adjacent parcels when possible
 - 2) Modeling – Identify the software used and specific assumptions in the model
 - 3) Wastewater Flows – Identify land use, population, population density, loads and peaking factors
 - 4) Collection System Network – Identify and describe mains that will be required to serve the development
 - 5) Lift Stations –
 - i. Describe temporary and/or permanent lift stations where required
 - ii. Provide design parameters, and layouts when appropriate
 - iii. Lift stations shall be designed in accordance with City standards for materials, equipment, communication and backup power
 - 6) Outfalls – Identify and describe location and size
- f. Wastewater Model and Results
- 1) Identify main sizes, slopes, and other results as obtained from the model
 - 2) Include a map of the collection system for each simulation where pipes are color coded or appropriately labeled
 - 3) Any interim areas that do not have sufficient self-cleansing velocity and may require additional maintenance activities until build out shall be clearly identified
- g. Table summarizing parcels, acreages, land use, and population
4. Mapping & Exhibits

- a. All mapping shall conform to City of Prescott adopted CADD standards and the GES Article 8
- b. The project name shall be located on each sheet
- c. The name, address, and phone number of the Engineer of Record shall be stated on each sheet
- d. The original submittal date and all subsequent revision dates shall be located on the cover
- e. All maps shall be to scale
- f. Include a vicinity map identifying the projects location with respect to major cross streets
- g. All proposed or existing on-site and off-site facilities, including, but not limited to, wastewater mains, lift stations, manholes, etc
- h. All proposed or existing street locations, parcel boundaries, and proposed lots within each parcel. Show dimension and label clearly all property lines, rights-of-way, tracts and easement lines
- i. Contour lines, shall have a maximum of two (2') foot intervals and ten (10') foot labels
- j. Graphics shall present existing utilities as dashed lines and proposed utilities as bold solid lines
- k. Screen existing topography into the background. Clearly label existing and proposed contour lines at ten (10') foot intervals. Show sufficient information to evaluate pipe cover

5.5.2 WASTEWATER MASTER PLAN REPORT CONTENT

- A. A wastewater master plan is required when phased construction is proposed or if required by the City Engineer. The Wastewater System Master plan shall show compliance with the City of Prescott's design criteria and development policies for each phase of the project and establish a skeletal system for the phased development of a master planed project.
- B. The Wastewater Master Plan Report shall specify the terms and requirements for

wastewater service to the development.

- C. All development projects shall be responsible for determining their specific wastewater discharge and shall include buildout flows from any upstream developments to ensure the system is designed properly.
- D. If the proposed development contemplates a change in the original master plan or zoning, which increases density or increases wastewater system flows different from the City of Prescott's existing Wastewater System Master Plan, then additional downstream analysis shall be required.
- E. Flows shall be calculated according to the hierarchy established in the GES, [Section 5.1.4.A.2.](#)
- F. Along with the model report, a computer disk containing all analysis and modeling shall be submitted along with the model report if the project is determined to be of sufficient complexity and compatible with EPA-NET model software.
- G. Each Model map shall show the following:
 - 1. All proposed on-site and off-site facilities including, but not limited to, lift stations, trunk mains and service connections and laterals
 - 2. Proposed street locations, parcel boundaries and proposed lots within each parcel
 - 3. Contour lines, with a maximum of two (2') foot intervals
 - 4. A separate area location map showing existing and proposed streets, as well as existing parcels surrounding the project to a distance of one (1) mile from the exterior boundaries of the project
 - 5. A scale that is sufficient to show all required information clearly
- H. The City of Prescott requires wastewater mains to be installed along the entire length of the property line frontage of the property being developed. The property line frontage is defined as that portion of a parcel of property that abuts a street, easement, or public rights-of-way. If a parcel to be developed has more than one frontage, improvements shall be installed along all frontages where future connectivity is possible.
- I. A construction phase matrix shall be included in a table format for all wastewater related infrastructure required to serve the development. The table shall include each phase, parcel, and each system component. A mark in each box shall specify when infrastructure is anticipated to be required, for each phase of the development.

- J. The masterplan report shall comply with the adopted City of Prescott Wastewater System Masterplan encompassing the respective area.

5.5.3 WASTEWATER DESIGN REPORT CONTENT

- A. All development projects shall be required to provide an analysis of the projects impact on the City of Prescott wastewater system. If the project involves a subdivision plat or lot split a preliminary design report is required with submission of the preliminary plat or lot split survey. The final sewer system design report, shall be submitted with the civil plans. The civil plans shall not be approved until the sewer system design report has been approved.
- B. The report shall determine the development's wastewater hydraulic load, and analyze the hydraulics of the proposed sanitary sewer system and its impact to the City's existing system. Additionally, commercial and industrial developments shall include both hydraulic and concentration loads. The analysis shall include the effects of the peak flow to ensure correct sizing and layout of the new wastewater facilities.
- C. Design Documentation
 - 1. Summarize the proposed development. Include a description based on aliquot parts or reference within a platted development and describe the proposed uses
 - 2. Include a narrative of which design procedures, policies and methodologies will be incorporated into the design of the proposed wastewater system
 - 3. Note compliance with the General Engineering Standards (GES) and all other applicable standards and codes on the design report
 - 4. List the title and version of any software used in the design analysis
- D. Analysis
 - 1. Base wastewater flows in accordance with GES [Section 5.6.2](#).
 - 2. Verify any variance from the stated design flows with the Public Works Department
 - 3. Identify wastewater peaking factors and peak flows

4. Scour analysis if drainage crossings are required
5. Gravity Main
6. Force Main
7. Lift Station
8. Appurtenances
9. Summary Tables
10. Wastewater modeling results with exhibits shall be included.

5.5.4 SEWER SYSTEM MODEL ANALYSIS

A. Provide a model of the planned wastewater system as delineated in this section of this manual. This modeling shall be required where new wastewater infrastructure is being added to the existing City of Prescott system. The model shall be provided in a format compatible with the City of Prescott's master model and verify flows in the City of Prescott system. Minor infill projects may not require a model at the discretion of the City Engineer.

1. Wastewater Model Requirements

The developer or developer's engineer shall create and utilize a hydraulic model to demonstrate that the proposed infrastructure of the wastewater system is adequate and satisfies the performance criteria. Hydraulic modeling information shall be provided to the City Engineer.

2. Model Software

The City of Prescott currently uses the H₂OMAP Sewer software, which is EPA-NET compatible. All modeling shall be compatible with the City of Prescott's software.

3. Hydraulic Model Data

- a. All final drawings and model data to be submitted to the City shall use the City of Prescott GES, Article 8 for coordinate system information.
- b. Attributes:

- 1) Each physical entity in the model of the proposed development shall require information and attributes as defined below
- 2) Wastewater Collection System
- 3) Gravity Mains – Diameter, material, length, Manning’s coefficient, location, connectivity with mains and manholes, upstream invert elevation, and downstream invert elevation
- 4) Manholes – Diameter, location, manhole losses (minimum one-tenth (0.1) foot drop connecting mains, drop manhole details, wastewater load, and invert elevations
- 5) Lift Stations – Layout, location, and connectivity with other entities, number of pumps, pump curve, , pump start/stop controls, rim elevation, wet well dimensions, wet well invert, outfall location and elevation
- 6) Force Mains – Diameter, material, length, Manning’s coefficient, location, connectivity and manholes, upstream invert elevation, and downstream invert elevation
- 7) Diversion Structures – Diameter, invert elevation, location, connectivity, weir height and size, control scheme, desired flow through each diversion
- 8) Outfalls – Location, connectivity, invert elevation

4. Hydraulic Evaluation

- a. New developments shall be modeled to determine if the proposed infrastructure is adequate to service the development and provide the level of service as defined by the City of Prescott’s performance criteria. Refer to [Table 5-1](#).
- b. The City Engineer will use the City of Prescott’s model to determine the available capacity in the City’s existing downstream sewer collection system to convey wastewater from the development per the design engineer’s proposal. The developer will model proposed collector mains and will be responsible to make certain that wastewater can be safely conveyed from the points of origin to the City of Prescott’s trunk mains. Off-site

improvements to the collection system and lift stations may be required for safe conveyance.

- c. Boundary conditions that represent the interface between the development and the rest of the collection system need to be identified and any deficiencies made clear.
- d. All improvements required to provide sufficient capacity for project flows shall be identified.
- e. Steady-state simulations shall be completed utilizing dry weather average day flow multiplied by the appropriate upstream population peaking factor plus ten (10%) percent Inflow and Infiltration (I&I) factor for new construction. Design documentation shall show that the collection system mains are sized adequately and that slopes are greater than the minimum slope to maintain self-cleansing velocities.

**TABLE 5-1
DESIGN CRITERIA**

| | |
|---|--|
| Gravity Mains Force Mains | 2 fps<V<10 fps 3fps<V<7 fps |
| <u>Flow Depth, d/D</u> d/D for New Sewer Mains with Diameters less than twelve (12") inches d/D for New Sewer Mains with Diameters greater than or equal to twelve (12") inches d/D for Evaluating Existing Mains in Developed Areas | =0.50 =0.75 =0.90 |
| <u>Headloss in Existing Mains</u> Gravity Mains Force Mains | Manning's N=0.013 Hazen William's C = 120 Max |
| <u>Changes in Main Size</u> When a Smaller Main Joins a Larger Main | Match Crowns (at a minimum) |
| <u>Headloss At Manholes</u> Manholes with Mains Intersecting at 90 Degrees or Greater | Provide a Minimum of 0.1' Invert Drop |

5.6/ WASTEWATER DESIGN FLOWS

5.6.1 WASTEWATER COLLECTION ANALYSIS

This section shall describe the measurement standards for gravity sewer mains that are to be used to evaluate the suitability of wastewater collection system improvements and additions.

5.6.2 WASTEWATER SYSTEM LOADS

- A. All projects shall provide a summary of the anticipated sewer loads for the project. Provide an analysis of the anticipated sewer flows. This analysis shall be based on actual flows from similar facilities or the hierarchy established in GES, [Section 5.4.1.A.2](#). If flows are unavailable from similar facilities the unit factors listed in [Table 5-2](#) shall be used. If the proposed use does not match the tables, provide an estimate of what the flows shall be from other sources and provide a justification for their use. Flows may be calculated on a sub-area basis, but provide a total flow for the entire development as well.

- B. [Table 5-2](#) lists unit loads that can be easily correlated to acreages for types of land use. The Arizona Administrative Code also provides guidelines on appropriate unit loads for different types of development that is not easily related to acreages for types of land use.

**TABLE 5-2
UNIT DAILY DESIGN FLOWS FOR SEWER**

| TYPE OF FACILITY SERVED | APPLICABLE UNIT | WATER DESIGN FLOW PER APPLICABLE UNIT GALLONS PER DAY (GPD) |
|--|--|---|
| Airport | Passenger (average daily number) Employee | 4 15 |
| Auto Wash Bar/Lounge Barber Shop | Facility Seat Chair | Per manufacturer, if consistent with this Article 30 35 |
| Beauty Parlor Bowling Alley (snack bar only) | Chair Lane | 100 75 |
| Day camp, no cooking facilities Campground, overnight, flush toilets Campground, overnight, flush toilets and shower Campground, luxury Camp, youth, summer, or seasonal | Camping Unit Camping Unit Camping Unit Person Person | 30 75 150 100-150 60 |
| Church, Without Kitchen Church, With Kitchen | Person (Maximum attendance) Person (Maximum attendance) | 5 7 |
| Country Club | Resident member Non-resident member | 100 10 |
| Dance Hall Dental Office Dog Kennel Dwellings | Patron Chair Animal, Max occupancy Person | 5 500 15 80 |
| Fire Station Hospital, AllFlows Hospital, Kitchen Waste Only Hospital, Laundry Waste Only Hotel/Motel, Without Kitchen Hotel/Motel, With Kitchen Industrial Facility, without Showers Industrial Facility, with Showers Industrial Facility, Cafeteria added | Employee Bed Bed Bed Bed (2 person) Bed(2 person) Employee Employee Employee | 45 250 25 40 50 60 25 35 5 |
| Institutions, Resident Institutions, Nursing Home | Person Person | 75 125 |

| | | |
|---|-----------------------------|---|
| Institutions, Rest Home | Person | 125 |
| Institutions, Laundry, Self Service | Wash Cycle | 50 |
| Institutions, Laundry, Commercial | Washing Machine | Per Manufacturer, if consistent with this Article |
| Office Building | Employee | 20 |
| Park (Temporary Use) | | |
| Picnic, with showers, flush toilets | Parking Space | 40 |
| Picnic, with flush toilets only | Parking Space | 20 |
| Recreational Vehicle, with no water and sewer connections | Vehicle Space | 75 |
| Recreational Vehicle, with water and sewer connections | Vehicle Space | 100 |
| Mobile Home/Trailer | Space | 250 |
| Restaurant/Cafeteria | Employee | 20 |
| With toilet, add | Customer | 7 |
| Kitchen Waste, add | Meal | 6 |
| Garbage Disposal, add | Meal | 1 |
| Cocktail Lounge, add | Customer | 2 |
| Kitchen Waste Disposal Service, add | Meal | 2 |
| Restroom, Public | Toilet | 200 |
| School, Staff and Office | Person | 20 |
| Elementary, add | Student | 15 |
| Middle and High, add | Student | 20 |
| With Gym & Showers, add | Student | 5 |
| With Cafeteria, add | Student | 3 |
| Boarding, Total Flow | Person | 100 |
| Service Station, with Toilets | First Bay | 1000 |
| | Each Additional Bay | 500 |
| Shopping Center, no Food or Laundry | Square Foot of Retail Space | 0.1 |
| Store | Employee | 20 |
| Store, Public Restroom, add | Square Foot of Retail Space | 0.1 |
| Swimming Pool, Public | Person | 10 |
| Theater, Indoor | Seat | 5 |
| Theater, Drive-In | Car Space | 10 |

5.6.3 PEAKING FACTOR

- A. The peaking factor shall be as defined in the Arizona Administrative Code Section R18-9-E301.D.1.b, and GES [Table 5-3](#).

- B. In addition to dry weather peaking factors, ten (10%) percent shall be added for wet weather inflow and infiltration (I&I).
- C. Where population estimates are not available or non-residential loads prevail, dry weather peaking factors can be calculated using:

$$PF=C(Q_{avg})^{-m}$$

Where:

- C = constant (empirical coefficient)
- Qavg = average flow rate (ft³/s, m³s)
- M = exponent

**TABLE 5-3
NON-RESIDENTIAL DRY WEATHER PEAKING FACTORS
FOR WASTEWATER COLLECTION SYSTEMS**

| Average Flow (ft ³ /s) | C | M |
|-----------------------------------|------|------|
| 0.012 < Qavg | 1.78 | 0.16 |
| 0.120 < Qavg | 1.79 | 0.15 |
| 120 < Qavg < 35.0 | 1.76 | 0.05 |

5.7/ INDIVIDUAL WASTEWATER FACILITIES

5.7.1 ON-SITE TREATMENT AND DISPOSAL FACILITIES

- A. When wastewater service is not available from the City of Prescott system, an on-site treatment and disposal system, (septic or alternative system), is required. Permitting is required through the Yavapai County Environmental Services Department (YCESD) for all on-site treatment.
- B. The property owner is responsible for the design, construction, operation and maintenance of septic systems/on-site wastewater treatment and disposal facilities. The City of Prescott shall not accept any type of on-site system for operation and maintenance, but may provide input to the YCESD regarding environmental concerns.
- C. All on-site wastewater treatment and disposal facilities shall be designed, constructed and maintained by the property owner compliant with the applicable requirements of YCESD.

5.7.2 WASTEWATER LIFT STATIONS

- A. When lift stations are contemplated, the Engineer of Record shall meet with the City Engineer to discuss development options and design requirements.
1. Site selection for wastewater lift stations shall consider accessibility, drainage patterns, visual and neighborhood impact, three-phase power availability, function and design constraints.
 2. The potential for flooding shall be considered when selecting a wastewater lift station location. The station's equipment, vaults and manholes shall be protected from damage and remain operable during a one-hundred (100) year flood.
 3. Each tract or lot used for lift station facilities shall include all weather ingress/egress, as shown on COP Standard Detail 612P.
 4. Wastewater facilities shall be located on a tract or lot dedicated to the City of Prescott (conveyed by a general warranty deed) and accompanied by a title policy accepted by the City of Prescott.
- B. Lift Station Requirements
1. Arizona Administrative Code, Title 18, Chapter 9, "Water Pollution Control," and ADEQ, Bulletin 11 contain minimum requirements for a wastewater lift station. Additional requirements specific to the City of Prescott (including but not limited to wetwell coating, SCADA, Electrical, pump preferences, etc) shall be obtained from the City Engineer before beginning design. At a minimum telemetry, pump and wet well redundancy, backup power supply, three-phase power, odor control and security perimeter walls shall be required. The site shall be large enough to fully contain all the facilities and service equipment for repairs and maintenance.
 2. A preliminary design report shall be prepared and submitted to the City Engineer for review with the preliminary plat. The preliminary report shall outline the type of equipment and controls proposed for the station, that meets the City of Prescott equipment standards.
 3. A final design report prepared by a registered professional engineer, licensed in the State of Arizona, shall accompany all lift station design drawings and specifications submitted to the City of Prescott for review with the final plat.

C. Lift Station Design

1. In addition to State requirements lift stations shall be sized per the following:
 - a. The wet well shall be sized so that the number of duty pump starts per hour does not exceed three (3) during max day flows, including Inflow & Infiltration (I&I)
2. Design for current, interim and ultimate conditions.
3. Station shall have a minimum of two (2) pumps and be capable of operating at the designed flow with any one pump out of service.
4. The individual pump capabilities shall be identical to facilitate O&M.
5. Pumps shall be submersible or flooded suction dry pit (Flygt, Fairbanks Morse or other approved equal) and shall pass a minimum three (3") inch sphere, or are grinder pumps.
6. Include valve vault with shut-off and check valves.
7. Aeration and odor control:
 - a. The Odor Control system (biofilter, biofilter with optional secondary activated carbon polishing unit, or stand alone activated carbon filter unit will be sized commensurate with hydrogen sulfide concentrations, wetwell size, pumping cycles and other factors as may be appropriate.
 - b. The unit must be in a non-corrosive container with a four (4") inch minimum static air vent or a VFD controlled centrifugal fan installed with a sound attenuation enclosure, as determined by the sizing requirements
 - c. Aeration of the wetwell shall be required where the retention time exceeds thirty-(30) minutes
 - d. A compressor with diffuser bar shall be placed in the wetwell, with the compressor sized for two (2) standard cubic feet per minute (SCFM) per one-thousand (1,000) gallons of wetwell storage

- e. The diffuser piping shall be constructed of stainless steel with coarse bubble diffusion and deflector shield
- 8. Lockable aluminum access covers shall be located directly over the pumps and shall be a minimum of twenty-four (24") inches clear of any equipment in order to remove and replace pumps. Aluminum hatches shall include non-corrosive hardware and internal safety netting.
- 9. Electrical control unit (EG Controls), shall not be located on the wetwell or adjacent manholes and shall have separate mounting pad and enclosure.
- 10. Pre-cast or cast-in-place concrete walls shall be protected against corrosion. All interior walls shall be lined using Sewer Shield 150, Sewer Guard 210S or Raven 405 and installed by a certified installer in accordance with manufacturers specifications.
- 11. Controls shall have hour meters, cycle meters, magnetic flow meter, and run and failure lights with audible alarm and rotating beacon light, hand off auto switches, and provide for alternating sequencing of pumps.
- 12. All pump rails and hardware shall be stainless steel.
- 13. A diversion by-pass line and manhole with diversion piping and valves shall be included for routine maintenance and emergency pumping.
- 14. An emergency power source shall be provided that will supply immediate service and have an automatic transfer switch.
- 15. Communication to City of Prescott's SCADA monitoring system, for operations, monitoring, and security.
- 16. Equipped with visual or audible alarms when high water levels are detected.
- 17. Level controls shall be avocado style floats, normally open.

5.7.3 FORCE MAINS

- A. Force mains shall be located within a public right-of-way, or a utility easement. The main shall be located under pavement where possible.

- B. The flow velocity in the force main shall be between three (3') feet and seven (7') feet per second (fps). Force main detention times and the potential for hydrogen sulfide generation shall be calculated for any interim phasing and full project buildout. Parallel mains of differing sizes may be required if interim/startup conditions result in insufficient discharge rates, velocities and excessive retention times.
- C. All pipe material used in design of the force mains shall have established ASTM, ANSI, AWWA and NSF standards of manufacture or seals of approval. All pipes shall be designated as pressure sanitary sewer pipe. Acceptable materials shall include epoxy lined Ductile Iron Pipe (DIP) or fused High Density Polyethylene Pipe (HDPE).
- D. Air/vacuum valve(s) and all appurtenances designed for wastewater shall be stainless steel. Air/vacuum valve(s) shall be provided on force mains at all local high points. Refer to COP Standard Detail 416Q.
- E. Cleanouts and flushing points may be required based on length, flow, and topography.
- F. Refer to COP Standard Detail 426Q-1 for details regarding discharge into a manhole from a force main.
- G. The separation between the force mains and water mains shall be six (6') feet exterior wall-to-wall vertically and six (6') feet horizontally. Where a force main crosses above, or less than six (6') feet below a water main, as identified in MAG 404-1 and MAG 404-2, both the water and sewer main shall have additional protection per AAC Title 18, Chapter 5, Section 502(C).
- H. Odor control and epoxy lining shall be required at the receiving manhole. Odor control shall be sized according to receiving manhole air volume, pumping rate, and pumping frequency. Additional odor control and epoxy lining may be required on downstream facilities. Acceptable means of odor control include bio-filtration, filtration by media such as activated carbon, Chlorine Dioxide, Permanganate, Alumina or Sulfatreat or other method approved by the City Engineer.
- I. A trace wire shall be attached to the force main as part of the installation per COP Standard Detail 319Q-1 and 319Q-2. Tracer wire shall be terminated at tracer wire stations and shall not enter any structure that can potentially have a corrosive environment.

5.7.4 FLOW METERING STRUCTURE

- A. The Public Works Department may require the installation of a flow metering structure. When required, the engineer shall design the flow metering structure as part of the projects improvements. The flow metering structure design shall include:
1. Adequate land area for the structure with access to public right-of-way
 2. Security perimeter fence for site
 3. Locking structure for appurtenances
 4. SCADA radio monitoring system
 5. Electrical control system, to include software
 6. Flow measurement and recording (magnetic or ultrasonic type)
 7. Pipe and conduit penetrations in flow metering structure shall be core drilled. Pipe penetrations shall be sealed with Link Seal
 8. Aluminum access hatches with minimum four (4') feet by six (6') feet clear opening
 9. Fiberglass reinforced polyester grating
 10. All vault interior walls, shall be lined using Sewer Shield 150, Sewer Guard 210S or Raven 405 and installed by a certified installer in accordance with manufacturers specifications

5.8/ COLLECTION SYSTEM

5.8.1 GENERAL REQUIREMENTS

- A. Property shall be abutting a sewer main for a service connection to be made, unless otherwise allowed pursuant to this section. In cases where the main is not abutting, the main shall be extended at the expense of the property owner before a connection is made.
- B. The City of Prescott requires sewer mains to be installed along the entire length of the property line frontage of that property being developed, unless constrained

by topography. The property line frontage is that portion of the property that abuts a street, public utility easement or public right-of-way. If a parcel to be developed has more than one property line frontage the City of Prescott may require a sanitary sewer main be installed along the entire length of all frontages where parcel development paving is required and future connectivity is possible.

- C. Developers shall install at their expense all on-site and off-site sewer improvements necessary to serve their developments, including but not limited to gravity mains, force mains, lift stations, and any improvements to existing City facilities, obtain permits, and meet all regulatory compliance.
- D. All building sanitary sewer service lines shall be privately owned, operated and maintained.
- E. Extension of sewer mains, which are adjacent to undeveloped parcels, shall be reviewed by the City and may be required to install sewer main stubs from the adjoining manhole(s) to facilitate the future extension of the public sewer system to serve future developments. Stubs from the manholes are to be extended to the right-of-way line and be eight (8") inches minimum in diameter and finished with a vertical clean-out riser and COP Standard Detail 270Q monument.
- F. Users who discharge non-residential wastewater shall install monitoring manholes in accordance with Prescott City Code (PCC), Chapter 2.
- G. Any sewer mains not built to City standards, shall be privately owned, operated, and maintained. Refer to GES [5.2](#) for additional requirements.

5.8.2 DESIGN POLICY

- A. Reconstruction of residential or commercial structures requires compliance with all current ordinances and design guidelines relating to sewer main extension.
- B. For developments where an existing sanitary sewer is not available, a dry sanitary sewer main shall be installed and tested in conformance with all the design requirements for a public sanitary sewer main. Use COP Standard Detail 440P-1 or 440P-2, as applicable to locate the ends of service line stubs on a dry system. Design a temporary on-site wastewater treatment system, located such that future removal and connection to the City system is possible when sanitary sewer service becomes available.
- C. Wastewater systems shall be designed to serve the ultimate population density expected in the collection basin. The design shall be in conformance with the current City of Prescott approved Wastewater System Model and take into consideration future connections. Where a wastewater collection system

extension is possible upstream of a subdivision, extend the sanitary sewer through the subdivision to the platted boundary to a point of connection that shall provide wastewater service to adjacent properties. Regulatory guidelines shall be met in order to provide adequate capacity for upstream flows without future reconstruction.

5.8.3 DESIGN STANDARDS

Wastewater collection systems shall be designed in compliance with the Arizona Administrative Code, Title 18, Chapter 9, "Water Pollution Control", Arizona Department of Environmental Quality (ADEQ), Bulletin 11 and the City of Prescott General Engineering Standards.

5.9/ MATERIALS

5.9.1 PIPE SELECTION

In selecting pipe material for sanitary sewer mains, consideration shall be given to the chemical characteristics of wastewater, especially industrial wastes. Consider velocity, the possibility of septicity, external and internal pipeline forces, and preventing infiltration, abrasion, and similar type problems.

5.9.2 MATERIALS

- A. The following types of sanitary sewer mains and force mains may be used:
1. Vitrified clay pipe (VCP), fifteen (15") inches and larger
 2. Polyvinyl Chloride (PVC) SDR35, up to fifteen (15") inches in diameter
 3. PVC (meeting ASTM F679-T1) between eighteen (18") and twenty-seven (27") inches in diameter; or
 4. Class 350 (CL350) Ductile Iron Pipe (DIP) epoxy coated and seal coated
 5. High Density Polyethylene Pipe (HDPE)
 6. Centrifugally Cast Fiberglass Reinforced Polymer Mortar (CCFRPM) pipe as manufactured by HOBAS or approved equal
- B. Pipe materials may not change between manholes, unless extra protection is required for utility separation.

- C. Trench loading conditions shall be analyzed for each pipe type used. The pipe used, including bedding requirements, shall meet City and manufacturers specifications for each condition.
- D. Pipe material used in design shall have established ASTM, ANSI or NSF standards of manufacture or seals of approval, and shall be designated for use with wastewater.

5.9.3 SYSTEM LAYOUT

- A. Public sanitary sewer mains within commercial, industrial or multi-family developments shall be located within drive aisles and a minimum of ten (10') feet from any structure, including overhead projections. The main shall also be located a minimum of five (5') feet from any parking stall. Public sanitary sewer mains shall be located within City of Prescott right-of-way or public utility easements. No private services are allowed longitudinally within a public utility easement or City of Prescott right-of-way.
- B. If the horizontal direction, slope, material, or size of the sanitary sewer main changes, a manhole shall be constructed. The horizontal angle formed between an inlet and outlet pipe shall not be less than ninety (90°) degrees.
 - 1. Curvilinear sanitary sewer mains are not permitted.
 - 2. Horizontal location of sewer mains shall be aligned parallel to property lines or street center lines to the maximum extent possible and shall not cross and re-cross the centerline
- C. The design report including water and sewer layouts shall be in accordance with the following criteria:
 - 1. Sanitary sewer mains and manholes shall be located on the alignment shown in COP Standard Details 601P thru 606P
 - 2. Sanitary sewer mains shall maintain a minimum of four (4') feet horizontal clearance to dry utilities per COP Standard Detail 402P

5.9.4 DESIGN FLOWS

- A. Design flows shall be based on [Table 5-2](#) and [5-3](#) in GES [Section 5.6.3](#). The priority of obtaining wastewater design flows is 1) flow measurements from the actual facility, 2) the use of flows from similar facilities in the immediate area, 3) the use of similar flows from similar facilities in other areas and, 4) flow tables.

- B. Peaking factors in accordance with [Table 5-4](#) shall be used, plus ten (10%) percent inflow and infiltration (I&I).

**TABLE 5-4
WASTEWATER FLOW PEAKING FACTOR**

| Upstream Population | Dry Weather Peaking Factor |
|---|--|
| 100 | 3.62 |
| 200 | 3.14 |
| 300 | 2.90 |
| 400 | 2.74 |
| 500 | 2.64 |
| 600 | 2.56 |
| 700 | 2.50 |
| 800 | 2.46 |
| 900 | 2.42 |
| 1,000 | 2.38 |
| 1,001 to 10,000 | $PF = (6.330 \times p^{-0.231}) + 1.094$ |
| 10,001 to 100,000 | $PF = (6.177 \times p^{-0.233}) + 1.128$ |
| More than 100,000 | $PF = (4.500 \times p^{-0.174}) + 1.945$ |
| PF = Dry Weather Peaking Factor p = Upstream Population | |

5.9.5 HYDRAULIC DESIGN

- A. No public sanitary sewer mains shall be less than eight (8”) inches in diameter.
- B. Sanitary sewer mains shall be designed and constructed to provide full flow velocities of not less than two and one-half (2.5) feet per second (fps), based upon Manning’s Formula, using an “n” value of thirteen thousandths (0.013).
- C. Engineer shall analyze all mains at average day and peak flow and provide velocities as close to two (2) fps as possible (average day). Any velocities under two (2) fps (average day) shall be noted as requiring additional maintenance.
- D. To prevent abrasion and erosion of the pipe material, the maximum velocity shall be below ten (10) fps at estimated peak flow. Where velocities exceed ten (10) feet per second (fps), the main shall be constructed of a pipe material that is abrasion resistant. In no case shall velocities greater than fifteen (15) fps be allowed.
- E. The sanitary sewer system shall be designed to achieve uniform flow velocities through consistent slopes between manholes.

- F. Minimum slopes shall comply with [Table 5-5](#) and shall ensure a minimum self-cleaning velocity of two and one half (2.5') fps when flowing full.

**TABLE 5-5
MINIMUM DESIGN SLOPES FOR CIRCULAR PIPES**

| Pipe Size (inches) | Minimum Slope ⁽¹⁾ (ft/ft) |
|-----------------------|---|
| 8 | 0.0050 ⁽⁴⁾ |
| 10 | 0.0025 |
| 12 | 0.0020 |
| 14 | 0.0016 |
| 15 | 0.0015 |
| 16 | 0.0014 |
| 18 | 0.0012 |
| 20 | 0.0010 |
| 21 | 0.0010 |
| 24 | 0.0008 |

Note:

- (1) Mains larger than twenty-four (24") inches shall still have a slope no less than eight ten-thousandths (0.0008)
- (2) Pipe Capacity presented based on full capacity flow
- (3) Table assumes Manning's N coefficient of thirteen-thousandths (0.013)
- (4) Minimum slope for an eight (8") inch pipe shall be five-thousandths (0.0050) ft/ft unless specifically evaluated and approved by the City Engineer

- G. Analyze junction losses and include in design report.
- H. The depth to diameter (d/D) ratio for gravity sanitary sewer mains less than twelve (12") inches in diameter shall be no greater than five tenths (0.50) in the ultimate peak flow condition. The d/D ratio for sewer mains twelve (12") inches or greater in diameter shall be no greater than seventy-five hundredths (0.75) for the ultimate peak flow condition.
- I. Each reach of pipe shall be analyzed for design flow (average day), peak flow, and full flow conditions and placed in a tabular form in the design report. The table shall include average, peak, and full flows, the corresponding velocities, flow depth, and flow depth/diameter ratios for each flow regime.

- J. The City may require that the wastewater design report include an analysis of the potential of the generation of hydrogen sulfide per EPA guidelines, and if so, the design shall provide mitigation.

5.9.6 MANHOLES AND CLEAN OUTS

- A. Manholes in City of Prescott streets shall be located in accordance with COP Standard Details 601P thru 606P. Manholes shall not be located in wheel paths, sidewalks, crosswalk crossings, valley gutter, and curb and gutter. Horizontal location of sewer mains shall be aligned parallel to property lines or street center lines to the maximum extent possible and shall not cross and re-cross the centerline.
- B. Manholes are required at all changes of grade, pipe size, pipe material or alignment and at distances not to exceed those shown below:

**TABLE 5-6
MANHOLE SPACING**

| Pipe Size (Inches) | Maximum Spacing (Feet) |
|--------------------|------------------------|
| 8 to 10 | 400 |
| 12 to 18 | 500 |
| Larger than 18 | 600 |

- C. All manhole sections and cones shall be pre-cast concrete as detailed in COP Standard Detail 420Q-1 AND 420Q-2. Manhole bases can be precast or poured in place.
- D. Manhole covers shall be in accordance with COP Standard Detail 423P-1 or 423P-2 as required, for applicable size.
- E. Manholes shall be lined or coated at the junction of a force main, at all drop manholes, on sanitary sewer mains fifteen (15") inches in diameter and larger or in other design situations where corrosive conditions are anticipated. Manholes downstream from force mains and private ejector lines shall be evaluated for hydrogen sulfide and shall be lined, if warranted. Manholes requiring linings or coatings shall be noted on the final plans.
- F. The manhole shall have a minimum one-tenth (0.10) foot drop across the trough. Inverts at junctions shall be designed to maintain the energy gradient across the junction and prevent backflow. Where pipe size increases through a manhole, pipe crowns shall be matched on each side.

- G. The difference in invert elevations between inflow and outflow mains shall not exceed two (2') feet, unless a drop connection is installed. If less than two (2') feet, COP Standard Details 427Q shall be used. Drop connections shall be in accordance with COP Standard Detail 426Q-1 AND 426Q-2 and per manufacturer recommendation.
- H. Existing manholes shall be replaced to accept new mainline connections unless a drop inlet is designed.
- I. Manholes shall be protected from storm drainage and flooding conditions. Sanitary sewer mains shall not be permitted in washes or drainage areas unless otherwise approved in writing, by the City Engineer.
 - 1. When approved by the City Engineer:
 - a. Manholes located within washes or drainage areas shall have bolted watertight covers, bases and barrels to prevent inflow in accordance with COP Standard Detail 420Q-2.
 - b. The rim elevation shall be a minimum of twelve (12") inches and a maximum of eighteen (18") inches above adjacent finish grade. Refer to COP Standard Detail 420Q-2
 - c. Provide structural protection against scour from a one-hundred (100) year storm event
 - d. The manhole shall be designed to structurally exceed the external forces acting upon the manhole from a one-hundred (100) year storm event
 - e. Manholes shall be verified as non-buoyant, or provisions provided to negate the effects of buoyancy
 - 2. Ingress/egress and maintenance roads shall be provided for all public sewer mains and appurtenances. Maintenance roads shall be in accordance with COP Standard Detail 612P
- J. Cleanouts per COP Standard Detail 441Q may be used in place of manholes at the end of mains that 1) cannot be extended, 2) are less than one-hundred fifty (150') feet in length, and 3) have less than four (4) service connections.
 - 1. Service connections are not allowed at the ends of main line cleanouts. Service connections shall be provided off the sanitary sewer main a minimum of three (3') feet downstream of the cleanout.

5.9.7 PRETREATMENT MONITORING VAULTS AND MANHOLES

- A. The City Engineer has the sole discretion of when the installation of a monitoring vault and or sample collection point will be required for non-residential discharges. The following conditions will automatically trigger monitoring point and or sample collection point requirements:
1. The projected process wastewater discharge is equal to or greater than twenty-five thousand gallons per day (25,000 gpd)
 2. The projected operation falls under Federal Categorical Classification under 40 CFR Parts 400 – 471
 3. The projected operation is otherwise classified by the City Engineer, the Arizona Department of Environmental Quality (AZDEQ), or the US Environmental Protection Agency (EPA) as a Significant Industrial User (SIU)
 4. The projected operation is classified as a Food Service Facility (FSF) or Vehicle Service Facility (VSF) by the City Engineer
 5. Any other discharger regulated under the City's pretreatment program as described in Prescott City Code (PCC), Chapter 2.
- B. Any developments that meet the criteria in [5.9.7.A.](#) are required to install a monitoring vault and sample collection point in accordance with the following provisions:
1. Monitoring vaults shall be constructed with a straight channel and no taps or bends for ten (10') feet upstream or downstream or as approved by the City Engineer.
 2. Monitoring vaults shall be located and designed for access at all times by monitoring crews and vehicles
 3. Monitoring vaults shall be installed to prevent inflow from surface runoff
- C. New developments that are classified as food service facilities (FSF), vehicle service facilities (VSF), or any other user as determined by the City Engineer that are required to install a sample collection point shall meet the following conditions:

1. The sampling point shall be of sufficient size to allow full access to an opening equal to the inside diameter of the discharge pipe.
2. The sampling point shall be accessible to City of Prescott staff during regular working hours
3. The sampling point shall be installed to prevent inflow from surface runoff
4. The sampling point shall be installed immediately downstream of the sand/oil (SO) separator, grease interceptor (GI), or interceptor and prior to any downstream connection

5.9.8 PIPE COVER AND SEPARATIONS

- A. Sanitary sewer main should be installed when possible at a depth sufficient to promote gravity drainage of wastewater from each service line and shall anticipate the lowest potential finish floor elevation for each building pad to the maximum extent possible. The depth shall include the additional height required for service tap placement at the ten (10) o'clock or two (2) o'clock position and an anticipated minimum slope of one-quarter ($\frac{1}{4}$ ") inch per foot. The service lateral shall pass below water mains and other utilities where possible and shall not steeply rise within the right-of-way.
1. In no case shall a sanitary sewer main be installed with less than four (4') feet of cover over the top of the pipe.
 2. All sanitary sewer mains and laterals shall be designed to resist superimposed live loads and backfill overburden without damage or deflection to the pipe material and without adversely affecting the hydraulic characteristics of the pipe. The engineer shall specify minimum depths of cover to be provided during the construction of roadways or other facilities affecting cover over the main and laterals.
- B. Caution shall be taken in the design and construction of the sanitary sewer mains to protect all water supplies from wastewater contamination. To minimize the potential of contamination, the Engineer of Record shall design the horizontal and vertical separation of water and sanitary sewer mains in accordance with Engineering Bulletin No. 10 "Guidelines for the Construction of Water Systems" published by the Arizona Department of Environmental Quality and the Arizona Administrative Code, Title 18, Chapter 5, "Environmental Reviews and Certification".
1. The minimum horizontal distance from a water main to a sanitary sewer main shall be six (6') feet, outside to outside. The minimum vertical

clearance shall meet COP Standard Detail 404P-1 and 404P-2. Concrete encasement, shall not be used, for extra protection.

- a. Both the water and sewer main shall be constructed of Class 350 (CL350) Ductile Iron Pipe (DIP) with restrained joints.
- C. Sanitary sewer mains shall have a minimum of ten (10') feet of horizontal clearance from any structure, including but not limited to footings, landscaping or plantings, which will obstruct maintenance access to the sewer main. Design shall consider any structural load imposed on the pipe.
- D. For information about separation from other utilities, see COP Standard Detail 402P.
- E. Sanitary sewer mains crossing less than two (2') feet below a stormdrain culvert shall require the additional protection of controlled low strength material (i.e. one sack slurry). Sanitary sewer mains crossing below large structures such as box culverts and bridges shall require casing pipe, be fully restrained, and have scour protection. Sanitary sewer mains crossing over stormdrains and culverts shall be a minimum of one (1') foot above and be adequately protected from freezing, and additional structural load that may be imposed on the sewer main.

5.9.9 WASH CROSSINGS

- A. All wash crossings shall be constructed using restrained joint epoxy coated, Class 350 (CL350) Ductile Iron Pipe (DIP) or welded HDPE. Bury requirements to place sanitary sewer mains under washes or channels shall be based upon the one-hundred (100)-year peak design discharge (Q100) in the channel or wash. The minimum depth of bury below the design flow line of the channel or wash shall be shown in [Table 5-7](#), unless site-specific geotechnical and hydraulic analyses are performed to document any reduction.

**TABLE 5-7
BURY REQUIREMENTS FOR WASHES**

| 100-Year Flow Rate | Minimum Depth of Bury |
|---------------------------|--|
| 1 to 49 cfs | 5 feet |
| 50 to 99 cfs | 6 feet |
| 100 to 499 cfs | 7 feet |
| Greater than 499 cfs | Scour depth based on scour analysis required |

- B. Wash crossings with a one-hundred (100)-year flow above five hundred (500) cubic feet per second (cfs), shall have the scour depth estimated using Arizona State Standard Attachment SSA 5-96, Guideline 2, Level I, as published by the Arizona Department of Water Resources. The engineer shall estimate the depth

of scour and design the top of pipe to be three (3') feet below the estimated scour depth. The engineer shall provide a detailed analysis of the scour depth with final plans for review and approval.

- C. All pipelines that must be located within the scour zone or do not meet the minimum required depth of bury as indicated above, shall be protected by installing a cut-off wall and energy dissipation downstream of the pipeline to stabilize the scour depth to a minimum of three (3') feet above the pipeline. The engineer shall design the cut-off wall, energy dissipation, and include details on the improvement plans. The City Engineer shall review pipe protection and scour stabilization requirements on an individual project basis.

5.9.10 GRAVITY SANITARY SEWER SERVICE CONNECTIONS

- A. The engineer shall make every effort to use existing sanitary sewer service connections that have been extended to a property through a "stub out" by previous construction. Where the use of a stub out is not feasible, the existing line shall be abandoned at the sanitary sewer main by installing a glued cap at the wye. Service connections will not be allowed directly into a manhole.
- B. Minimum diameter for a service connection is four (4") inch inside diameter (ID).
- C. All service line connections shall be installed perpendicular to the sanitary sewer main in accordance with COP Standard Detail 440P-1 and 440P-2. In addition, service line connections shall be extended to the back of the right-of-way or public utility easement. No horizontal bends in the service line will be allowed.
- D. Location
 - 1. All proposed service line connections shall be shown on the final plans with stations and dimensions or offsets, from sewer centerline.
 - a. Each lot or building shall be provided with its own individual service line unless exempt under Prescott City Code (PCC) 2-1-15.
 - b. Future lot splitting will require a separate service line connection for each lot created.
 - 2. The service line location shall be coordinated to avoid conflicts with other utilities and driveway locations. Locations shall be on the lower elevation side of the lot, whenever possible.
 - 3. A two (2') foot minimum separation between service taps is required.

4. Sanitary sewer lines shall be designed to allow the sanitary sewer service lines to pass under water mains with twenty-four (24") inches of vertical clearance to minimize potential health hazards.
 5. When minimum separation is not possible or the sanitary sewer service line passes over the water main, the sanitary sewer service line shall be Ductile Iron Pipe (DIP) for a minimum of nine (9') feet on each side of the water main. No joints are allowed within nine (9') feet of the water main.
 6. All sewer service laterals shall have a sewer backwater valve installed per the most recently adopted International Plumbing Code (IPC) and in accordance with COP Standard Detail 440P-3.
 7. Water and sewer service laterals shall not be installed in the same trench. A minimum six (6') foot horizontal separation shall be maintained.
- E. Sanitary sewer service connections shall be located a minimum of four (4') feet away from a manhole or two (2') feet from a main line cleanout.

5.9.11 PRESSURIZED SANITARY SEWER SERVICE CONNECTIONS

- A. Pressurized sanitary sewer ejector systems shall be, owned, operated and maintained, by the property owner. The Engineer of Record is responsible for the design and hydraulic analysis of the pressurized system.
- B. The sanitary sewer service constructed within the right-of-way shall be per COP Standard Detail 440P-1 or 440P-2, as applicable. Connection to the gravity service shall be at the cleanout connection located at the right-of-way or easement line.
- C. The City of Prescott does not allow extending private ejector lines across the frontages of adjacent lots or properties except where it is not feasible to extend the City main due to controlling grades. Any such ejector line shall be placed within a private easement shown on the plat or lot split survey. The developer shall extend City of Prescott main to provide frontage to the maximum extent possible.

5.9.12 GREASE, OIL AND SAND INTERCEPTORS

- A. The installation of interceptors shall be in accordance with Prescott City Code (PCC), Article 2.
- B. Grease interceptors (GIs) shall be installed for all food service facilities (FSFs). Oil and sand interceptors shall be provided for vehicle service facilities (VSFs),

laundry facilities, car washes, and other similar facilities. The Engineer of Record shall contact the City Engineer to determine if an interceptor is required, and what type and size of interceptor is best suited for the proper handling of the projected waste streams. Interceptors shall be installed to be accessible to City of Prescott Utilities Operation staff and vehicles during operating hours.

- C. Each business, restaurant or establishment shall discharge to a separate interceptor. Each interceptor shall be, shown to scale and stationed on the plans.
- D. The Engineer of Record shall coordinate with the mechanical engineer to assure the following are considered in the design of grease interceptors:
 - 1. Design is compliant with the International Plumbing Code as adopted, Prescott City Code (PCC) Title 2, and approved by the City Engineer
 - 2. Tank size is appropriate to the maximum projected flow from the establishment and anticipates a ninety (90) day maintenance schedule
 - 3. Potential to develop odors
 - 4. Separation from pedestrian areas or corridors
 - 5. Avoid locating grease interceptor in parking garages, streets and under public parking spaces
 - 6. Ease of cleanup after maintenance and pumping
 - 7. Kitchen garbage grinders, shall not be installed
 - 8. Dishwashing equipment shall not be installed as to cause temperatures in the GI to inhibit separation of fats, oil, and grease (FOG) in the interceptor
 - 9. Grease traps will only be allowed in lieu of a GI in very small FSF's
 - 10. Interceptors shall be traffic rated and constructed of steel reinforced pre-cast concrete or other approved rigid corrosion resistant material

5.9.13 TRACT AND EASEMENT REQUIREMENTS

- A. All public sanitary sewer mains and appurtenances located outside the public rights-of-way or a private street tract with easement overlay shall be placed in the center of a minimum twenty (20') foot easement. The sanitary sewer main shall be located a minimum of ten (10') feet from the edge of the easement. The

easement shall have legal access from a public right-of-way, shall be free of obstructions, shall not be located within a fenced area, and shall be accessible at all times to City of Prescott service equipment such as trucks, backhoes, etc. Sanitary sewer main easements located outside of paved areas shall have a twelve (12') foot wide all weather access road per COP Standard Detail 612P.

1. Sewer main depth of ten (10') feet or less: Minimum width shall be twenty (20') feet.
 2. Sewer main depth greater than ten (10') feet: Minimum width shall be two (2) times the depth (centered in easement).
- B. Vegetation within the easement may consist of hydroseed, grasses or other materials, required to comply, with erosion control requirements. Trees shall be located outside of the easement.

5.9.14 EASEMENT ABANDONMENT REQUIREMENTS

When a property owner or developer believes a sanitary sewer easement or portion thereof is no longer required by the City of Prescott abandonment may be requested through the City Engineer. The owner or developer shall submit the request in writing with detailed documentation identifying specifically the reasons for abandonment. The City shall make the final decision based upon its long-term needs.

5.9.15 ALTERNATIVE SANITARY SEWER SYSTEMS

Developers or property owners may request that the City Engineer consider the use of alternative wastewater systems upon the Engineer of Records determination that conventional gravity or forced sanitary sewer systems cannot provide service to the development without conflicting with other provisions of these standards.

ARTICLE 6/ TRANSPORTATION AND TRAFFIC

SECTION 6.1/ INTRODUCTION

6.1.1 PURPOSE

- A. The purpose of this Article is to provide transportation and traffic engineering standards, guidelines, and geometric requirements for the design and construction of both public and private project improvements within the City of Prescott. This Article is complimentary and supplemental to the City Municipal Code: Title 8 and Title 9, the Land Development Code and other Articles of the General Engineering Standards.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

6.1.2 APPLICABILITY

Transportation and Traffic facilities may include arterial, collector, and local streets, alleys, traffic signals, roundabouts, medians, street lighting, street signing, pavement markings, public transit, pedestrian facilities, bicycle facilities, traffic calming devices, pedestrian trails, and other traffic related facilities. The minimum requirements described herein are primarily based on safety considerations; therefore, under most circumstances, standards that provide a greater degree of safety may be used.

6.1.3 DESIGN STANDARDS AND GUIDELINES

The most recently adopted editions of the following design standards and guidelines are approved references and shall be used in conjunction with this Manual.

- A. (AASHTO) The American Association of State Highway and Transportation Officials Policies on Highway Design, Development of Bicycle Facilities, Roadside Design Guide, and Guide for the Planning, Design and Operation of Pedestrian Facilities
- B. (MUTCD) The Manual on Uniform Traffic Control Devices, prepared by the U.S. Department of transportation
- C. (COP) City of Prescott Standard Details
- D. (MAG) The Maricopa Association of Governments Uniform Standard, Specifications and Details for Public Works Construction
- E. (ADOT) Arizona Department of Transportation Manual of Approved Signs

- F. (ITE) Institute of Transportation Engineers Trip Generation
- G. Transportation Research Board Highway Capacity Manual

SECTION 6.2/ PLANNING

6.2.1 TRAFFIC IMPACT ANALYSIS

Traffic Impact Analysis (TIA) shall be required for proposed developments, or additions to existing developments, that generate more than one-hundred (100) vehicle trips during the morning or afternoon peak hour period. In addition, the City Engineer may require a TIA when changes in land use, traffic, and roadways occur, such as proposed property rezoning, when the original TIA is more than two (2) years old or where the projected traffic volumes increase by more than ten (10%) percent. The City Engineer shall make the final determination regarding TIA requirements and the applicable category. The TIA shall be in a format as described in Section 6.14 of the Prescott Land Development Code. The TIA shall be prepared under the direction of a Professional Engineer (Civil) licensed to practice in the State of Arizona qualified to prepare TIA reports. There shall be no deviation from these requirements.

6.2.2 ROADWAY CLASSIFICATION

A. The City of Prescott has six (6) street classifications relating to land development. The location and street classification is determined as part of the site planning and platting process and shall be in conformance to the most recent City of Prescott Street Classification Map available on the City of Prescott website at <http://www.cityofprescott.net/services/streets/traffic/>. The City of Prescott shall review each subdivision plat and shall specify any changes needed to conform with previously planned and approved street alignments. The City Engineer shall also specify the classification for each street involved in the subdivision plat. Refer to COP Standard Details for street typical sections. All land development shall provide for streets in accordance with Prescott City Code (PCC), the Land Development Code and any other planning documents at their normal alignments, widths and geometrics, as determined by the City Engineer.

- 1. Major Arterial (7-Lane)

Facilitate relatively long trip lengths at high (45 MPH) operating speeds with limited access to adjacent properties. Major arterials serve major centers of activity in urban areas and have the highest traffic volume corridors. These streets are often major gateways to the community.

2. Minor Arterial (5-Lane)

Provide somewhat shorter trip lengths than major arterials, interconnected with and augment major arterial routes at moderate (35-40 MPH) operating speeds, and allow somewhat greater access to adjacent properties than major arterials.

3. Commercial/Industrial Collectors

Collect, distribute, and provide direct access to commercial and industrial properties. Provide direct access to commercial properties. The commercial/industrial collector street usually has lower volumes, moderate speeds (30-35 MPH), trip lengths and minimal through traffic, but may experience a proportionally high percentage of truck traffic.

4. Residential Collector

Collect and distribute moderate amounts of traffic between arterials and local streets at relatively low (25-30 MPH) operating speeds with greater accessibility than major collectors.

5. Local Residential

Provide direct access to abutting properties. Local residential streets possess relatively low volumes, operating speeds (25 MPH), trip lengths and minimal through traffic movements. Local streets shall be arranged as to discourage their use by traffic originating outside the immediate area.

6. Rural Local Residential

Provide direct access to rural abutting properties. Shares many of the properties of local residential streets, but allows a rural option without curb and gutter and sidewalk.

6.2.3 ROADWAY RIGHT-OF-WAY REQUIREMENTS

- A. The public right-of-way requirements shall be based upon recommendations of the City of Prescott's General Plan circulation element, Street Classification Map, Airport Area Transportation Plan, and the Central Yavapai Metropolitan Planning Organization (CYMPO) 2030 plan; and the ultimate needs of the development. The dedicated right-of-way shall provide sufficient area for the installation of utilities, cut and fill of slopes, drainage, postal gang boxes, sidewalks, traffic

control devices, access management devices (medians), signs, fire hydrants, landscaping, auxiliary turn lanes, transit facilities and other facilities that may be located adjacent to street corridors. The minimum right-of-way requirements are provided in COP Standard Details 601P-606P and in [Table 6-1](#):

- B. Standard Details for street typical sections are available on the City of Prescott website at <http://www.cityofprescott.net/services/engineering/> or at the City of Prescott Public Works Department.

**TABLE 6-1
STREET RIGHT-OF-WAY REQUIREMENTS**

| Classification | Minimum Right-of-Way(1) | Parking | Lanes | COP Detail |
|-------------------------|--------------------------------|----------------|--------------|-------------------|
| Major Arterial | 120' | None | 7 (3) | 606P |
| Minor Arterial | 100' | None | 5 (3) | 605P |
| Comm./Ind. Collector | 70' (2) | None | 3 (3) | 604P |
| Residential Collector | 60' | 2 sides | 2 | 603P |
| Local Residential | 50' | 1 side | 2 | 602P |
| Rural Local Residential | 50' | None | 2 | 601P |

(1) Additional right-of-way may be required in special circumstances. These may include the need for auxiliary traffic lanes, to facilitate transit facilities, offsets and/or roadway shifts.

(2) Bike lane required.

(3) Shared turn lane/median provided

6.2.4 INTERSECTION SIGHT VISIBILITY TRIANGLE EASEMENTS

Sight visibility triangle easements shall be used as a means to limit the height of structures, vegetation, and other improvements on the portion of property and/or right-of-way immediately adjacent to intersections, alleys and driveways. Section 6.3.10 and Table 6.3.9 of the Land Development Code (LDC) sets forth the minimum required corner easements by intersection type. These minimum sight visibility triangle easements however, shall not preclude the design engineer from calculating any additional right-of-way, easements or sight visibility triangle restrictions required per COP Standard Detail 611P.

6.2.5. SUBDIVISION STREET PLANNING

- A. The planning of subdivision streets shall discourage through traffic.
 - 1. Street Location and Arrangement
 - a. Street layout shall provide for the continuation of arterial and collector streets in adjacent areas, and shall conform to a standard

Article 6/Transportation and Traffic

grid system when possible, as specified in GES, and Section 7.4.3 B of the Land Development Code (LDC). In certain cases, paved street sections are required beyond the subdivision boundary to provide connectivity to the nearest City of Prescott maintained and paved street.

- b. Other classifications shall be required to follow a circulating system as well.
- c. Certain proposed streets, as designated by the City of Prescott Street Classification Map, shall be extended to the sub-division boundary to provide future connection with adjoining un-platted lands. In general, these extensions shall not be farther apart than the maximum permitted length of a block, as specified in Section 7.4.5 A of the Land Development Code.
- d. Local streets alignments shall be so arranged as to discourage their use as through routes by traffic originating outside the immediate area.

2. Partial Street Improvements

- a. When a street is required adjacent to and parallel with two subdivision boundaries, the first subdivision to develop shall complete all required improvements. The first subdivision shall submit civil improvement plans to include, in dashed lines, the half street, which will be constructed in the future, and be responsible for the construction of full improvements, at a minimum from their boundary to centerline with the required AC structural section paved width no less than twenty-four (24') feet. Additionally they shall be required to construct sidewalk, curb and gutter along their development frontage. *(Dependant on right-of-way width, circulation requirements and length of street, the first sub divider may be required to construct beyond centerline).
- b. Half-street improvements terminating at the roadway monument or centerline shall be constructed with a thickened edge per COP Standard Detail 201Q.
- c. For all projects, a full street cross-section is required for interior streets and a complete half-street cross-section for perimeter streets if the street centerline is the project's boundary line.

- d. The half-street is to be designed to match existing construction as much as possible unless doing so is likely to create an unsatisfactory condition. If changes are needed to correct conditions on an existing half-street to construct the other half of the street, the solutions must be developed in coordination with the City Engineer on an individual project basis. The plans for the new half-street must contain sufficient information on the profile and cross-sections of the existing street to demonstrate that the new construction will match the old construction and result in a full street with proper cross-sections.

3. Subdivision Blocks

- a. Block lengths, widths and shapes of blocks shall be determined with due regard to:
 - 1) Provision of sites suitable to the type of use contemplated.
 - 2) Zoning requirements, related to lot sizes and dimensions.
 - 3) Need for convenient access, circulation, control and safety of street and pedestrian traffic.
 - 4) Limitations and opportunities of topography
 - 5) Circulation within the subdivision, and access to the community facilities
 - 6) Emergency vehicle access and circulation
 - 7) Lengths as short as practicable and the discouragement of excessive vehicular speeds, but not to exceed one-thousand two-hundred (1,200') feet, measured along the centerline of street and between intersecting street centerlines
 - 8) Cul-de-sac lengths less than one-thousand three-hundred (1,300') feet in length, as specified in Section 7.4.3 D of the Land Development Code

4. Traffic Calming

Traffic calming measures can be implemented as part of new communities, or retrofitted into existing neighborhoods using the Council adopted Traffic Calming

Policy available on the City of Prescott website at <http://www.cityofprescott.net/services/streets/traffic> or at the City of Prescott Public Works Department. If a community design calls for traffic calming elements, as required by Prescott City Code (PCC), the City Engineer shall work with the project engineer on suggestions and guidance on acceptable treatments. Traffic calming measures can vary, depending on the specific application. Example, traffic-calming measures include medians, traffic circles and roundabouts, speed humps per COP Standard Detail 210Q, raised crosswalks/intersections, chicanes, and street narrowing. The City Engineer must approve the use of traffic calming measures within the City of Prescott right-of-way.

SECTION 6.3/ HORIZONTAL ALIGNMENT

A horizontal curve is required when the angle of change in horizontal alignment is equal to or greater than five (5°) degrees. The nature of the surrounding development and topography, and the street classification will establish the factors that determine the radius of the curve for small deflection angles.

6.3.1 MINIMUM CURVE RADIUS

- A. The minimum radius of curvature is determined by the design speed or by the stopping distance unless otherwise approved by the City Engineer.
- B. Minimum Radii Based on Design Speed:

[Table 6-2](#) contains the minimum radius of curvature for each street classification with and without a super elevation of two-tenths (0.02) ft/ft as derived from Exhibit 3-40 in Chapter 3 of the AASHTO “Green Book”. Wherever possible, the radii used should be larger than that specified. If stopping sight distance conditions require a larger radius than that shown, then that larger radius becomes the minimum radius for the curve.

**TABLE 6-2
MINIMUM HORIZONTAL CURVE RADIUS**

| | Major Arterial | Minor Arterial | Com/Ind Collector | Residential Collector | Local Residential | Rural Local Residential |
|--|----------------|----------------|-------------------|-----------------------|-------------------|-------------------------|
| Minimum Radius of Horizontal Curve without Super elevation | 1000' | 600' | 450' | 320' | 205' | 135' |
| Minimum Radius of Horizontal Curve with 2% Super elevation | | 500' | 375' | 270' | 150' | 100' |
| Minimum Horizontal Curve Length | 500' | 400' | 200' | 150' | 100' | 80' |
| Stopping Sight Distance | 495' | 425' | 305' | 250' | 200' | 155' |
| Design Speed (MPH) | 55 | 50 | 40 | 35 | 30 | 25 |
| Posted Speed (MPH) | 45 | 45-35 | 35-30 | 30-25 | 25 | 25 |

C. Stopping Sight Distance

When walls, buildings, bridge piers, cut slopes, vegetation, or other obstructions are near the roadway on the inside of a curve, they can block a driver's view of the road ahead. If they are too close, the driver will not have sufficient distance along the curved roadway to stop when a hazardous condition comes into view. For design purposes, the driver's eye is assumed to be three and one-half (3½') feet above the center of the inside lane (the driving lane closest to the inside of the curve) and a hazardous condition is assumed to be an object two (2') feet high in the center of the inside lane, or most recent accepted AASHTO standards. The clear distance is measured from the center of the inside lane to the view obstruction. All designs shall ensure that sufficient sight distance exists. Refer to [Table 6-3](#) for the minimum stopping sight distances for various street classifications.

**TABLE 6-3
MINIMUM STOPPING SIGHT DISTANCE
(Various Street Classifications)**

| Street Classification | Design Speed (MPH) | Stopping Sight Distance |
|-------------------------|--------------------|-------------------------|
| Major Arterial | 55 | 495' |
| Minor Arterial | 50 | 425' |
| Comm./Ind. Collector | 40 | 305' |
| Residential Collector | 35 | 250' |
| Local Residential | 30 | 200' |
| Rural Local Residential | 25 | 155' |

6.3.2 SUPER ELEVATION IN CURVES

- A. Super elevation may be used in conjunction with the minimum radius on horizontal curves to provide improved comfort for the road user. Additionally super elevation of two (2%) percent may be used when the minimum radius cannot be provided due to circumstances beyond the control of the Engineer, such as when the general alignment cannot be changed, as determined through the GES, Article 9, process. Super elevation greater than four (4%) percent may not be used, except when approved through the GES, Article 9, process.
1. Run out and Run off: For super elevation transitions, refer to the AASHTO publication, "A Policy on Geometric Design of Highways and Streets".
 2. Stormdrain Requirement: Whenever super elevation is allowed a storm drainage system to collect the run off along the median curb shall be provided to ensure that nuisance water from the higher traveled way is not allowed to cross the lower traveled way.

6.3.3 COMPOUND CURVES

Compound curves (two curves with different radii in same direction) should be avoided. However if site conditions make the use of compound curve unavoidable, the shorter radius shall be at least two-thirds ($\frac{2}{3}$) the length of the longer radius when the shorter radius is one-thousand (1,000') feet or less. Compound curves are not permitted when design speeds require the shorter radius to be greater than one-thousand (1,000') feet.

6.3.4 SPECIAL TANGENT SECTIONS BETWEEN CURVES IN THE SAME DIRECTION

- A. On two-lane roads, tangent sections should be provided between two curves in the same direction. If the pavement cross-sections throughout the curves do not have super elevation, then the minimum lengths for tangent sections are per [Table 6-4](#).

**TABLE 6-4
TANGENT SECTIONS
(Curves in Same Direction)**

| Design Speed Roadway Type | Minimum Tangent Section |
|--|------------------------------------|
| 40 MPH Commercial/Industrial Collector | 300' |
| 35 MPH Residential Collector | 200' |
| 30 MPH Local Residential | 150' |
| 25 MPH Rural/Local Residential | 100' |

- B. If super elevation is provided in the curved portions of the roadway, then the super elevation transition lengths per AASHTO will determine the tangent lengths.
- C. The City Engineer may approve the elimination of the tangent section between curves in the same direction on an individual project basis, when justified by terrain challenges or special topographic constraint.

6.3.5 TANGENT SECTIONS BETWEEN REVERSE CURVES

- A. Tangent section should be provided between two curves that curve in the opposite direction. Abrupt reversals in alignment should be avoided when possible. The distance between reverse curves should be at least the sum of the super elevation run out length and the tangent run out lengths. The required minimum lengths for tangent sections between reverse curves without super elevation are provided in [Table 6-5](#):

**TABLE 6-5
MINIMUM TANGENT SECTIONS
(Reverse Curves)**

| Street Classification | Minimum Tangent Section |
|------------------------------|------------------------------------|
| Major Arterial | 300' |
| Minor Arterial | 300' |
| Comm./Ind. Collector | 200' |
| Residential Commercial | 200' |
| Local Residential | 100' |

- B. The City Engineer may approve the elimination of the tangent section between reverse curves on an individual project basis when justified by terrain challenges or special topographic constraint.

6.3.6 TANGENT SECTIONS APPROACHING INTERSECTIONS

A tangent section shall be provided between a street intersection and a curve unless otherwise approved by the City Engineer. The minimum tangent length is shown in [Table 6-6](#).

**TABLE 6-6
TANGENT SECTIONS AT INTERSECTIONS**

| Street Classification | Minimum Tangent Section |
|------------------------------|--------------------------------|
| Major Arterial | 200' |
| Minor Arterial | 200' |
| Comm./Ind. Collector | 150' |
| Residential Collector | 100' |
| Local Residential | 50' |
| Rural Local Residential | 50' |

SECTION 6.4/ VERTICAL ALIGNMENT

Vertical curves shall be designed to provide adequate sight distance, public safety and effective street drainage. Refer to AASHTO for sight distance requirements.

6.4.1 VERTICAL CURVES

All straight grades, which deflect by more than one (1%) percent must be joined by a parabolic vertical curve. The length shall be determined using the current AASHTO "A Policy on Geometric Design of Highways and Streets." The minimum vertical curve length for a given design speed is determined as the greater of the value in the 'Minimum Length' column in [Table 6-7](#), Minimum Vertical Curve Length or the length calculated from the 'Rate of Vertical Curvature' column by the formula:

$$L = K \times A \text{ (where: } \begin{array}{l} L = \text{minimum curve length (ft)} \\ K = \text{rate of vertical curvature (ft/\%)} \\ A = \text{algebraic difference in grades (\%)} \end{array}$$

**TABLE 6-7
VERTICAL CURVES REQUIREMENTS**

| Design Speed (mph) | Minimum Length (Ft.) | Rate of Vertical Curvature K (ft. per % grade change) (Crest) | Rate of Vertical Curvature K (ft. per % grade change) (Sag) |
|---------------------------|-----------------------------|--|--|
| 20 | 60 | 7 | 17 |
| 25 | 75 | 12 | 26 |
| 30 | 90 | 19 | 37 |
| 35 | 105 | 29 | 49 |
| 40 | 120 | 44 | 64 |
| 45 | 135 | 61 | 79 |
| 50 | 150 | 84 | 96 |
| 55 | 165 | 114 | 115 |

In all cases, every section of a street’s vertical alignment must meet passing and stopping sight distance requirements for the design speed established for the street.

6.4.2 HORIZONTAL AND VERTICAL CURVES

When horizontal and vertical curves are combined, the horizontal curve should lead or follow the vertical curve, and not be introduced near the top of a crest vertical curve or near the bottom of a sag vertical curve.

6.4.3 LONGITUDINAL STREET GRADES

The maximum longitudinal street grade requirements are per [Table 6-8](#).

**TABLE 6-8
LONGITUDINAL STREET GRADE**

| Street Classification | Maximum | Minimum |
|------------------------------|----------------|----------------|
| Major Arterial | 6% | 0.50% |
| Minor Arterial | 6% | 0.50% |
| Comm./Ind. Collector | 8% | 0.50% |
| Residential Collector | 8% | 0.50% |
| Local Residential | 12% | 0.50% |
| Rural Local Residential | 12% | 0.50% |

6.4.4 SPEED LIMITS

A. Setting Speed Limits

1. On new roadway construction, speed limits are to be set within the following range:

| | | |
|--------------------------|-------|-----|
| Rural Local Residential: | 25 | mph |
| Local Residential: | 25 | mph |
| Residential Collectors: | 25-30 | mph |
| Com./Ind. Collectors: | 30-35 | mph |
| Minor Arterials: | 35-45 | mph |
| Major Arterials: | 45 | mph |

2. A reduction in speed limit shall not normally be considered due to topographic, alignment or grade issues that can be resolved through traditional grading or construction techniques. However, with exceptional circumstances a modification may be considered by the City Engineer.

6.4.5 STREET INTERSECTION SPACING

A. Separation of street intersections shall be designed to enhance safe ingress and egress, promote traffic circulation on the roadway system, and conform to the standards noted in [Table 6-9](#). The following schedule shall serve as a guide for allowable street intersection spacing unless otherwise justified by a Traffic Impact Analysis.

1. Street Intersections Spacing on State Highways and Major Arterials:

A minimum one-half (1/2) mile spacing shall be required for all full access, signalized or roundabout controlled street intersections in all cases, except where subject to a highway access management plan or as otherwise approved by the City. Consideration for intersection spacing as close as one-quarter (1/4) mile may be considered through the GES, Article 9, process. Recorded easements for shared access, provision of acceleration/deceleration lanes, traffic signals and other right-of-way improvements, for these cuts may be required prior to approval.

2. Street Intersection Spacing on Minor Arterials:

A minimum, one-quarter (1/4) mile spacing shall be required for all full access, signalized or roundabout controlled street intersections in all cases. Consideration for intersection spacing as close as one-quarter (1/4) mile to six-hundred sixty (660') feet may be considered to provide required access through the GES, Article 9, process. Recorded easements for shared access may be required prior to approval. Adjoining parcels under common ownership may be required to share a curb cut.

3. Street Intersection Spacing on Commercial/Industrial Collectors and Residential Collectors:

Street intersection Spacing on Residential Collectors shall be spaced between one-quarter (1/4) mile and six-hundred sixty (660') feet and is subject to the approval of the City Engineer.

4. Street Intersection Spacing on Local Residential and Rural Local Residential Streets:

Street Intersection Spacing on Commercial/Industrial Collectors and Local Streets shall be spaced between six-hundred sixty (660') feet and two-hundred fifty (250') feet.

**TABLE 6-9
STREET INTERSECTION AND CURB CUT SEPARATION**

| Street Classification | Minimum Street Spacing |
|---|---------------------------------|
| State Highway & Major Arterial | ½ Mile (1/2 to ¼ mile)* |
| Minor Arterial | ¼ Mile (1/4 mile to 660' feet)* |
| Comm./Ind. And Residential Collector | ¼ Mile to 660' feet |
| Local and Rural Local Residential Streets | 660' to 250' feet |

** Consideration for reduced spacing as indicated can be made through the GES, Article 9, process.*

6.4.6 AUXILIARY TRAFFIC LANES

A. Auxiliary turning lanes permit the separation of conflicting traffic movements and remove vehicles from the flow of through traffic. Auxiliary lanes apply to both right and left turn lanes at street intersections and for deceleration lanes at mid-block driveways. The requirements for an auxiliary lane may necessitate additional rights-of-way in addition to the standard sections. Determination of the need for any auxiliary turn lane shall normally be made using the Traffic Impact Analysis process, or when determined by the City Engineer. Exceptions to the storage and transition lengths may be granted through the GES, Article 9, process, where conditions do not allow the full recommended design standard to be met.

1. General Auxiliary Lane Design Considerations
 - a. Lane width: Auxiliary lanes shall equal that of the through lanes but shall be at least ten (10') feet wide.
 - b. Lane length: The length of the auxiliary lanes for turning vehicles consists of two (2) components, entering taper and storage length.
 - 1) Entering Taper

It is common practice to use a taper rate that is between eight to one (8:1)(longitudinal: transverse) for low speed roadways classified as major collectors and lower and fifteen to one (15:1) (longitudinal: transverse) for higher speed roadways classified as minor and major arterials. When these standards cannot be met a standard taper length of ninety (90') feet for a single turn lane or and one-hundred fifty (150') feet for a dual turn lane may be considered for urban streets by the City Engineer.

2) Storage Length

At signalized intersections, the storage length needed depends on the signal cycle length, the signal phasing arrangement, and the rate of arrivals and departures of left and right-turning vehicles. The storage length is a function of the probability of occurrence of events and shall usually be based on one and one-half to two times the average number of vehicles that would store per cycle, which is predicated on the design volume. This length shall be sufficient to serve heavy surges that occur from time to time. As in the case of an un-signalized intersection, provision shall be made for storing at least two vehicles. A minimum left turn storage length of one-hundred fifty (150') feet shall be provided on streets with a design speed greater than forty-five (45) MPH or at signalized intersections, all other driveway and streets shall be sixty (60') feet. A minimum right turn, storage length of one-hundred (100') feet shall be provided on all streets with a design speed greater than forty-five (45) MPH or at signalized intersections, all other driveways and streets shall be sixty (60') feet. A maximum storage of three-hundred (300') feet is allowed for a single left turn lane.

2. Right-turn lanes

Dedicated right-turn lanes are required at all arterial intersections and may be required by the City Engineer at other intersection and driveway locations as determined by a Traffic Impact Analysis. Right-turn lanes are required on roadways where right-turning vehicles create delays or safety problems for other traffic movements. The need for a right-turn lane at a site access intersection depends on the speed of traffic on the road, the

volume of traffic turning right, and the through traffic volume in the same lane as the right-turning traffic. Right turn deceleration lanes shall be required based on the ADOT Traffic Engineering Policies, Guidelines and Procedures (PGP), Section 245 and [Table 6-10](#).

TABLE 6-10
Right-Turn Lane Warrants

| Peak Hour Traffic Volume on the Highway in Advancing Direction | Minimum Peak Hour Right-turn Traffic Volume | | | | |
|--|---|-----------------------|-----------------------|-----------------------|------------|
| | # of thru lanes per direction | | | | |
| | 1 | | 2 | | 3 |
| | < 45 MPH Posted Speed | ≥ 45 MPH Posted Speed | < 45 MPH Posted Speed | ≥ 45 MPH Posted Speed | All Speeds |
| ≤ 200 | | | | | |
| 201 - 300 | - | 30 | - | - | - |
| 301 - 400 | - | 19 | - | 55 | - |
| 401 - 500 | 85 | 14 | - | 30 | - |
| 501 - 600 | 58 | 12 | 140 | 25 | - |
| 601 - 700 | 27 | 9 | 80 | 18 | - |
| 701 - 800 | 20 | 8 | 53 | 15 | - |
| 801 - 900 | 12 | 7 | 40 | 12 | - |
| 901 - 1000 | 9 | 6 | 30 | 11 | - |
| 1001 - 1100 | 8 | 5 | 23 | 9 | 18 |
| 1101 - 1200 | 7 | 5 | 18 | 8 | 16 |
| 1201 - 1300 | 6 | 4 | 14 | 8 | 15 |
| 1301 - 1400 | 6 | 4 | 11 | 6 | 12 |
| 1400+ | 5 | 3 | 8 | 6 | 10 |

3. Left-Turn Lanes

Left-turn lanes are required at all arterials and commercial/industrial collector intersections. Left-turn lanes may also be required at street intersections on residential collectors and local residential streets based on projected left-turn volume and conflicting through volume, or other safety issues. Left-turn lanes are required on roadways where left-turning vehicles create delays or safety problems for other traffic movements. The need for a left-turn lane at a site access or intersection depends on the speed of traffic on the road, the volume of traffic turning left, and the through traffic volume in the same lane as the left-turning traffic. Left-turn lanes can be accommodated with a two-way left-turn lane on most

collector streets at uncontrolled minor intersections. Dual left turn lanes should be considered when the turning volume exceeds three-hundred (300) vehicles per hour; or physical constraints signal timing or the Traffic Impact Analysis, indicates a need as determined by the City Engineer. Left turn lanes shall be required based on the ADOT Traffic Engineering Policies, Guidelines and Procedures (PGP), Section 245 and [Table 6-11](#).

TABLE 6-11
Left-Turn Lane Warrants

| Peak Hour Traffic Volume on the Highway in Advancing Direction | Minimum Peak Hour Left-turn Traffic Volume | | | |
|--|--|-----------------------|-----------------------|-----------------------|
| | # of thru lanes per direction | | | |
| | 1 | | 2 (Undivided)* | |
| | < 45 MPH Posted Speed | ≥ 45 MPH Posted Speed | ≤ 45 MPH Posted Speed | ≥ 45 MPH Posted Speed |
| ≤ 200 | 30 | 15 | - | - |
| 201 – 300 | 12 | 12 | 40 | 30 |
| 301 – 400 | 12 | 12 | 30 | 25 |
| 401 – 500 | 12 | 12 | 25 | 18 |
| 501 – 600 | 12 | 12 | 15 | 12 |
| 601 – 1000 | 12 | 12 | 10 | 8 |
| 1000+ | 12 | 8 | 10 | 8 |

* On non-freeway divided highways, left-turn or U-turn lanes should be provided at median breaks.

4. Acceleration Lanes

Acceleration lanes allow vehicles to make the necessary change between the speed of operation on a highway or street and the lower speed on the intersecting roadway. These lane types are only required on arterials or State Highways with high speed limits. Determination of the need for an acceleration lane shall be determined by the Traffic Impact Analysis.

6.4.7 DRIVEWAY ACCESS GUIDELINES

A. Driveways shall be designed and located in such a way to provide safe ingress and egress with the fewest number while promoting efficient traffic circulation on the City of Prescott’s roadway system.

1. Driveway Spacing

Minimum driveway spacing shall conform to the standards noted in [Table 6-12](#) unless otherwise approved by the City Engineer. This minimum spacing applies to proposed site driveway separation, as well as separation from existing or planned driveways on adjacent parcels and across the street. To provide safe turning movements from driveways, on streets without raised medians, new driveways shall align with existing driveways on the opposite side of the street. Corner parcels may be required to have all access from a side street as determined by the City Engineer. A non-vehicular access easement (NVAE) may be required to restrict curb cuts to approved locations.

**TABLE 6-12
DRIVEWAY SPACING**

| Street Classification | Minimum Distance Driveway Spacing (Feet) |
|--------------------------------|---|
| Controlled Access Facility | None |
| Limited Access Facility | TBD by Traffic Impact Analysis |
| State Highway & Major Arterial | 300 |
| Minor Arterial | 200 |
| Collectors & Local Streets | As determined by TIA or City Engineer |

2. Driveway Number

[Table 6-13](#) and the following shall serve as a guide for allowable number of driveways on a site unless justified by a Traffic Impact Analysis. Safe driveway requirements per the above section shall be provided at a minimum.

- a. At least one (1) driveway per abutting street shall be allowed unless physical constraints or otherwise directed by the City Engineer.
- b. One (1) additional driveway may be allowed for a site with continuous frontage of three-hundred (300’) feet or more on a

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roadway classified as minor arterial or lower and four-hundred (400') feet or more on a major arterial or higher.

- c. Two (2) additional driveways (three (3) total) may be allowed for a site with continuous frontage of more than six-hundred (600') feet or more on a roadway classified as minor arterial or lower and eight-hundred (800') feet or more on a major arterial or higher.
- d. An additional service type driveway may be allowed for a site under the following conditions:
 - i. Continuous frontage of six-hundred (600') feet or more on a roadway classified as minor arterial or lower
 - ii. Continuous frontage of eight-hundred (800') feet or more on a major arterial or higher
 - iii. The site layout is such that the service driveway is unlikely to be used by customers of the businesses on the site, and designed as such
- e. Shared access and internal connectivity between adjacent parcels shall be encouraged. Adjoining parcels under common ownership may be required to share a curb cut.
- f. Driveway location must be evaluated with respect to the particular site layout and location. Variations may be permitted where a Traffic Impact Analysis approved by the City Engineer justifies a departure from these guidelines or an exception is granted through the GES, Article 9, process.

**TABLE 6-13
NUMBER OF DRIVEWAYS**

| Street Classification | Street Frontage (feet) | Number of Driveways |
|---------------------------------------|------------------------|--------------------------------|
| Controlled Access Facility | | None |
| Limited Access Facility | >1 Mile | TBD by Traffic Impact Analysis |
| State Highway & Major Arterial | > 800 | 3* |
| State Highway & Major Arterial | ≥ 400 to 800 | 2 |
| State Highway & Major Arterial | < 400 | 1 |
| Minor Arterial & Comm./Ind. Collector | >600 | 3* |
| Minor Arterial & Comm./Ind. Collector | ≥300 to 600 | 2 |

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| | | |
|---------------------------------------|-------|----|
| Minor Arterial & Comm./Ind. Collector | < 300 | 1 |
| Residential Collector & Local Streets | Any | 1* |

* Actual number to be determined through a Traffic Impact Analysis or by approval of the City Engineer.

3. Driveway Location Limitations

A new driveway shall not be allowed within twenty (20') feet of any commercial property line except when it is a joint use driveway, serving two abutting commercial properties. Commercial driveways shall not be allowed within seventy-five (75') feet of the right-of-way line of an intersecting collector street, two-hundred (200') feet of an intersecting Minor Arterial, and three-hundred (300') feet of an intersecting Major Arterial. Exceptions may be granted through the GES, Article 9, process.

4. Right-In-Right-Out-Design

When a right-in-right-out design is required by the City of Prescott, the design engineer shall refer to COP Standard Detail 650Q-1 and 650Q-2 for specific design requirements.

6.4.8 MEDIAN DESIGN

A. In the interest of public safety, traffic management and street aesthetics, medians are used to separate traffic flow, channelize left turns, control access and reduce conflicts.

1. Raised Median

Raised median islands shall be installed in accordance with MAG standards.

- a. Storage Lane Lengths and Tapers: Refer to GES, [Section 6.4.6](#) "Auxiliary Traffic Lanes" for minimum specific dimensions, unless the Traffic Impact Analysis results or City Engineer demonstrates longer lengths are required.
- b. Median curb type: Refer to GES, [Section 6.6.3](#) "Median Curb" for requirements.
- c. Termination: Medians shall terminate in a bull nose per MAG Standard Detail 223. Other types may be considered based on the application through the GES, Article 9, process.

- d. Full access median openings consist of right in/right out and left in/left out turning movements. Full access median openings may be required to be signalized.
- e. Partial access median openings consist of right in/right out and left ingress only while prohibiting left egress. Partial openings allow fewer traffic conflicts and create a lower potential for collisions.
- f. Spacing for median openings is measured from the center of the median opening to the center of the adjacent median opening or intersection.
- g. All medians shall be signed per the MUTCD guidelines. The beginning of each median where none exists prior, are to be signed. Median breaks in a continuous median are required to have object markers.

2. Median Openings

Raised medians on arterial streets are provided to reduce conflicts, channelize movements for safety, and improve traffic flow. It is not possible to provide an opening in the median for every driveway. Careful consideration shall be given to each request for a median opening to ensure that the safety and the intent of the median is not compromised by a proliferation of median cuts. The preference for access along arterial streets is to have full access median openings that align at not less than one-quarter (1/4) mile intervals. The design engineer shall line up full access openings in compliance with the street system planned for the arterial corridor. Full access openings shall occur at the mile, one-half (1/2) mile, and one-quarter (1/4) mile interval. The preference for access at less than one-quarter (1/4) mile spacing is to have a partial access median opening; however these openings are subject to such parameters as safety of operation, flow of traffic requirements for storage, and feasibility of geometrics. These parameters shall be addressed in a Traffic Impact Analysis when requesting a partial access opening. All median openings shall be prohibited within six-hundred sixty (660') feet of an arterial-to-arterial intersection. Full access median openings at less than one-quarter (1/4) mile intervals or within six-hundred sixty (660') feet of an arterial-to-arterial, intersection must go through the GES, Article 9, process. The applicant shall be required to submit a standard Traffic Impact Analysis that also demonstrates the following:

- a. The full access opening is spaced safely and shall not be in conflict with the planned grid system for the corridor.
- b. The full access opening shall allow for safe operation.
- c. The full access opening shall not compromise storage requirements.
- d. The full access shall not compromise safety for all other turning movements.
- e. The full access opening shall not significantly affect the flow of traffic on the arterial.
- f. The full access opening will not negatively affect accesses to future property development.

SECTION 6.5/ EASEMENTS AND DEDICATIONS

6.5.1 PUBLIC UTILITY EASEMENT (PUE)

A minimum eight (8') foot wide, Public Utility Easement (PUE) shall be located adjacent to each side of the dedicated street right-of-way. Public Utility Easements may not be located within the side or back property lines without the approval of the City Engineer. Landscaping installed in a PUE shall be of the shallow root, and non-intrusive variety, and shall be maintained by the property owner.

6.5.2 NON-VEHICULAR ACCESS EASEMENT (NVAE)

Non-Vehicular Access Easement (NVAE) limits vehicular access to a site from arterial, and collector streets. On local streets, a NVAE on private lots is required adjacent to all greenbelts and open space areas. Vehicular access shall be restricted by use of a one (1') foot NVAE, at locations other than street intersections and approved driveways. The ownership and maintenance of the NVAE remains with the property owner of the parcel from which the NVAE is granted. NVAE shall be shown on the final plat.

6.5.3 TEMPORARY CONSTRUCTION EASEMENT (TCE)

An easement placed on a parcel of land temporarily for the use of construction personnel and equipment. The easement shall be extinguished upon the completion of the construction project.

SECTION 6.6/ CURB AND GUTTER

6.6.1 VERTICAL CURBS

- A. Vertical curbs are required on all streets classified as Minor Arterial and higher, Residential Collectors and Local Residential streets at curb returns, adjacent to common areas, and other areas to restrict vehicle access.
- B. The height of all vertical curb shall be six (6”) inch unless otherwise approved by the City Engineer. Installation shall be per COP Standard Detail 220Q-1, Type “A”. Vertical curbs shall be six (6”) inch at all medians, and curbs adjacent to landscaping tracts or vertical structures (e.g. cluster mailboxes, utility cabinets, signal equipment, monument signing, etc.).

6.6.2 ROLL CURB

Roll curb per COP Standard Detail 220Q-1, Type “C” and 220Q-2 may be installed on Local Residential, Residential Collector streets, and Commercial/Industrial Collector streets.

6.6.3 MEDIAN CURB

In locations where raised medians are constructed, vertical curb and gutter is required. Median curb shall be installed per COP Standard Detail 220Q-1, Type “A” with depressed lip. With City Engineer approval roll curb may be used around medians installed in low speed, low volume streets, to facilitate truck turning movements with traffic calming projects, or where needed to maintain adequate width for emergency vehicles.

6.6.4 CURB RETURNS

- A. Vertical curb shall be used through the curb return from point of curvature (PC) to point of tangent (PT) regardless of whether the tangent curb sections are vertical or roll curb.

- B. Curb Return Radii on Streets

The radii for curb returns shall be in accordance with [Table 6-14](#): All dimensions are to back of curb.

**TABLE 6-14
CURB RETURN RADII**

| Street Classification | Intersecting Street Classification | | | | | |
|-------------------------|------------------------------------|----------------|---------------------|-----------------------|-------------------|-------------------------|
| | Major Arterial | Minor Arterial | Com./Ind. Collector | Residential Collector | Local Residential | Rural Local Residential |
| Major Arterial | TBD* | TBD* | 35' | 35' | 35' | - |
| Minor Arterial | TBD* | TBD* | 35' | 35' | 35' | - |
| Comm./Ind. Collector | 35' | 35' | 30' | 30' | 30' | - |
| Residential Collector | 35' | 35' | 30' | 30' | 30' | 25' |
| Local Residential | 35' | 35' | 30' | 30' | 25' | 25' |
| Rural Local Residential | - | - | - | 30' | 25' | 25' |

**Curb radii for Arterial streets shall be determined by speed and a TIA. In all cases, they shall be a minimum thirty-five (35') feet.*

C. Sidewalk Ramp

Sidewalk ramps shall be constructed at all curbed street intersections, at medians with pedestrian refuges, and wherever a pedestrian access route crosses a street. Sidewalk ramps shall align with the sidewalk ramps on the opposite side of the street. If a traffic signal exists or is planned, the sidewalk ramp and apron shall provide access to the pedestrian push button, per ADA requirements.

1. Directional or double sidewalk ramps per COP Standard Detail 232Q shall be installed at all arterial and collector street intersections. Where directional sidewalk ramps are required, the minimum curb return radius shall be thirty (30') feet.
2. Single sidewalk ramps per COP Standard Detail 231Q shall be installed at all local residential street intersections.
3. At T-intersections one sidewalk ramp shall be installed per COP Standard Detail 233Q-1 or 233Q-2. The ramp shall normally be placed on the right-

hand side of the TEE and align with ramps on the opposite side of the street.

4. Along safe routes to school, any crosswalk location or trail connections, additional sidewalk ramps may be required.
5. Projects that include construction improvements at existing street intersections where sidewalk ramps are located shall note whether the ramps comply with current City of Prescott standards. If the sidewalk ramps are not in compliance, they shall be removed and replaced with sidewalk ramps that meet City of Prescott standards.

SECTION 6.7/ SIDEWALKS

Installation of sidewalks shall promote and enhance pedestrian safety and the aesthetic quality of the roadway. Streets constructed to City of Prescott standards shall have sidewalks installed per COP Standard Detail 230Q. Sidewalks shall remain within the right-of-way.

6.7.1 SIDEWALK WIDTHS

A. Arterial Street:

1. Detached Sidewalk = Five (5') feet with a nine (9') foot separation (parkway) from back of curb.
2. Paved Multi-Use Path = Twelve (12') feet wide with a four (4') foot separation (parkway) from back of curb.

B. Commercial/Industrial Collector Street:

1. Detached Sidewalk = Five (5') feet wide with a four (4') foot separation (parkway) from back of curb.

C. Residential Collector and Local Residential Street:

1. Detached Sidewalk with two (2') foot gutter = Four (4') feet wide with a four (4') foot separation (parkway) from back of curb.
2. Detached Sidewalk with three (3') foot gutter = Four (4') feet wide with a three (3') foot separation (parkway) from back of curb.

NOTE: Detached sidewalks and multi-use paths shall connect to the attached sidewalk at each curb return. They shall also use a curved design with a minimum radius of three (3') feet for the connection at curb return sidewalk. Sidewalk dimensions shall not include the curb width.

6.7.2. PEDESTRIAN WAYS

Pedestrian ways shall be constructed to connect sidewalks with public and private facilities not located in the public street right-of-way. Public pedestrian ways shall be within a tract or easement for the purpose of defining the access and maintenance responsibility. The minimum width shall be six (6') feet, or a width consistent with adjacent trails, and may be used for additional purposes as approved by the City Engineer. If additional uses are approved, the minimum required width may be increased depending on the specific use.

6.7.3. MULTI-USE PATHS/TRAIL FACILITIES

Developments which impact existing or planned trails as detailed in the Prescott Bicycle and Pedestrian Master Plan, are required to provide right-of-way or other accommodation to extend the trail system through or adjacent to the development, as directed by the City Engineer. The development shall design and build appropriate and safe crossings, which shall include consideration of street classification, speed, traffic volume, trail volume, proximity to signalized intersections, and cost; and link new local pedestrian facilities to the regional system.

A. Multi-use path and trail surfaces

Multi-use path and trail surfaces should be firm, stable, and slip resistant material. Minimum design criteria for multi-use path are as follows:

1. Multi-use paths where required shall have a minimum of fifteen (15') foot public right-of-way or easement dedicated to the public when not included as part of the roadway right-of-way
2. Two-way travel standard width shall be twelve (12') feet to accommodate the use of bicycles, pedestrians and maintenance vehicles. In no case shall the travel width be less than ten (10') feet
3. One-way travel shall be a minimum six (6') feet in width
4. Maximum longitudinal grade shall match that of the roadway it parallels

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5. Minimum one (1') foot graded area adjacent to both sides of the path
6. Minimum separation of four (4') feet from a roadway
7. Maintain a minimum vertical clearance of eight (8') feet, and keep free of protruding objects
8. Paths designated for equestrian use shall have a minimum vertical clearance of ten (10') feet and be constructed with trail surfaces suitable for that use
9. Fall protection if required, shall be a minimum height of forty-two (42") inches for pedestrian facilities and fifty-four (54") inches for facilities with bike and equestrian uses
10. Lighting shall be required for all underground pedestrian crossings

B. Trail Crossing Type and Design

Several federal resources address trail and shared use path design, which can be used, by the design Engineer as references. These include the American Association of State Highway and Transportation Officials (AASHTO) "Guide for the Development of Bicycle Facilities"; and the Federal Highway Administration (FHWA) "Equestrian Design Guidebook for Trails, Trailheads and Campgrounds. Whenever practical, a grade-separated crossing is preferred operationally over an at-grade crossing, however the City Engineer may consider at grade crossings based on a technical submittal from the design Engineer. This submittal shall either show that the grade separated crossing is unfeasible because of topography or that an at-grade crossing is operationally safe and the added expense and maintenance of the grade-separated crossing is not justified. The following summarizes the requirements of any trail crossings:

1. Trails, which consider equestrian at-grade crossings, shall use an alignment that is perpendicular to the street. The crossing shall be on a straight segment of the street, and optimally in a location where motorists might expect an intersection. Consistency in the placement and design of intersections allows all users to identify them more readily.
2. Appropriate tread surfaces at crossings are critical to equestrian safety and the use of rough textured concrete, washed concrete or rubberized railroad type inserts is preferred.

3. Provision of waiting areas is required where trails cross at-grade. These are created by expanding the width of the trail surface as it approaches the street, or forming a rectangular or fan-shaped waiting area using fencing.
4. At locations where the street exceeds eighty (80') feet curb to curb a median refuge is required to break the crossing into two (2) legs. The refuge shall be designed to allow storage of tandem bikes or equestrians without encroachment into travel lanes.
5. Traffic signals or hybrid Beacons (HAWKs) can enhance at-grade trail crossings when warranted. Deployment of such devices must not be taken lightly. They shall only be installed at locations with significant trail user volumes.

SECTION 6.8/ PAVEMENT CROSS SECTIONS

6.8.1 UNDIVIDED STREETS

Undivided streets should have a normal crown that is a two-way cross-slope with the cross section high point on the street centerline. A raised crown with a constant cross slope of two (2%) percent is required on all public streets. Inverted crown sections are not allowed, except as required at arterial or collector street intersections and alleyways. Within an arterial or collector street intersection, the cross-slope shall comply with COP Standard Detail 600Q to accommodate ride ability through the intersection.

6.8.2 DIVIDED STREETS

Divided streets should have cross-slope on each pavement section. The high point of each slope on each pavement section must occur on the edge of the pavement nearest to the median. Unusual conditions may cause cross slope requirements to vary, but normally, the desirable cross-slope is two (2%) percent. The desirable slope from top of median curb to top of median curb shall not exceed fifteen (15%) percent.

SECTION 6.9/ CUL-DE-SACS AND KNUCKLES

6.9.1 CUL-DE-SAC STREET

- A. A cul-de-sac street is a street that serves more than one (1) property owner and has only one (1) direct access to the public street system.
- B. Cul-de-sac streets shall be a maximum of one-thousand three-hundred (1,300') feet in length and terminate in a circular turnaround. Refer to COP Standard Detail 620P-1 and 620P-2 for right-of-way, street improvement requirements and dimensions and Section 7.4.3.D of the Land Development Code.

6.9.2 KNUCKLES

Knuckles are areas on the roadway expanded to provide a turn-around and additional access or lot frontage on local streets. Knuckles are required at intersections where changes in alignment are present but a centerline curve is not provided. Knuckles are permitted to improve accessibility to odd-shaped sites. The use of knuckles (except on a cul-de-sac) on other than local streets must be approved by the City Engineer. Refer to COP Standard Detail 622P-1 and 622P-2 for right-of-way, street improvements and dimensions.

SECTION 6.10/ DEAD END STREETS

Dead end streets shall be prohibited except short stubs where a street connection is necessary to serve adjacent un-platted land that will develop at a future date. Temporary turnarounds shall be required where the street stub exceeds one (1) lot or one-hundred (100') feet in length. The developer shall provide a sign at the stub declaring that the particular street shall connect with any future development.

SECTION 6.11/ MAIL BOXES

- A. Curb Side Mailboxes
 - 1. Curb side mailbox with Parkway: When an open space exists between the back of curb and the sidewalk of three (3') feet or more (parkway), the mailbox shall be installed so that the front is flush with the back of curb. The mailbox shall be a minimum of forty-two (42") inches above ground, and on a two (2") inch diameter steel or four by four (4"x4") inch wood breakaway post. Refer to COP Standard Detail **134Q-1**
 - 2. Curb side mailbox behind sidewalk: When the sidewalk starts at the back of curb, the mailbox shall be installed so that the front is a minimum of twelve (12") inches behind the back of the sidewalk, forty-two (42") inches

above ground and on a two (2") inch diameter steel or four by four (4"x4") inch wood breakaway post. Refer to COP Standard Detail **134Q-1**

3. Masonry mailboxes will not be permitted within the street right-of-way.

B. Cluster Mailboxes

Cluster style mailboxes: a style whereby mailboxes, meeting the specifications of the United States Postal Service, are assembled and grouped together in a single area of land so that they are regarded as one unit. Requirements:

1. The location of a cluster box shall be approved by the City Engineer and the local, United States Postal Service. For cluster boxes in new subdivisions, the location of the unit shall be shown on the preliminary plat.
2. No driveway access shall be constructed within ten (10') feet of the cluster mailbox.
3. Units shall not be installed within one-hundred (100') feet of a street intersection. Units shall not be installed in locations where because of the position of, shape or color it may interfere with, obstruct the view of or be confused with any traffic sign, signal or device.
4. Cluster mailboxes shall be located on property lines or on a dedicated open tract on the same side of the street designated as a "no parking" zone and in a manner as to provide sufficient sight distance for pedestrian access.
5. A sidewalk shall be constructed perpendicular to the curb adjacent to the unit providing a pedestrian connection from the unit to the curb.
6. The cluster box unit shall be located on a concrete pad and the unit shall be setback a minimum of one (1') foot from the back edge of the sidewalk to the nearest edge of the unit.
7. Cluster mailboxes units shall not be located to conflict with any public utilities, including but not limited to manholes, fire hydrants, meters, vaults and cabinets.
8. When in the opinion of the City Engineer it is necessary for pedestrian and vehicular safety, an access driveway turnout shall be required at the developer's expense, built three (3') feet outside of the face of curb.

SECTION 6.12 TRAFFIC CONTROL DEVICE SELECTION

A. Traffic control device selection is critical to the successful operation of the transportation network. Several options exist for controlling traffic at intersections but this section shall focus on the two highest levels of control, which include traffic signals and roundabouts. These are often interchangeable and the use of one over the other is not always evident. The following information provides guidance for the design Engineer on choosing one over the other. This justification procedure shall be documented in the Traffic Impact Analysis for each intersection requiring all way stop control or traffic signalization.

1. Installation Warrants

The studies required to justify the installation of a traffic signal are based on the warrants and requirements set forth in the Manual of Uniform Traffic Control Devices (MUTCD). No such warrants or requirements exist for roundabouts however, roundabouts shall be considered as an alternative to traffic signals and all way stop control whenever they are warranted.

2. Roundabout Criteria

Roundabouts shall be considered the most appropriate form of control at any intersection if three general questions can be answered positively.

- a. Will a roundabout be expected to perform better than other alternative control modes? In other words, will it reduce delay, increase capacity, reduce queue lengths or solve some other operational problem?
- b. Will a roundabout provide improved safety over other alternative control devices?
- c. Will a roundabout be expected to provide improved operational characteristics over other alternative control devices? This includes adjacent access operations and emergency vehicle operations.

3. Comparative Performance

Once it is determined a roundabout is the most appropriate form of control based on the operational criteria; the final stage is to compare the

construction cost, life-cycle cost, right-of-way considerations, “reserve capacity” (the ability to accommodate traffic growth), and constructability. If no clear disadvantage is evident with the roundabout then it shall be moved forward as the preferred traffic control device.

4. Roundabout and Traffic Signal Design

- a. Roundabout design can be performed by either a general civil engineering firm or a specialized roundabout designer, however if a general civil is used the plans shall require a final design review by a recognized roundabout expert as agreed upon by the City and paid for by the developer/engineer.
- b. Traffic signal design shall be conducted by a registered Civil or Traffic Engineer with particular expertise in traffic signal control systems.

5. Roundabout Plan Checklist

The Round-About Plan Checklist shall be used as a guide to ensure that all of the basic elements are included in the roundabout plans. Any roundabout plan that does not include each of the basic elements included on the checklist shall be returned for revision and re-submittal.

SECTION 6.13/ ON-STREET PARKING

A. Sign Spacing

Signs restricting on street parking shall normally be placed in accordance with the distances shown in [Table 6-15](#). These may be adjusted as required by the City Engineer when:

1. An assessment of the anticipated parking demand indicates a change is needed
2. The roadway vertical or horizontal alignment reduces the visibility of the signs
3. Parking is allowed in some areas and not in others to indicate the beginning and ending of no parking zones. This can be accomplished using “Begin” and “End” placards, arrows or specific wording on the signs.

This also applies when parking time limits are used, or handicap spaces are designated, in which case individual stalls shall be signed.

4. For special conditions such as hydrants, sight visibility requirements, special restrictions needed for traffic control devices. etc.

**TABLE 6-15
PARKING RESTRICTION SIGN SPACING**

| Street Classification | Min. Sign Spacing (Feet) | Max. Sign Spacing (Feet) |
|------------------------------|---------------------------------|---------------------------------|
| Major Arterial | 400 | 600 |
| Minor Arterial | 400 | 500 |
| Residential Collector | 200 | 300 |
| Com./Ind. Collector | 200 | 300 |
| Local Residential | 150 | 250 |
| Rural Local Residential | 150 | 250 |

B. Parking Restriction Guidelines

On street parking shall be restricted on those roadways with insufficient width to allow both parked vehicles and the safe, efficient movement of through traffic as shown in [Table 6-16](#).

**TABLE 6-16
PARKING RESTRICTION GUIDELINES – STREET WIDTH**

| Roadway Pavement Width (Feet) | Parking On One Side | Parking On Two Sides | No Parking Allowed |
|--------------------------------------|----------------------------|-----------------------------|---------------------------|
| ≤ 28 | | | X |
| >28 to ≤32 | X | | |
| >32 | | X | |

These parking guidelines assume bike lanes are not used.

C. Sign Type

Signs restricting on street parking shall be of the type and size in accordance with those shown in [Table 6-17](#). Alternative types and sizes shall only be approved for special circumstances as approved by the City Engineer.

**TABLE 6-17
PARKING RESTRICTIONS SIGN TYPE & SIZE**

| Street Classification | Approved Sign | Size | Placard or Arrow | Orientation to Travel Way (Degrees) |
|------------------------------|----------------------|-------------|----------------------------|--|
| Major Arterial | R8-3 | 30"x30" | N/A | 90 |
| Minor Arterial | R8-3 | 30"x30" | N/A | 90 |
| Com./Ind. Collector | R8-3 | 24"x24" | N/A | 90 |
| Com./Ind. Collector | R7-9 | 12"x18" | N/A | 90 |
| Residential Collector | R7-1 | 12"x18" | Begin & End Here to corner | 90 30 to 45 |
| Local Residential | R7-1 | 12"x18" | Begin & End Here to corner | 90 30 to 45 |
| Rural Local Residential | R7-1 | 12"x18" | Begin & End Here to corner | 90 30 to 45 |

SECTION 6.14/ BUS BENCH LOCATIONS

A. When properly located, adequately designed, and effectively enforced, bus stops can improve service and expedite general traffic flow. Decisions regarding bus stop spacing and location call for a careful analysis of passenger service requirements (demand, convenience, and safety), the type of bus service provided and the interaction of stopped buses with general traffic flow. The following guidelines provide direction for the design Engineer for the installation of bus stops in the City of Prescott. Refer to Prescott City Code (PCC): Title 8-2-17.

1. Stop Spacing

Bus stops are locations where passengers access the transit system. Bus stops must therefore be convenient to the places where passengers wish to go. Convenience and speed must be balanced in determining appropriate bus stop placement, as too many bus stops can slow down travel times and impede traffic movements. The following guidelines on bus stop spacing shall apply:

- a. In the downtown area, bus stops shall have a spacing of eight-hundred (800') feet to one-thousand three-hundred (1,300') feet.

- b. Outside the downtown area, bus stops shall be spaced no less than one-thousand five-hundred (1,500') feet apart.

2. Bus Stop Locations

Bus stops shall be placed along the street curb for direct safe passenger access to and from the sidewalk, waiting and walking areas. Bus stops are placed in one of three locations: far side (located immediately after an intersection); nearside (located immediately before an intersection); and mid-block (located between intersections). Each of these locations offers advantages to vehicle drivers and pedestrians. However, in general the following guidelines apply to their use:

- a. Far-side stops are the preferable choice for service because they reduce conflicts between right-turning vehicles and stopped buses, eliminate sight-distance deficiencies on approaches to an intersection, and encourage pedestrian crossing at the rear of the bus.
- b. Nearside stops shall be avoided whenever possible. However, they may be considered on an individual project basis if in the opinion of the City Engineer a far-side stop is deemed unsafe or impractical.
- c. Mid-block stops are considered special case stops and are to be used only in special circumstances when in the opinion of the City Engineer there is a special need based on trip attractors, transfer points or other special demands that cannot be accommodated at an intersection nearby. Bus turnouts shall be used for mid-block stops.

3. Bus Stop Benches

- a. Each bus stop location shall consist of one bench or shelter unit unless otherwise approved. Multiple benches at one stop location may only be considered when ridership records indicate a clear need for their installation.
- b. Bus benches shall be placed in a manner that sidewalk passage is not narrowed to less than the minimum widths set forth by the Americans with Disabilities Act.
- c. Bus bench placement, which would result in sidewalk widths less than thirty-six (36") inches, shall require the transit provider to

install additional sidewalk width and acquire additional right-of-way at their expense, as needed to maintain ADA required sidewalk widths.

SECTION 6.15/ TRAFFIC SIGNALS & LIGHTING

6.15.1 PUBLIC STREET LIGHTING

Developers of all residential, commercial, industrial or other types of properties are responsible for the design and installation of street lighting on all streets within and adjacent to their sites when required by the Prescott City Code (PCC). Street light plans shall be prepared and sealed by a licensed electrical engineer registered in the State of Arizona. The street lighting design shall be reviewed and approved by the City and shall take into account any policy promoting “dark sky initiatives” in recommendations on the number, spacing, and shielding of new street lighting. The street light design shall include numbering of the street light poles as directed by the City Engineer and/or Arizona Public Service.

6.15.2 STREET LIGHT SELECTION

These street lighting criteria and guidelines apply for all standard installations. Alternative street light designs using old-style posts and lamps, for example, may also be acceptable by the City for public street light systems when consistent with the Historic Globe Street Light Overlay District Map and requested by Developers and/or property owners. These alternative designs may be granted through the GES, Article 9, process.

6.15.3 STREET LIGHT DESIGN

The developer shall retain a Professional Electrical Engineer, registered in the State of Arizona, to prepare the lighting system design and appropriate calculations relative to illumination levels. Illumination design shall follow the recommendations of the American National Standard Practice for Roadway Lighting, Illuminating Engineering Society of North America, IES RP-8. The Luminance criteria, with light loss factors of point eight (0.80), shall be used to determine the compliance with the IES RP-8 and City street lighting design guidelines. High Pressure Sodium (HPS) fixtures are the standard for all installations.

6.15.4 GENERAL STREET LIGHT NOTES

- A. Street lights shall normally be located on property lines whenever possible, at least five (5') feet from driveways or any above ground facility, and at such locations to maximize their separation from trees.
- B. Streetlights shall be located on the outside edge of the curve for horizontal curvilinear street alignments.
- C. The minimum curb overhang by street light luminaries (excluding historic globe style) shall be no less than two (2') feet.
- D. Where the sidewalk is separated from the curb (parkway exists), the street light pole shall be centered two (2') feet from the gutter flow line.
- E. Where the sidewalk is contiguous to the curb and the sidewalk the street light pole shall be placed one and one-half (1 ½') feet behind the sidewalk.
- F. Street trees can significantly interfere with the efficient lighting of streets, and shall not be permitted within thirty (30') feet of any street light standard.
- G. All electrical connections and supply shall be placed underground.

6.15.5 COBRA HEAD STREET LIGHT LUMINAIRES

A. Standard Details

For the purpose of simplifying street light inventories, street light maintenance and consistency with APS requirements, the City of Prescott has adopted a pre-approved cobra head street light standard for use on all City of Prescott owned street light systems (except areas where historic globe lighting is used).

- 1. Cobra head streetlights shall be per COP Standard Detail 170P.
- 2. All luminaries shall be "Cobra" head style, grey in color.
- 3. Multi tapped 120-240-277 voltage ready.
- 4. All lamps for Cobra head street light systems shall be high-pressure sodium (HPS) lamps. These HPS lamps shall meet one of the following criteria based on the classification of the roadway and the use specified in [Table 6-18](#). One-Hundred (100) watt or ninety-five hundred (9500) minimum initial lumens, one-hundred fifty (150) watt or sixteen thousand

(16,000) initial lumens, or two-hundred fifty (250) watt or thirty-thousand (30,000) minimum initial lumens.

**TABLE 6-18
STREET LIGHT LUMINAIRE WATTAGE**

| Street Classification | Head Type (HPS) | Location (Use) | Wattage |
|------------------------------|------------------------|------------------------|----------------|
| Major Arterial | Cobra Head | Roadway & Intersection | 250 |
| Minor Arterial | Cobra Head | Roadway & Intersection | 250 to 150* |
| Com./Ind. Collector | Cobra Head | Intersection | 250 to 150* |
| Com./Ind. Collector | Cobra Head | Roadway | 250 to 150* |
| Residential Collector | Cobra Head | Intersection | 150 or 100* |
| Residential Collector | Cobra Head | Roadway | 150 or 100 |
| Local Residential | Cobra Head | Roadway & Intersection | 100 |
| Local Residential | Historic Globe | Roadway | 70 |
| Rural Local Residential | Cobra Head | Roadway & Intersection | 100 |

* Specific use shall be directed by the City Engineer based on local conditions.

B. Location and spacing

Lighting Layout: The street lighting layout required is dictated by the street classification and shall be continuous. The design engineer shall consult the City Engineer to assist in determining the appropriate street classification. The spacing of streetlights shall conform to the criteria below.

1. Arterial Streets: Lights shall be required at an average spacing of one-hundred fifty (150') feet (staggered) with spacing of three-hundred (300') feet on each side.
2. Commercial/Industrial and Residential Collector Streets: Lights shall be required at an average spacing of two-hundred (200') feet. Light may be placed on one side of the street or staggered.
3. Local Streets: Lights shall be required at or near the end of cul-de-sacs longer than one-hundred forty (140') feet measured from a typical Stop sign location, at changes of horizontal street alignment of forty-five (45°) degrees or more that are two-hundred (200') feet or more from a street intersection. If optional mid-block locations are used then the spacing of lights is about two-hundred seventy-five (275') feet, but never less than two-hundred feet (200') or more than three-hundred fifty (350') feet.
4. Intersection Lighting: All arterial-to-arterial and signalized intersections shall have four (4) streetlights. All other intersections require two (2) streetlights except Local to Local and in some cases, as determined by

the City, Residential Collector to Local, which shall have one (1) streetlight. Knuckles require one (1) streetlight.

C. Historic Globe Street Light Luminaries

1. Standard Details

- a. Historic streetlights shall be Sternberg 4200 Augusta Series – 4208 TFP4 (Ten (10') feet tall, four (4") inch tapered Fluted cast aluminum-extruded poles) or approved equivalent.
- b. Multi tapped 120-240 voltage ready.
- c. All lamps for historic globe street light systems shall be seventy (70)-watt HPS incandescent lamps.
- d. Eight (8') foot poles are 4208 TFP5 – Verde Green; Globe – A840, seventy (70)-watt HPS.
- e. Ten (10') foot poles are 5210 TFP – RE5Q and Alzak FH/DBA/Black; Globe – B750, seventy (70)-watt HPS-MT.

2. Location and Spacing

- a. Lights shall be placed on alternating sides of the roadway at approximately seventy-five (75') foot intervals.
- b. Lights shall be used in designated historic districts only as defined by the Historic Globe Street Light Overlay District Map.
- c. Light spacing shall be such that at least one (1) streetlight is placed in close proximity of any roadway intersection (within five (5') feet of the back of curb return or edge of curb return). At larger intersections with collector-to-collector or arterial to collector street classifications, two (2) lights shall be placed diagonally opposite to one another (within five (5') feet of the back of curb return or edge of curb return).
- d. In some cases, cobra head streetlights may be required at intersections requiring higher levels of safety lighting.

6.15.6 TRAFFIC SIGNALS

This section shall present the criteria and procedures to be utilized by consultants when performing traffic signal work in and for the City of Prescott.

A. Traffic Signal Spacing

Street Intersection Spacing on State Highways and Major Arterials:

1. Minimum one-half (1/2) mile spacing shall be required for all warranted traffic signal intersections on all state highways, parkways and major arterial in all cases, except where subject to a highway access management plan or as otherwise approved by ADOT. Consideration for intersection spacing as close as one-quarter (1/4) mile may be considered through the GES, Article 9, process. Recorded easements for shared access, provision of acceleration/deceleration lanes, traffic signals and other right-of-way improvements, for these cuts may be required prior to approval.
2. Minimum one-quarter (1/4) mile spacing shall be required for all warranted traffic signal intersections on Minor Arterials, Major Collectors, and Minor Collectors in all cases. Consideration for intersection spacing as close as one-quarter (1/4) mile to six-hundred sixty (660') feet may be considered to provide required access through the GES, Article 9, process. Recorded easements for shared access may be required prior to approval. Adjoining parcels under common ownership may be required to share a curb cut.

B. Traffic Signal Design Guidelines

The following publication most recent editions and approved supplements by the State and City of Prescott are to be used in conjunction with the design criteria when designing traffic signals for the City of Prescott.

1. Manual on Uniform Traffic Control Devices for Street and Highways; USDOT/FHWA
2. Traffic Signals and Lighting (Standard Drawings); ADOT
3. ADOT Standard Specifications for Road and Bridge Construction
4. International Signal Association Specifications
5. Equipment and Materials Standards of the Institute of Transportation Engineers

C. Controllers and Cabinets

1. Control cabinet shall be type IV Econolite TS2, Type 1 with elevator base.
2. Meter Pedestal Cabinet shall be Myers PBM 2000 or 1250 UPS w/foundation or approved equal.
3. Provision for battery backup shall be provided in all traffic signal cabinets.

D. Emergency Vehicle Preemption

Emergency vehicle preemption shall be installed at all traffic signal intersections. A complete installation to include Opticom 700 series system, sensors, processors and confirmation hardware or approved equal shall be used.

E. Loop Detectors

Loop detector shall be six (6') feet by fifty (50') feet quadrupole unless otherwise specified by the City Engineer and shall conform to the current ADOT Traffic Signals and Lighting Standard Drawings.

F. Pedestrian Signal Indications

Provide pedestrian signals at vehicular signal locations. Install crosswalks at intersections, when sidewalk connections exist, or are installed. Pedestrian indications shall be LED countdown style with accessible pedestrian push buttons provided.

G. Pole and Head Location

Traffic signal pole and head locations shall conform to the MUTCD and direction of the City Engineer.

H. Street Light Luminaries

All traffic signal installations shall include streetlights on all mast arm equipped poles consistent with the requirements of GES, [Section 6.13.4](#) "Cobra Head Street Light Luminaries".

I. Pull Boxes

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Traffic signal pull boxes shall be No. 7, signal interconnect pull boxes shall be No 5, with locking lid and labeled "TRAFFIC SIGNAL" and shall conform to T.S. 1-4, 1-5, and 1-6 of the ADOT Traffic Signals and Lighting Standard Drawings.

J. Traffic Signal Conduit

Traffic signal conduit shall be three (3") inch Schedule 40 PVC.

K. Traffic Signal Interconnect

Traffic signal interconnect conduit shall be installed along all arterial roadways and at other locations as directed by the City Engineer per COP Standard Detail 640Q to extend the existing interconnect systems.

1. Conduit shall be two (2") inch Schedule 40 PVC
2. Interconnect cable shall be 6-pair, filled 19-gauge solid, run un-spliced.

L. Traffic Signal Plan Checklist

The Traffic Signal Plan Checklist shall be used as a guide to ensure that all of the basic elements are included in the traffic signal plans. Any signal plan that does not include each of the basic elements included on the checklist shall be returned for revision and re-submittal.

SECTION 6.16/ SIGNING

6.16.1 SIGNING

In new development, the applicant shall install all traffic control signs, which shall include but not be limited to street name, parking, stop, warning, dead end and pedestrian signing. The applicant shall be responsible for supplying and installing the required signs, posts and hardware in accordance with the required standard plans and specifications.

6.16.2 SIGN DESIGN GUIDELINES

The following publications most recent editions and approved supplements by the State and City are to be used in conjunction with the design criteria when designing traffic signing for the City of Prescott.

- A. Manual on Uniform Traffic Control Devices for Street and Highways (MUTCD); USDOT/FHWA
- B. City of Prescott Standard Specification
- C. City of Prescott Standard Details
- D. Manual of Approved Signs (ADOT)
- E. American Association of State Highway Transportation Officials (AASHTO)

6.16.3 GENERAL SIGNING GUIDELINES

- A. All signing shall conform to the most recent editions of the publications shown above with regard to size, color, shape and placement.
- B. All signs shall be new other than those shown on the plan to be relocated. All new and relocated signs shall be mounted on new posts with new hardware. Signs designed for installation on existing street light poles shall be mounted with new hardware.
- C. Traffic sign dimensions, colors and lettering shall conform to the latest MUTCD specifications. Traffic sign size shall be standard unless otherwise specified here or on the plans.
- D. All signs shall be located at least one (1') foot from the curb face to the nearest edge of the sign. All other roadways signs shall be mounted from six (6') feet to twelve (12') feet from the edge of the pavement to the nearest edge of the sign.
- E. Roadways with guardrail signs shall be located at least six (6') feet from the face of the guardrail to the nearest edge of the sign.
- F. Sign location shall be coordinated with landscaping plans to ensure sign visibility per AASHTO standards.
- G. Signs shall be mounted on street light poles whenever feasible.
- H. All signs installed in areas where parking or pedestrian movements occur shall be erected at a height of seven (7') feet above the pavement or sidewalk to the bottom of the sign or to the lowest sign in a multiple sign installation assembly with the following exceptions:

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1. The height to the bottom of a secondary sign mounted below another sign may be up to one (1') foot less than the height specified above in rural areas where pedestrians are not expected.
 2. Object markers shall be installed at least four (4') feet above the pavement.
- I. All R1-1 "STOP" signs and pedestrian warning signs shall be retro-reflective with all sheeting material to be diamond VIP grade, meeting or exceeding ASTM 4956-04.
 - J. All other signs are to be retro-reflective with all sheeting material to be high intensity prismatic meeting or exceeding ASTM 4956-04.
 - K. Sign blanks shall be 5052-H38 alloy treated aluminum with Alodine 1200 conversion coating, eight-hundredths (0.080") inch thick with rounded corners.
 - L. Stop signs are to be shown at all local street intersections within a subdivision unless an engineering study shows that no control or yield control is warranted. Stop signs shall be designed and shown on the minor leg of all collector and non-signalized arterial street intersections.
 - M. "STOP" signs and "YIELD" signs shall be a minimum of thirty (30") inches and thirty-six (36") inches in width, respectively. When specified by the requirements of the MUTCD, thirty-six (36") inch and/or forty-eight (48") inch signs may be required on major collectors and arterial streets.
 - N. At intersections where all approaches are stop controlled, "All Way" placards shall be placed below the R1-1 stop signs.

6.16.4 SIGN POSTS

- A. Sign posts shall conform to COP Standard Detail 131Q.
- B. For new construction the Telspar, Uni-strut or approved equal twelve gauge, galvanized steel, four (4) sided perforated square tubing is required. Two (2") inch tubing shall be used for smaller signs while two and one-half (2½") inch tubing shall be used for the larger signs.
- C. The post shall be tall enough to provide the minimum clearances specified in the GES, [Section 6.16.3](#) H.

- D. The base and sleeve system for the sign shall be anchored in a minimum of a twenty-four (24") inch deep, twelve (12") inch diameter foundation of concrete. The base shall have a breakaway slip base system. The exposed post from the base shall be four (4") inches to six (6") inches high.
- E. Signs over forty-eight (48") inches wide shall be mounted on two (2), two and one-half (2½") inch posts with a horizontal support frame.

6.16.5 STREET NAME SIGNS

- A. Street name signs shall conform to COP Standard Detail 132P.
- B. All reflective sheeting material shall be high intensity prismatic, meeting or exceeding ASTM 4956-04.
- C. All transparent acrylic, pressure-sensitive film shall be 3M#177 green electro cut film or approved equal.
- D. Letter font shall be uppercase four (4") inch minimum with the block numbers in the 100's with a directional arrow.
- E. These signs are constructed by applying white sheeting to the entire blank. On top of this sheeting, a green translucent pressure-sensitive film from which the legend has been cut and removed is applied. Thus, the green background is applied on top of the white sheeting resulting in a sign with a white legend and a green background.
- F. Signs that are located in the center of the block shall have " ← →".
- G. Signs that are at the beginning and end of the block have "→" " pointing up the block.

6.16.6 LOW WATER CROSSING GATES

- A. Gates shall be required on all new low water crossings.
- B. Two-hundred (200') feet prior to the low water crossing gate a thirty six (36") inch by thirty six (36") inch black on yellow warning sign shall be posted on each approach that reads "Flood Area Do Not Enter When Flooded".
- C. Black and yellow object marker (OM-3R(L)) shall be mounted on all four (4) corners of the gate facing traffic.

- D. When the gates are closed, a “Road Closed” (R11-2) shall be affixed to the gate facing oncoming traffic. To the left of the R11-2 sign, in the left most lane shall be affixed to the gate a red on black nine ball (OM4-2).
- E. Knox padlocks will be required on all low water crossing gates.

6.16.7 END OF ROAD DELINEATORS

- A. Two types of end of road barriers can be used. These are a modified MAG Standard Detail 130.
 - 1. Type A is constructed of two (2) treated four by four (4” x 4”) inch redwood posts sunk in the ground thirty-six (36”) inches and four (4’) feet apart, extending to a height of five (5’) feet. Three (3), eight (8”) inch minimum, twelve (12”) inch maximum by six (6’) foot boards shall be mounted horizontally twenty (20”) inches on center between the posts. The boards shall be painted in a chevron design, black and yellow. Starting in the middle of the board the chevrons shall slope to both sides. Attached to the center board shall be a thirty six (36”) inch by thirty six (36”) inch black on yellow warning sign with the words “Road Closed”.
 - 2. Type B is a smaller version of Type A. The two (2) treated four by four (4” x 4”) inch posts shall be five (5’) feet apart and extend three (3’) feet above ground. One horizontal board shall be attached at the three (3’) foot level. On the board will be the same color scheme, chevrons and sign as Type A.
 - 3. In all cases in which a road closure is greater than one-hundred fifty (150’) feet from the nearest street intersection or the roadway alignment obscures the closure, pre-warning signs shall be required and placed as directed by the City Engineer.

6.16.8 SIGNING PLAN CHECKLIST

The Signing Plan Checklist shall be used as a guide to ensure that all of the basic elements of a project signing and striping plan are included in the signing plans. Any signing plan that does not include each of the basic elements included on the checklist shall be returned for revision and re-submittal.

SECTION 6.17/ STRIPING

6.17.1 STRIPING

New plats and developments shall install all striping and pavement legends as determined by the City Engineer, which shall include but not be limited to lane lines, crosswalks, stop bars legends, and arrows. The applicant shall be responsible for supplying and installing the required striping in accordance with standard plans and specifications.

6.17.2 STRIPING DESIGN GUIDELINES

- A. The following publications' most recent editions and approved supplements by the State and City are to be used in conjunction with approved design criteria when designing striping for the City of Prescott.
 - 1. Manual on Uniform Traffic Control Devices for Street and Highways (MUTCD); USDOT/FHWA
 - 2. City of Prescott (COP) Standard Details
 - 3. American Association of State Highway Transportation Officials (ASSHTO)

- B. General Striping Guidelines
 - 1. Striping shall conform to the most recent editions of the MUTCD specifications with regard to size, color, reflectivity and placement unless otherwise specified here.
 - 2. All roadways classified as Commercial/Industrial Collector or higher shall require centerline striping, stop bars, crosswalks and other appropriate legends and arrows. Local residential streets are normally exempt.
 - 3. All thermoplastic applications shall conform to ADOT specification 704. Transverse markings, symbols and legends shall be 90 mil (0.090 inch) thick, longitudinal markings shall be 60 mil (0.060 inch) thick alkyd extruded thermoplastic
 - 4. All paint application shall conform to ADOT specification 708.
 - 5. All conflicting striping, pavement markings, and curb paint shall be removed by wet sandblasting or other approved method prior to the installation of new striping. Slurry or paint shall not be used to cover existing paint. Pavement that is damaged due to the removal of markers or striping shall be repaired to the satisfaction of the City Engineer.

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6. Through lanes shall be a minimum of twelve (12') feet in width, with the exception that lane(s) of eleven (11') feet minimum are used on the center lane of a Major Arterial street, per COP Standard Detail 606P.
7. Turn lanes shall equal that of the through lanes but shall be at least ten (10') feet wide.
8. Parallel parking lanes adjacent to the curb with a gutter pan shall be a minimum of five (5') feet in width from the edge of gutter.
9. A maximum of two (2') foot offset in the lane striping through an intersection is acceptable.
10. All skip lane lines and edge of road markings shall be four (4") inches in width unless otherwise specified.
11. Four (4") inch solid white edge lines shall be used on streets that do not have curb and gutter.
12. All skip white or yellow road markings shall be a ten (10') foot stripe with a thirty (30') foot gap (forty (40') foot repeated interval).
13. Crosswalks shall normally be twelve (12") inches in width and shall be installed as shown in COP Standard Detail 630P and 631Q.
14. Stop bars shall be eighteen (18") inches wide and set back two (2') feet from the crosswalk, or in the absence of a crosswalk placed at the desired stopping point. They are required at all intersection locations where crosswalks are used and at all stop controlled intersections on streets classified a Residential Collector and higher. Additionally stop bars are required at all Local Residential and Rural Local Residential street intersections that intersect streets classified a Residential Collector and higher.
15. Dual turn lanes shall require the installation of lane delineation markings, which shall consist of white six (6") inch wide thermoplastic skips placed in a two (2') foot stripe with a four (4') foot gap pattern.

C. Crosswalks

Crosswalks shall be required at all legs of signalized intersections and roundabouts and may be required at other locations in yellow or white, standard

or high visibility types as determined by the City Engineer. Refer to COP Standard Detail 630P and 631Q.

1. A white twelve (12") inch transverse crosswalk shall be used at all designated pedestrian crossings that require the minimum visual treatment.
2. A white crosswalk with twelve (12") inch diagonal and longitudinal marking shall be used on collector streets with higher volumes to provide increased visibility to drivers. This will result in a more noticeable pavement marking than the standard white crosswalk.
3. A white crosswalk with twenty-four (24") inch transverse and twelve (12") inch longitudinal markings provides the highest visibility to drivers and shall be used on arterial streets with the greatest volumes, requiring a more noticeable pavement marking than the diagonal white crosswalk.
4. A yellow crosswalk with twelve (12") inch transverse and optional diagonal markings shall be used at designated school crossings. Its twelve (12") inch wide diagonal markings help to provide increases visibility to drivers and is the standard school crossing on streets with lower volumes.
5. A yellow crosswalk with twenty-four (24") inch transverse and twelve (12") inch longitudinal markings shall be used at designated school crossings on streets classified as Major Collector and higher, requiring a more noticeable pavement marking than the diagonal yellow crosswalk.

D. Speed Hump Markings

Speed humps shall be striped in accordance with the COP Standard Detail 210Q. The markings shall be white extruded or precut thermoplastic (90-mil thickness).

E. Bike Lane Marking

1. Bike lanes shall be installed in accordance with the most recent version of the Bicycle and Pedestrian Plan as adopted by the City Council and marked per COP Street Typical Sections and the MUTCD Part 9.
2. Bike lane symbols shall be installed as part of on-street bike lane striping at the beginning of each block and at a spacing of six-hundred (600') feet thereafter in the absence of intersecting streets.

Article 6/Transportation and Traffic

3. Bike lane striping lines shall be six (6") inches in width, solid white unless otherwise specified.
4. Bike lanes shall be a minimum of four (4') feet in width when placed adjacent to the lip of gutter and five (5') feet in width when placed between a travel lane and a parking aisle.

SECTION 6.18/ GUARDRAIL

All new developments shall provide for the design and construction of guardrail as determined by the warrants, procedures and policies in the AASHTO Roadside Design Guide. In areas where short sections of guardrail are warranted the developer shall design and construct additional length sufficient to ensure that the guardrail system has collective post strength to resist failure or total displacement of the system.

ARTICLE 7/ PRIVATE DRY UTILITIES

SECTION 7.1/ GENERAL INFORMATION

7.1.1 PURPOSE

- A. The purpose of this section is to provide guidelines for location and installation of private utilities within dedicated City Right-of-Way (ROW), private streets and Public Utility Easements (PUE). This section also provides criteria to integrate private utilities with City water and sewer.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

7.1.2 APPLICABILITY

This Article will identify the minimum criteria for design, permitting, construction and installation of private utilities in dedicated City ROW, private streets and PUE.

SECTION 7.2/ PRIVATE UTILITIES

7.2.1 FRANCHISE AGREEMENTS

- A. Franchise Agreements are currently in effect with the following utility providers:
- Power Arizona Public Service Company (APS)
 - Natural Gas Unisource Energy
 - Cable TV Cable One
 - Telephone Century Link

All franchise agreements are on file in the office of the City Clerk and may be reviewed upon request.

- B. Availability of Private Utilities

The property owner, developer or contractor shall be responsible for contacting private utility companies regarding the availability of services for private development.

SECTION 7.3/ DEDICATION OF PUBLIC UTILITY EASEMENTS

The developer shall dedicate all public utility easements necessary to provide utility service to and through the proposed project. All Public Utility Easements (PUE) shall be recorded on the plat, survey or by separate instrument in the office of the Yavapai County Recorder.

SECTION 7.4/ BLUE STAKE NOTIFICATION

A Blue Stake notification shall be placed on each sheet of the plans. Prior to excavation in the City Right-of-Way (ROW) or Public Utility Easement (PUE), the contractor shall call Blue Stake (1-800-STAKEIT) for location of all underground utilities.

SECTION 7.5/ UNDERGROUND PLACEMENT

New and existing private utilities within a proposed development shall be placed underground. Transformers, switching cabinets, terminal boxes and 69 KV power transmission lines or larger may be placed above ground, within the Public Utility Easement (PUE). Other locations will require the approval of the City Engineer. Refer to Land Development Code (LDC), Section 7.4.13.

SECTION 7.6/ CODE CONFLICTS

Prescott City Code (PCC), Land Development Code (LDC), and Maricopa Association of Governments (MAG) may also apply to the installation and construction of private utilities. If a regulatory conflict exists, the code, which is most restrictive, shall apply.

SECTION 7.7/ CONFLICT REVIEW

- A. Private Utilities located within the Public Utility Easement (PUE) or crossing City ROW will require a conflict review by the City Public Works/Engineering Department prior to the issuance of a Right-of-Way Permit.
- B. Construction plans for private utility facilities located within City ROW, Private Street or PUE shall be submitted to Public Works/Engineering Department for review and approval to ensure that the integrity and minimum separation of both the public and private utilities is maintained in accordance with Prescott City Code (PCC), Chapter 8-2.

SECTION 7.8/ PLAN REQUIREMENTS AND GUIDELINES

The plans submitted shall conform to the CADD mapping standards. The City reserves the right to reject the plans as submitted by the private utility company. It is recommended that a meeting be scheduled with the City to review requirements.

Minimum standards of the plans shall meet the following:

- A. Identify and dimension all existing and/or proposed utilities in the City Right-of-Way (ROW) and Public Utility Easement (PUE).
- B. Show all existing rights-of-way and/or proposed PUE(s).
- C. Show existing and/or proposed street improvements. (e.g. pavement, curb, gutter, sidewalk, driveways, street light poles, water meters, fire hydrants, traffic signals and signal cabinets).
- D. Identify and dimension all existing and/or proposed City of Prescott public utilities. (E.g. water, sewer, drainage and streetlights). To avoid conflict, underground utilities shall be shown on the profile with depth elevations and stationing. Profile shall be on the same sheet as the plan view, aligned with the matching station. Utilities may have different stationing, in this case reference the corresponding street station. The nearest City of Prescott monument shall be referenced.
- E. Identify and dimension all underground and above ground facilities including those owned by other providers within the City ROW or PUE.
- F. Plans shall be a minimum sheet size of eleven by seventeen (11" X 17") inches and a maximum size of twenty-four by thirty-six (24"X 36") inch. Refer to the City of Prescott CADD standard for right-of-way.
- G. Minimum separation between non-city private utilities and City of Prescott public utilities is four (4') feet horizontal, and two (2') feet vertical in accordance with COP Standard Detail 402P.
- H. Submit three (3) bond sets of plans to City Hall including a traffic control plan in accordance with MUTCD along with the right-of-way application for review by the Public Works/Engineering Department. A traffic control plan must be approved by the City of Prescott Engineer.

SECTION 7.9/ PERMIT REQUIRED

Prior to issuance of a permit to allow the excavation and installation of private underground utilities (e.g. power, telephone, natural gas, cable TV and fiber optic) in any City Right-of-Way (ROW), private street or Public Utility Easement (PUE), the developer or contractor installing underground private utilities shall apply for and receive a Right-of-Way Permit from the Public Works Department. The permit request shall include construction plans and a Traffic Control Plan. All work performed as part of the Right-of-Way Permit shall conform to the requirements of this section and Prescott City Code (PCC), Chapter 8-2.

SECTION 7.10/ COMMON TRENCH AND LOCATION

- A. On local residential streets, private utilities, except natural gas, shall be located in a common trench on the side of the street without sidewalk, behind the curb and within the Public Utility Easement (PUE). Exceptions to using a common trench and location would require approval by the City Engineer. Refer to COP Standard Detail 402P for typical minimum clearances of City and private utilities.
- B. On collector and arterial streets, with sidewalks on both sides of the street, private utilities, except natural gas, shall be located in a common trench behind the sidewalk and within the PUE, on the side of the street with the least conflict with City utilities. Exceptions to using a common trench and location would require approval by the City Engineer. Refer to COP Standard Detail 402P for typical minimum clearances of City and private utilities.
- C. Natural gas shall be located in a separate trench within the City ROW, PUE or private streets on the opposite side of the street of the other private utilities. Exceptions to this location would require approval by the City Engineer. Natural gas may be placed in a common trench with other utilities, with written approval from all utility franchisees within the City.
- D. Trench and backfill for all underground private non-city utilities shall conform to COP Standard Detail 200Q.

SECTION 7.11/ MINIMUM CLEARANCES

All proposed private utility facilities shall maintain the following minimum clearances between City of Prescott utilities: Refer to COP Standard Detail 402P.

- Horizontal – Four (4') feet
- Vertical – Two (2') feet

SECTION 7.12/ PAVEMENT CUTS

- A. There shall be no open trenching (pavement cut) on public streets with paved surfaces less than five (5) years old.
- B. Open trenching (pavement cut) is allowed on streets with paved surfaces that are a minimum of five (5) years old. Backfill and pavement replacement shall conform to COP Standard Detail 200Q.
- C. Pavement patching for the street cuts shall be made within twenty-four (24) hours of completion of work as specified on COP Standard Detail 200Q.
- D. Depending on condition of existing pavement, additional removal and replacement of pavement wider than the trench may be required. When the trench of any lineal utility project is within three (3') feet or less from lip of gutter, the pavement area between the trench and lip of curb will also require full removal and replacement. Pavement replacement shall be the minimum structural section for said street classification. In no case shall the replacement of existing asphalt be less than three (3") inches asphaltic concrete (AC) over six (6") inches aggregate base course (ABC) (the minimum residential section). In addition should any raveling or damage occur to the existing pavement within the construction area, the damaged areas shall be saw cut, removed, and replaced. Damage to curb, gutter and sidewalk shall require full panel removal with saw cuts made at each joint. Removal and replacement of any and all existing infrastructure, damaged pavement, concrete, landscaping and irrigation, etc. shall be at the contractor's or private utility's expense.

SECTION 7.13/ HORIZONTAL BORES

In lieu of pavement cuts, developers and contractors may use boring and jacking. Designs with pavement boring shall include a boring profile to insure proper separation is maintained from all existing utilities. Profiles of existing shall be potholed prior to start of work and included on the boring profile. Utility potholes shall remain open with plate at critical crossings to visually verify depth of bore and avoid utility conflicts.

SECTION 7.14/ DRY UTILITIES CHECKLIST

- A. Dry Utility Plan sheets shall be submitted in accordance with the General Engineering Standard (GES), Article 8. Plans shall provide at a minimum the following:

Article 7/Private Dry Utilities

1. Property lines;
2. Right-of-Way (ROW) lines;
3. All recorded and proposed public utility easement(s) (PUE(s));
4. Identify and dimension all existing and proposed private utilities;
5. All existing and proposed infrastructure including curb, gutter, sidewalk, pavement, water, water meters, fire hydrants, wastewater, drainage culverts and catch basins, street light poles, traffic signals, signal cabinets; and
6. Traffic Control Plan

ARTICLE 8/SURVEY

SECTION 8.1/ INTRODUCTION

8.1.1 PURPOSE

- A. The intent of this section is to ensure all projects that involve topographic, boundary, control, as-built, and ALTA surveys; aerial mapping survey ground control; construction staking; right-of-way and easement investigation; preparation of legal descriptions and exhibits; and improvement plans will apply consistent methods and standards pertaining to ground surface measurement within the City of Prescott.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

8.1.2 APPLICABILITY

All survey work in the City of Prescott will be performed under the direction of a professional land surveyor registered in accordance with requirements of the State of Arizona Board of Technical Registration at <http://www.btr.state.az.us/>

8.1.3 STATEMENT OF INTENT WITH REGARD TO DIGITAL DATA

The City of Prescott requires digital CADD data for all submittals. It is the City's intent to use this base form data in-house to benefit the public by updating GIS models, City Survey Models, and As-Built Records with increased accuracy and efficiency. The City shall not release proprietary CADD format digital information in base form without the consent of the design engineer or registered land surveyor. The City shall have the right to release IMAGE files of plan set data to the public as public record information in the same way that hard copy plan files are released.

8.1.4 GEOGRAPHIC INFORMATION SYSTEM (GIS) TECHNOLOGY

The City is using GIS technology for capturing, managing, analyzing, and displaying all forms of geographically referenced data and information. GIS data submitted to the City shall comply with the Arizona Spatial Data Accuracy and Geo-Referencing Standards available from Arizona Professional Land Surveyors (APLS).

SECTION 8.2/ LAND SURVEYING

8.2.1 BOUNDARY SURVEY STANDARDS

- A. Boundary Surveys: All land survey work will be performed within the guidelines of the Arizona Boundary Survey Minimum Standards. These standards may be obtained from the State of Arizona Board of Technical Registration at 1110 West Washington Street, Suite 240, Phoenix, Arizona 85007.
- B. ALTA Surveys: ALTA surveys will be based on American Land Title Association and National Society of Professional Surveyors, latest edition, Minimum Standard Detail Requirements for ALTA/ACSM Land title Surveys:
<http://www.nsps.us.com/index.cfm?fuseaction=page.viewpage&pageid=670>

8.2.2 HORIZONTAL AND VERTICAL CONTROL SYSTEM

- A. Survey Datum Requirements. When submitting survey data for engineering plans that are to be submitted to the City of Prescott (i.e.: capital improvement projects, improvement plans for subdivision and commercial site improvements, as-built plans, etc.) the data must meet the following survey datum requirements:
 - 1. Units of measure shall be in international feet for horizontal and vertical coordinates, North American Vertical Datum of 1988 (NAVD88) for vertical, and City of Prescott coordinates for horizontal. Only City of Prescott Coordinates will be accepted. Please refer to [Table 8-1](#).
 - 2. A survey block or note listing at least two on-site points conforming to “City of Prescott Survey Datum Requirements” must be provided. These two points must have a Northing, Easting and a NAVD88 elevation.
 - 3. For establishment of survey monuments refer to COP Standard Detail 120P entitled, “Survey Marker.”
 - 5. All CADD files shall be on a single disk media in AutoCAD (a release currently supported by Autodesk subscription plans) or DGN format (for projects created in Microstation). CADD files shall consist of base/design files on the city coordinate system to include, but not limited to the following: Topography, Right-of-way, Sewer, Water, Drainage, Grading, Typical sections, etc. All deliverables to include, but not limited to the following: General, Civil, Sewer, Water, Standard Details, etc. Also, include a PDF file of all deliverables. All CD/DVD’s shall be labeled with the project name, company name, contact name, phone number, and the City of Prescott tracking number.

SECTION 8.3/ BENCHMARKS

8.3.1 BENCHMARK REFERENCE

All civil engineering design plans must reference on the cover sheet the published NGS monument and City of Prescott benchmarks used for vertical control. Description and elevation in the required NAVD88 datum shall be shown.

8.3.2 PLAN DATUM REFERENCE

All civil engineering design plans shall contain a statement certifying the horizontal and vertical datum used for all horizontal locations and elevations, which are represented in the plans. The statement shall be included on the cover sheet immediately below the benchmark.

**TABLE 8-1
SURVEY DATUM REQUIREMENTS**

| CITY OF PRESCOTT SURVEY DATUM REQUIREMENTS | | |
|---|---|-------------------------------------|
| COORDINATE UNITS: | International Feet | |
| DISTANCE UNITS: | International Feet | |
| HEIGHT UNITS: | International Feet | |
| DATUM | | |
| GEODETTIC (HORIZONTAL) DATUM: | North American Datum of 1983 (1992), (NAD83 (1992)) | |
| COORDINATE SYSTEM: | Arizona Coordinate System (State Plane) | |
| ZONE: | Central (0202) | |
| VERTICAL DATUM: | North American Vertical Datum of 1988, (NAVD88) | |
| GEOID MODEL: | GEOID99 (Conus) | |
| CITY OF PRESCOTT COORDINATE SYSTEM (COPCS) – CONVERSION FROM STATE PLANE | | |
| COPCS NORTHING= | $(\text{State Plane Northing} \times 1.000329975) - 701,456.0090$ | |
| COPCS EASTING= | $(\text{State Plane Easting} \times 1.000329975) + 69,457.2499$ | |
| Note: Distances computed between COPCS coordinates approximate “ground” distances | | |
| STATE PLANE – CONVERSION FROM CITY OF PRESCOTT COORDINATE SYSTEM | | |
| STATE PLANE NORTHING= | $(\text{COPCS Northing} + 701,456.0090) \times 0.999670134$ | |
| STATE PLANE EASTING= | $(\text{COPCS Easting} - 69,457.2499) \times 0.999670134$ | |
| EXAMPLE - CITY OF PRESCOTT MINGO BASE | | |
| LATITUDE | 34°34'29.27969" N | |
| LONGITUDE | 112°28'48.72638" W | |
| HEIGHT | 5582.412' | |
| STATE PLANE COORDINATES | | CITY OF PRESCOTT COORDINATES |
| NORTHING | 1,301,026.703 | 600,000.0000 |
| EASTING | 530,367.742 | 600,000.0000 |
| ELEVATION | 5,673.955' | 5,673.955' |

ARTICLE 9/ ALTERATIONS AND MODIFICATIONS

SECTION 9.1/ INTRODUCTION

9.1.1 PURPOSE

- A. The purpose of this Article is to provide guidance to those seeking a formal request to allow an alteration or modification of the adopted General Engineering Standards (GES) and City of Prescott supplement to MAG Specifications and Standard Details. This article outlines submittal guidelines, processing timelines for review, fees and the decision and appeal processes for all types of alternations or modification requests. There are three types of alterations or modifications.
1. Administrative Alteration or Modification
 2. Technical (Minor) Alteration or Modification
 3. Design (Major) Alteration or Modification
- B. The most appropriate time to submit a formal alteration or modification to the engineering standards request is in the preliminary stages of a project's development, in order to avoid delays to the project. Once an applicant decides to move forward with the formal request for an engineering standards alteration or modification, the applicant must follow the process outlined.
- C. Submittal of the formal request for an alteration or modification of the engineering standard does not guarantee approval. Therefore, if the applicant proceeds with design prior to City staff providing a written determination on the deviation, it is at the applicant's own risk.
- D. While a request is under review, the plan review for the project will be placed on an administrative hold until the requested alteration or modification has been completed.

9.1.2 QUALIFICATION CRITERIA

- A. For an engineering standards alteration or modification request to be considered, the applicant must demonstrate that there is a unique property condition causing substantial hardship. In order to qualify for an engineering standards deviation request, the following criteria must be true:
1. There are unique conditions or circumstances applying to the use of the subject property in regards to:

Article 9/Alterations and Modifications

- a. Lot size or shape
 - b. Topography
 - c. Access control
 - d. Drainage patterns
 - e. Distance to nearest Water Service
 - f. Distance to nearest Sewer Service
 - g. Unusual utility conflicts
2. The condition causing hardship was not created by the applicant, owner, or any previous owner of the property. The property hardship cannot be self-imposed.
 3. Without the granting of the alteration or modification, the property cannot be reasonably used.
 4. Certification that the alteration or modification will not be materially detrimental to persons residing or working in the vicinity, to the adjacent property owner, to the neighborhood, or public welfare in general.
 5. State or Federal requirements or standards cannot be altered or modified by the City.
- B. It is the burden of the applicant to prove all of the above criteria true. If all of the above criteria can be proven, the Engineer of Record should contact the Public Works Department to discuss the feasibility of applying for an alteration or modification.
- C. In no case shall economic hardship qualify as criteria for evaluating an engineering deviation.

SECTION 9.2/ ALTERATION OR MODIFICATION PROCESS

9.2.1 SUBMITTALS

When deemed appropriate, the design engineer/applicant shall prepare a formal submittal to request consideration of an alteration or modification of the adopted General Engineering Standards.

9.2.2 APPLICATION AND DOCUMENTS

- A. All requests for alterations or modifications to General Engineering Standards shall be submitted on a Public Works, “General Engineering Standard Alteration/Modification Application”. Applications shall be filled out completely with the printed name and signature of the current property owner and/or agent, as applicable.
- B. All requests for Alteration or Modification to General Engineering Standards shall be made with the first submittal or as soon as the issue is identified.
- C. In addition to the General Engineering Standard Alteration/Modification Application, the submittal shall include:
 - 1. A narrative describing the project, the proposed modification, justification for the request as defined in the General Engineering Standards, [Section 9.1.2](#), and the section of the General Engineering Standards, which relates to the modification request.
 - 2. A site plan and an engineered plan signed by the registered professional for the proposed project.
 - 3. Specific engineering analysis needed by staff to complete the evaluation of the request to include but not be limited to, traffic studies, drainage studies, water and sewer design reports, geotechnical studies, bridge reports, structural reports, typical sections, cross sections, engineering calculations, and alternative designs.
 - 4. Should additional information be necessary to clarify or facilitate the review of an application, the City may request any other pertinent information deemed necessary, regardless of items already submitted for other review processes.
- D. A complete application, technical documents, and fees may be submitted to the permit center at City Hall, 201 S. Cortez, Prescott, AZ 86301.
- E. Application for Alteration or Modification to General Engineering Standards may be submitted prior to or with the underlying development application.
- F. The City Engineer shall determine the classification for each alteration or modification request (i.e. Administrative Modification, Technical (Minor) Modification or Design Standard (Major) Modification) based upon the complexity

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of the request. The City Engineer may modify the classification after reviewing the submittal.

9.2.3. APPLICATION COMPLETENESS

- A. An application submittal shall be deemed complete only after the City Engineer reviews the application, including all required or requested information and exhibits.
- B. The City Engineer shall make a determination of application completeness within three (3) working days of the submittal.
 - a. If an application is determined to be incomplete, the City Engineer will contact the applicant to explain the application's deficiencies and return the incomplete submittal.
 - b. No further processing of the application shall occur until the required submittal elements are provided.
 - c. If the deficiencies are not corrected by the applicant within thirty-(30) calendar days of notification, the application shall be considered withdrawn.

9.2.4. JUSTIFICATION FOR REQUEST

- A. City staff shall evaluate the alteration or modification request and reach a determination based upon the information submitted and the criteria listed below:
 - Public Safety (No deviation will be granted which compromises the public's safety)
 - Function
 - Traffic Management
 - Durability/Reliability
 - Cost of Operations and Maintenance to the City
 - Water and Sewer Service availability
 - Environmental Quality
 - Storm Water Quality
 - Appearance
 - Land Development Code/Prescott City Code (PCC)
 - Quality of Life
 - Other appropriate factors that may contribute to the council goals for the planning and development of the City
- B. Self-imposed conditions or economic hardship shall not be used as justification to grant an alteration or modification.

9.2.5. FEES

Alteration/Modification submittals shall be accompanied by the payment of a fee. Submittals without the fee will not be accepted. Fees shall be collected in accordance with the most recent fee schedule adopted by the City Council. If an application is denied or withdrawn, the fee payment is non-refundable.

| | |
|---|----------|
| Administrative Alteration/Modification | \$175.00 |
| Technical Alteration/Modification | \$350.00 |
| Design Standard Alteration/Modification | \$700.00 |
| Appeal Fee | \$350.00 |

9.2.6. EVALUATION

The technical evaluation of the alteration or modification request, once deemed complete, will be completed within the period of time specified in General Engineering Standards, Section [9.3](#), [9.4](#), or [9.5](#). If the evaluation is expected to take longer than specified timeframe, the applicant will be notified at the time of acceptance of the application, whenever possible.

SECTION 9.3/ ADMINISTRATIVE ALTERATION OR MODIFICATION

Administrative alterations or modifications are defined as a modification involving minor aspects of a particular standard that do not materially affect the functionality of the standard. Examples include but are not limited to:

- Selection of Material and Appurtenances for roadways
- Driveway Grades
- Vertical & Horizontal Alignments for Utilities
- Extra Protection Method to Meet Water/Sewer Separation Requirements

9.3.1 REVIEW PROCEDURE

Within five (5) working days of the determination of application completeness, the City Engineer shall:

- A. Review and evaluate the request in light of the General Engineering Standards and other relevant information.
- B. Consult with other staff, as necessary.
- C. Render a decision to grant or deny the request

9.3.2 WRITTEN DETERMINATION

Within five (5) working days of the determination of application completeness, the City Engineer will provide a written decision in response to the application.

SECTION 9.4/ TECHNICAL (MINOR) ALTERATION OR MODIFICATION

Technical (Minor) Alteration or Modification is defined as a modification involving aspects of a particular standard that may change its general function, but not its essential function. These include but are not limited to:

- Asphalt or Base Course Thickness
- Surface Materials
- Geometric Design (length, width, bulb radius, medians, etc)
- Sight Distances
- Street and Driveway Grades
- Rolled Curb versus Straight Curb (Not Related to Drainage)
- Grade Restrictions with Less than Minimum Cover over Water/Sewer Mains
- Proposed Structural Resolution at Point of Connection into Existing City Sewer System

9.4.1 REVIEW PROCEDURE

Within ten (10) working days of the determination of application completeness, the City Engineer shall:

- A. Review and evaluate the request in light of the General Engineering Standards and other relevant information.
- B. Consult with other staff, as necessary.
- C. Render a decision to grant or deny the request

9.4.2 WRITTEN DETERMINATION

Within ten (10) working days of the determination of application completeness, the City Engineer will provide a written decision in response to the application.

SECTION 9.5/ DESIGN (MAJOR) ALTERATION OR MODIFICATION

Design (Major) Alteration or Modification is defined as a modification from the General Engineering Standards substantial enough to affect the essential function of the standard. Examples include but are not limited to:

Article 9/Alterations and Modifications

- Selection of Material and Appurtenances (For Utilities or Roadways)
- Vertical and Horizontal Alignments for Roadway and Utilities
- Geometric Design (length, width, bulb radius, medians, etc)
- Design Speed Variances
- Pavement Widths
- Crossroad Circulation
- Offset Cross Street Alignments
- Access Policy and Cross Circulation Requirements
- Rolled Curb versus Straight Curb Related to Drainage
- Typical Section Modifications
- Quantity & Size of Water Meters/Sewer Services from Approved Design Reports and Plans
- Modification of Water System Looping from Approved Design

9.5.1 REVIEW PROCEDURE

Within fifteen (15) working days of the determination of application completeness, the City Engineer shall:

- A. Review and evaluate the request in light of the General Engineering Standards and other relevant information.
- B. Consult with other staff as necessary.
- C. The City Engineer may obtain an independent review by a private design professional with expertise in the area of the request, if deemed necessary. An additional fee equal to any cost the City may incur for the independent review will be assessed to the applicant.
- D. Render a decision to grant or deny the request.

9.5.2 WRITTEN DETERMINATION

Within fifteen (15) working days of the determination of application completeness, the City Engineer will provide a written decision in response to the application. If an independent review is required, an extension of the review time may apply. In no case shall the written decision exceed thirty (30) working days.

SECTION 9.6/ APPEAL

9.6.1 APPEAL PROCEDURE

An applicant has the right to appeal the determination rendered by the City Engineer, however they must submit a formal appeal to the Public Works Director. Submitting a formal appeal requires payment of a subsequent appeal application fee, the completion of an appeal application with a brief narrative discussing the rationale for the appeal, and a copy of the original alteration/modification submittal.

9.6.2 REVIEW PROCEDURE

Within fifteen (15) working days of the determination of application completeness, the Public Works Director will:

- A. Seek a final recommendation of the City Engineer.
- B. The Public Works Director may, at his/her discretion, request that a Technical Review Panel consider the appeal and provide the Director with a recommendation. The Technical Review Panel will be comprised of a five member panel consisting of representatives from the following areas:
 - a. Elected/Appointed Official (1 member)
 - b. Professional Registrant, not associated with applicant or project (2 members)
 - c. Community Development Representative (1 member)
 - d. Contractor (A License), not associated with applicant/project (1 member)
- C. The Public Works Director will compile the comments and recommendations from all participating departments, City Engineer and Technical Review Panel.
- D. After review of this information, the Public Works Director will render a decision to grant or deny the request.

9.6.3 WRITTEN DETERMINATION

Within thirty (30) working days of the appeal submittal, the Public Works Director will provide a written decision in response to the application. The Public Works Directors decision shall be final.

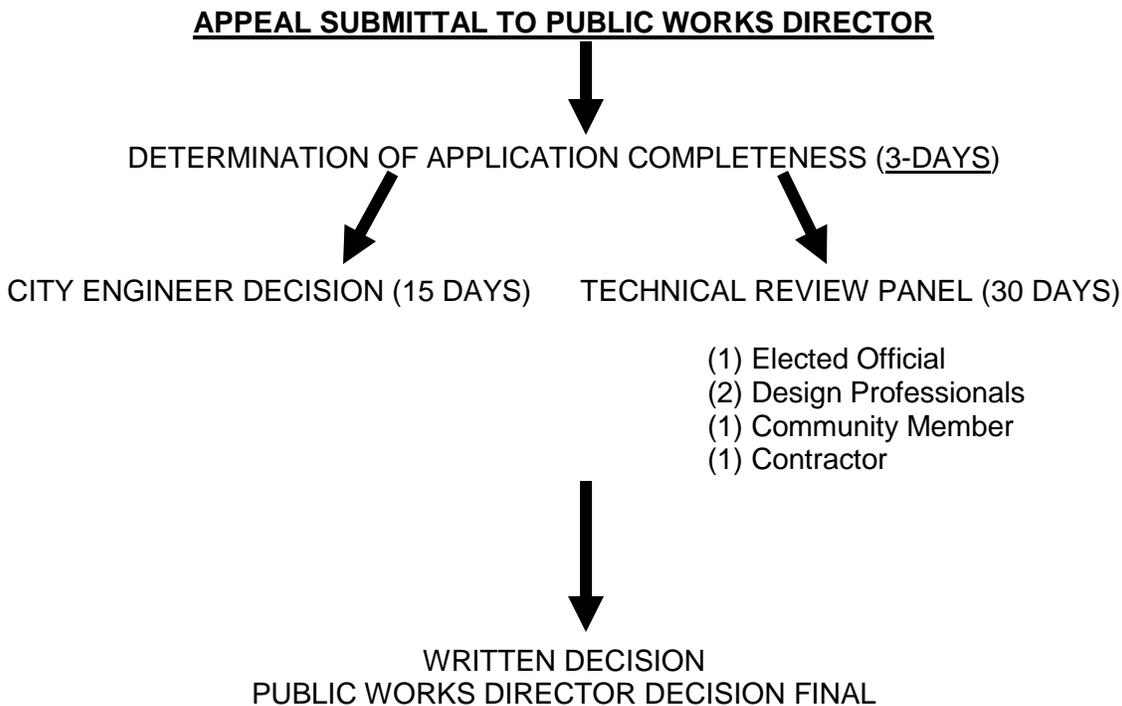
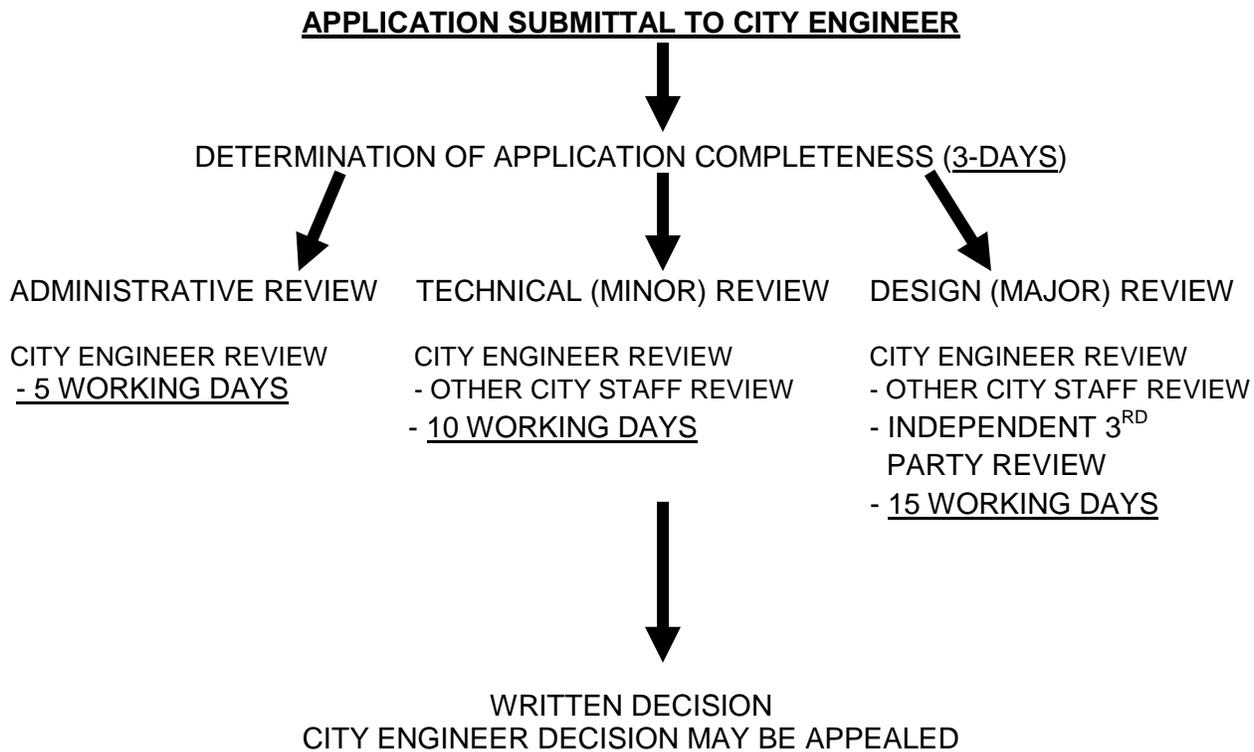
SECTION 9.7/ GENERAL CONDITIONS

- A. The alteration or modification of the same standard may not be submitted for evaluation more than once on the same project site.

Article 9/Alterations and Modifications

- B. Although alteration or modification requests can be submitted until the point which construction plans are approved, any additional plan review fees required as a direct result of submitting later in the process will be the sole responsibility of the applicant.
- C. If no appeal is filed in writing within thirty-(30) calendar days, the decision shall be considered final.

Article 9/Alterations and Modifications



ARTICLE 10/RECORD DRAWINGS (AS-BUILTS)

10.1/ INTRODUCTION

10.1.1 PURPOSE

- A. The purpose of this Article is to provide record drawing submittal, review approval guidelines and requirements for the record drawings of public infrastructure improvement projects within the City of Prescott.
- B. Alterations or modifications to the standards of this section shall be through the procedure described in Article 9 of this document.

10.1.2 APPLICABILITY

The plan submittal, review and approval guidelines shall apply to all public infrastructure projects which include grading, drainage, storm, water infrastructure, water main, reclaimed water, wastewater main, fire line, transportation or traffic infrastructure, landscaping or survey data as defined in the City of Prescott's General Engineering Standards.

10.2/ RECORD DRAWING PLANS

10.2.1 RECORD DRAWING PLAN SUBMITTAL

- A. At the completion of the construction of the project and after receiving "APPROVAL" of the "PW-Final" Inspection, the engineer of record shall make a record drawing submittal to City Hall, 201 S Cortez Street. The Engineer of record shall submit the following items as part of the Record Drawing Submittal:
 - 1. One (1) mylar copy, twenty-four by thirty-six (24" x 36") inch, of the complete "Record Drawing" set of plans. The mylar shall have an original engineers seal and signature and each page shall be noted as "As-Built". Partial set of plans shall not be accepted.
 - 2. Three (3) bond copies, twenty-four by thirty-six (24" x 36") inch, of the complete "Record Drawing" set of plans. Each copy shall have an original engineers seal and signature and each page shall be noted as "As-Built". Partial sets of plans, shall not be accepted.
 - 3. PDF file of the complete "Record Drawing" set of plans.

Article 10/Record Drawings (As-Builts)

4. Arizona Department of Environmental Quality (ADEQ), "Approval of Construction" (AOC), if applicable.
5. Digital File on the City of Prescott Coordinate System in conformance with GES, Article 8. All CAD files will be on a single disk media in AutoCAD (a release currently supported by Autodesk subscription plans) or DGN format (for projects created in Microstation). CAD files shall consist of base/design files on city coordinates to include, but not limited to the following: Topography, Right of way, Sewer, Water, Drainage, Grading, Typical sections, etc. All deliverables to include but not limited to the following: General, Civil, Sewer, Water, Details etc. All CD/DVD's will be labeled with the project name, company name, contact name, phone number, and the City of Prescott tracking number.
6. Test data in PDF and hardcopy format. e.g., water line disinfection/chlorination, pressure and bacteria test results; low-pressure air mandrel and vacuum manhole testing results, main & lateral video inspection results. Refer to GES, Article 4, Water and Article 5, Wastewater.

Failure to submit the items listed above in conformance with the GES's may result in the delay of the setting of a water meter on the site as well as delay of an "Approval to Operate and Release" from the City of Prescott Public Works Director.

- B. The "Record Drawing" submittal shall have a company transmittal sheet listing the name and number of items submitted. The transmittal sheet shall also contain the company name, contact name and phone number.
- C. Record Drawings shall be signed and sealed by a Registered Professional Engineer with the "AS-BUILT CERTIFICATION" approval block as shown below:

I HEREBY CERTIFY, TO THE BEST OF MY KNOWLEDGE, INFORMATION AND BELIEF, THAT THIS PROJECT HAS BEEN COMPLETED IN SUBSTANTIAL CONFORMANCE WITH THE APPROVED PLANS, SPECIFICATIONS AND REFERENCED STANDARDS, EXCEPT AS SHOWN HEREON; THAT THESE AS-BUILT PLANS REFLECT THE POSITION OF CONSTRUCTED IMPROVEMENTS BASED ON FIELD MEASUREMENTS; AND THAT THE MATERIALS USED IN CONSTRUCTION ARE AS SHOWN BASED ON FIELD OBSERVATION AND TEST RESULTS.

THIS CERTIFICATION DOES NOT WARRANT MATERIALS, WORKMANSHIP, METHODS OF CONSTRUCTION, OR OTHER ITEMS AFFECTING THE WARRANTY OF THIS PROJECT, TO THE CITY OF PRESCOTT. USERS OF

Article 10/Record Drawings (As-Builts)

THIS INFORMATION ARE ADVISED TO OBTAIN INDEPENDENT VERIFICATION OF ACTUAL CONDITIONS.

REGISTERED PROFESSIONAL ENGINEER (CIVIL)

DATE

- D. Benchmark locations and elevations must be shown on the cover sheet. Only the City vertical datum is acceptable.
- E. Improvements that have been constructed in accordance with the design plans shall have a bold (AB) lettered next to them. Minor as-built changes to the approved plans must be shown clearly by boldly striking through the item changed and placing the as-built information next to or as near as possible to it, and the letters (AB). All As-built changes must be larger and bolder than the original and free from background obscuring.
- F. The lines and symbols of design improvements that have not been constructed in substantial conformance with the approved design drawings shall be depicted in the as-built location on the Record Drawings; the design improvements of which shall be clearly marked with hatching, X's, clouding, or other notation, to clearly denote the improvement has not been constructed, or has been relocated from the design location.
- G. The following must be shown on the cover sheet by the City of Prescott department responsible for construction inspection:
 - a. Name of the contractor.
 - b. Permit number {identify phase if applicable}
 - c. Date of approval by City of Prescott inspector.
 - d. Name of City of Prescott inspector who witnessed the construction.
- H. Record Drawing plans shall be reviewed and approved, by the Fire Department, Water Operations Division, Wastewater Division, Field Operations, Public Works Inspector, and City Engineer.
- I. Record Drawing plans shall include the following information, if it is applicable to the project:
 - 1. Grading and Drainage Plans

Article 10/Record Drawings (As-Builts)

- a. Finished pad grades. An average pad grade may be used if the pad is not flat. Pad elevations shall not exceed plus 0.5 feet tolerance (plus 0.2 feet if located adjacent to an existing development). Pad elevations shall not exceed minus 0.2 feet tolerance (0.1 feet if located in a floodplain or adjacent to a wash or channel)
 - b. Flow line elevations of channels
 - c. Hinge point elevations on all slopes and grade breaks
 - d. Percentage of all slopes, flow lines and channels
 - e. Catch basin grates elevation at top of grate
 - f. Inverts of storm drain lines and headwalls
 - g. As-built elevations shall be provided at all drainage control points (i.e. detention overflow point, tops and bottoms of detention basins, drain rims, valley gutters, curbs, curb openings, flow line elevations in swales, etc.). As-built enough spot elevations to verify the design intentions are met (i.e. grade breaks, high/low points, scuppers, extreme outfall, etc.) Show the direction of drainage flow to illustrate that design intent has been met
 - h. Provide calculations to verify the actual as-built volume of all detention facilities included on the as-builts, as well as a table which compares the as-built volumes with the approved, required volumes indicated on the design drawings or in the approved Drainage Report
 - i. Detention calculations shall be revised to as-built condition by the engineer of record.
 - j. Flow line elevations and/or pipe inverts, grate elevations for catch basins, underground detention storage tanks, and all other drainage structures
 - k. Top of flood walls, retaining walls, and cutoff walls
 - l. Stations, offsets, and invert elevations for spillways and box culverts
 - m. When storm drain lines and appurtenances are included in the G&D drawings, please refer to the Storm Drain Plan record drawing requirements for additional required items
2. Water/Fire/Reclaimed Water Plans

Article 10/Record Drawings (As-Builts)

- a. Location of all valves
- b. Main line horizontal angle points
- c. Pipe sizes, lengths and materials
- d. Blow-offs and meter boxes
- e. Horizontal and vertical separation from existing and new utilities and drainage culverts/storm drain
- f. Street centerline station and offset dimension to:
 - 1) All fire hydrants and fittings (e.g. valves)
 - 2) Main at all changes in alignment
 - 3) All horizontal control points (e.g. centerline intersects, PC, PT)
- g. Centerline station and offset to each service tap; size of tap and meter
- h. Note centerline station, offset and elevations to all changes in vertical alignment (e.g. dips, bends, etc. required to avoid conflicts with other utilities)
- i. The drawings must clearly indicate the specific points of reference. No dimensioning from points of curvature or tangency is acceptable for record drawing purposes. In all cases where the pipeline is constructed within, or parallel in close proximity with the right-of-way, all stationing and dimensioning must be from the nearest appropriate monument line and monument line intersection
- j. When water services are not installed perpendicular to the water main, both the location of the tap at the main and the distance of the meter set from the nearest side property line of the lot must be shown
- k. On phased projects, the phase lines must be clearly shown on the key map and on the plan and profile sheets, and their locations clearly identifiable. Actual pipe end locations relative to phasing lines must be shown by dimensioning or stationing

Article 10/Record Drawings (As-Builts)

- b. Horizontal location of conduit along with elevations to top of conduit
- 6. Signing & Striping Plans
 - a. Street centerline station and offset dimension to all signage, painted arrows, wording, and symbols
 - b. Face of curb dimensions to all striping
- 7. Storm Drains Plans
 - a. Street centerline station and offset dimension to the main at all changes in alignment and/or changes in grade
 - b. Street centerline station and offset dimension to all structures and changes in alignment
 - c. Top and invert elevations for all structures
 - d. Drainage pipe inverts
 - e. Finish elevation for catch basins
 - f. Invert elevations of box culverts
 - g. Length of headwall wing
 - h. Length of catch basin wings
- 8. Landscaping

Revise as needed to reflect the addition, removal, relocation or change of irrigation main lines, plant materials or hard-scape
- 9. Street Light Plans
 - a. Record drawings for street lights are required to have the Arizona Public Service (APS) ID number of each street light noted on the plan
 - b. Street centerline stationing and offsets for street lights

APPENDIX A LIST OF TABLES

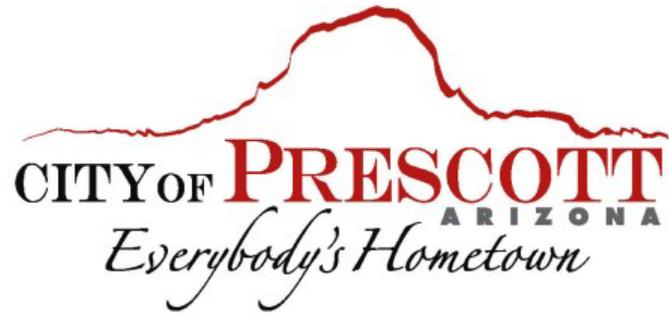
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CITY OF PRESCOTT

STANDARD DETAIL

100 SERIES: GENERAL INFORMATION

| DETAIL | TITLE |
|--------|---|
| 100Q | INDEX |
| 101P | GENERAL NOTES |
| 103P | WATER PLAN NOTES |
| 104P | SEWER PLAN NOTES |
| 105P-1 | GRADING AND DRAINAGE NOTES |
| 105P-2 | EROSION AND SEDIMENTATION CONTROL NOTES |
| 106P-1 | SIGNING AND STRIPING NOTES |
| 106P-2 | TRAFFIC SIGNAL NOTES |
| 120Q | SURVEY MARKER |
| 131Q | SIGN POST INSTALLATION |
| 132P | STREET NAME SIGN |
| 134Q-1 | MAILBOX |
| 134Q-2 | MAILBOX |
| 140Q | BOLLARD |
| 144P | SANITATION DUMPSTER |
| 145Q | SAFETY RAIL |
| 170P | BASE MOUNTED LUMINAIRE |
| 173P | ANTIQUE STREETLIGHT FOUNDATION |

200 SERIES: STREET INFORMATION

| DETAIL | TITLE |
|---------|---|
| 200P-2 | PVC PRESSURE PIPE INSTALLATION |
| 200Q-1 | TRENCH BACKFILL AND SURFACE REPLACEMENT |
| 201Q | THICKENED EDGE PAVEMENT SECTION |
| 210Q | RESIDENTIAL SPEED HUMP |
| 212Q | SOFT DIG POTHOLE REPAIR |
| 220Q-1 | CURB AND GUTTER TYPES A,B,C, AND D |
| 220Q-2 | 3' ROLL CURB |
| 221Q | CURB AND GUTTER TRANSITIONS |
| 230Q | SIDEWALKS |
| 231Q | LOCAL STREET SIDEWALK RAMP |
| 232Q | DIRECTIONAL SIDEWALK RAMPS |
| 233Q-1 | MIDBLOCK DETACHED SIDEWALK RAMP |
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| 251Q | RETURN TYPE DRIVEWAY |
| 255P | RESIDENTIAL DRIVEWAY SECTION |
| 270Q | 8" FRAME AND COVER ADJUSTMENT |
| 275Q | UTILITY CASING |

300 SERIES: WATER INFORMATION

| DETAIL | TITLE |
|---------|---|
| 301Q | VALVE BLOCKING |
| 303Q-1 | JOINT RESTRAINT FOR DIP WATER PIPES (DRAWING) |
| 303Q-2 | JOINT RESTRAINT FOR DIP WATER PIPES (TABLES) |
| 303Q-3 | REVERSE THRUST BLOCKING ACP |
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| 310PV-1 | 5/8" THROUGH 1" METER BOX APPLICATIONS |
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| 310PV-3 | 5/8" THROUGH 1" WATER SERVICE CONNECTION |
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| 317Q-2 | 2" TO 4" AIR RELEASE VALVE |
| 318P | BLOW OFF |
| 319Q-1 | TRACER WIRE |
| 319Q-2 | TRACER WIRE STATION |
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| 340Q-1 | TAPPING SLEEVES AND VALVES |
| 340Q-2 | EXPOSING WATER MAIN FOR HOT TAP |
| 360Q | FIRE HYDRANT INSTALLATION |
| 362Q | LOCATIONS FOR NEW FIRE HYDRANTS |
| 363Q | FIRE HYDRANT VERTICAL DEFLECTION |
| 364P | FIRE HYDRANT PROTECTION POLES |
| 370Q | VERTICAL REALIGNMENT OF WATER MAINS |
| 371PV | UTILITY SEPARATION |
| 390Q | BLOW OFF |
| 391Q | VALVE BOX INSTALLATION |
| 395PV | ASBESTOS CEMENT PIPE |
| 396PV | UTILITY ABANDONMENT IN PLACE |

400 SERIES: SEWER INFORMATION

| DETAIL | TITLE |
|--------|--|
| 402P | DRY UTILITY SEPARATION |
| 404P-1 | WATER AND SANITARY SEWER SEPARATION/PROTECTION |
| 404P-2 | WATER AND SANITARY SEWER SEPARATION/PROTECTION |
| 404PV | WATER AND SEWER SEPARATION/PROTECTION |
| 405Q | BROKEN SEWER LINE REPLACEMENT |
| 412Q | MANHOLE/VAULT STEPS |
| 414P | UTILITY MAIN EXPOSURE |
| 416Q | SANITARY FORCE MAIN AIR/VACUUM RELEASE VALVE |
| 420Q-1 | PRECAST CONCRETE SEWER MANHOLE |
| 420Q-2 | MANHOLE IN DRAINAGEWAYS |
| 421Q | OFFSET MANHOLE 8" TO 30" PIPE |
| 422Q | MANHOLE FRAME ADJUSTMENT |
| 423P-1 | 24" CAST IRON FRAME AND COVER |
| 423P-2 | 30" CAST IRON FRAME AND COVER |

400 SERIES: SEWER INFORMATION (CONT.)

| DETAIL | TITLE |
|--------|--|
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| 426Q-2 | BEAVER SLIDE MANHOLE |
| 427Q | SEWER STUB-OUT AND PLUGS |
| 440P-1 | SEWER SERVICE LATERAL GREATER THAN 2' DEEP |
| 440P-2 | SEWER SERVICE LATERAL LESS THAN 2' DEEP |
| 440P-3 | BACKWATER VALVE |
| 440PV | SEWER CLEANOUT - DOUBLE |
| 440Q-4 | WATER/SEWER SERVICE CURB CROSSING STAMP |
| 441Q | SEWER MAIN CLEANOUT |
| 443PV | SEWER MANHOLE VACUUM TESTING |
| 444PV | SEWER AND STORM DRAIN CLEANING |
| 445PV | TELEVISION INSPECTION |
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500 SERIES: IRRIGATION AND STORM DRAIN

| DETAIL | TITLE |
|---------|--------------------------|
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| 523PV-6 | INLET DROP STRUCTURE |
| 556PV | HYDROSEED REQUIREMENTS |

600 SERIES: TRAFFIC ENGINEERING

| DETAIL | TITLE |
|---------|---|
| 600Q | INTERSECTION APPROACH TYPICAL SECTION |
| 601P | RURAL LOCAL RESIDENTIAL X-SECTION |
| 602P | LOCAL RESIDENTIAL X-SECTION |
| 603P | RESIDENTIAL COLLECTOR |
| 604P | COMMERCIAL/INDUSTRIAL COLLECTOR |
| 605P | MINOR ARTERIAL |
| 606P | MAJOR ARTERIAL |
| 610Q | PRIVATE GATED ACCESS AT PUBLIC ROADWAY |
| 611P | INTERSECTION SIGHT VISIBILITY TRIANGLES |
| 612P | UTILITY EASEMENT ACCESS ROAD |
| 614P | FIRE LANE ACCESS |
| 620P -1 | CUL-DE-SAC 50' R/W |
| 620P-2 | CUL-DE-SAC 60' R/W |
| 621P-1 | OFFSET CUL-DE-SAC 50' R/W |
| 621P-2 | OFFSET CUL-DE-SAC 60' R/W |
| 622P-1 | KNUCKLE 50' R/W |
| 622P-2 | KNUCKLE 60' R/W |
| 630P | PAINTED CROSSWALK AT INTERSECTION |
| 631Q | STANDARD CROSSWALK MARKINGS |
| 632Q | STOP MARKINGS |
| 634P | PAVEMENT MARKINGS |
| 640Q | TRAFFIC SIGNAL INTERCONNECT CONDUIT |
| 650Q-1 | RESIDENTIAL RIGHT IN/RIGHT OUT DRIVEWAY |
| 650Q-2 | COMMERCIAL RIGHT IN/RIGHT OUT DRIVEWAY |

1. ALL CONSTRUCTION SHALL CONFORM TO MARICOPA ASSOCIATION OF GOVERNMENTS (MAG), & CITY OF PRESCOTT (COP) CONSTRUCTION STANDARDS & SPECIFICATIONS. LATEST REVISIONS, UNLESS SPECIFICALLY APPROVED BY THE CITY AND MODIFIED ON THE PLANS, IN CONJUNCTION WITH THE LATEST REVISIONS OF THE MARICOPA ASSOCIATION OF GOVERNMENTS STANDARD SPECIFICATIONS AND DETAILS (MAG STANDARDS), UNLESS SPECIFICALLY MODIFIED ON THE PLANS.
2. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN COPIES OF MAG AND CITY OF PRESCOTT STANDARDS AND SPECIFICATIONS AS WELL AS ALL OTHER STANDARDS AND SPECIFICATIONS NECESSARY TO COMPLETELY AND ACCURATELY INTERPRET THESE PLANS.
3. ALL PLANS SIGNED BY THE CITY ENGINEER ARE NULL AND VOID ONE YEAR FROM DATE OF SIGNATURE IF CONSTRUCTION HAS NOT STARTED. RESUBMITTAL AND REVIEW SHALL BE REQUIRED, AFTER ONE YEAR.
4. ALL QUANTITIES SHOWN ON THE PLANS ARE APPROXIMATE, ARE NOT VERIFIED BY THE PUBLIC WORKS DIRECTOR, AND ARE FURNISHED SOLELY FOR THE CONTRACTOR'S CONVENIENCE. THEY DO NOT NECESSARILY CORRESPOND TO BID SCHEDULE ITEMS. PAYMENT SHALL BE BASED ON BID SCHEDULE ITEMS FOR ACTUAL QUANTITIES PROVIDED AND INSTALLED. THE CONTRACTOR SHALL NOT BE RELIEVED OF HIS RESPONSIBILITY FOR INDEPENDENTLY ESTIMATING WORK QUANTITIES PRIOR TO BIDDING.
5. CITY OF PRESCOTT PUBLIC WORKS DEPARTMENT PERMID(S) WILL BE REQUIRED FOR ALL OFF-SITE CONSTRUCTION AND CONSTRUCTION WITHIN THE PUBLIC RIGHT OF WAY.
6. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN, AT HIS OWN EXPENSE, SUCH PERMITS AS ARE REQUIRED FROM THE APPROPRIATE AGENCIES.
7. THE PUBLIC WORKS DEPARTMENT SHALL BE NOTIFIED A MINIMUM OF 24 HOURS PRIOR TO BEGINNING ANY CONSTRUCTION IN THE PUBLIC RIGHT OF WAY.
8. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE AND APPROVAL OF THE PUBLIC WORKS DIRECTOR AND/OR ALL WORK MATERIALS NOT IN CONFORMANCE WITH THE PLANS AND SPECIFICATIONS IS SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
9. A THOROUGH ATTEMPT HAS BEEN MADE TO SHOW THE LOCATION OF ALL UNDERGROUND OBSTRUCTIONS AND UTILITY LINES IN THE WORK AREA. THE ENGINEER AND THE CITY OF PRESCOTT WILL NOT GUARANTEE ANY LOCATIONS OR ELEVATIONS OF EXISTING UNDERGROUND UTILITIES SHOWN ON THESE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE AND SHALL NOT BE RELIEVED OF RESPONSIBILITY FOR MAKING A COMPLETE AND ACCURATE ON-SITE DETERMINATION OF THE LOCATIONS, MATERIAL, AND SIZE OF ALL UTILITIES, STRUCTURES, AND FIELD CONDITIONS WHICH MAY AFFECT THE PROGRESS OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO STRUCTURES AND UTILITIES ENCOUNTERED DURING CONSTRUCTION AND SHALL FIELD EXPOSE EXISTING UNDERGROUND UTILITIES PRIOR TO TRENCHING IN THEIR VICINITY.
10. THE CONTRACTOR IS REQUIRED TO CONTACT BLUE STAKE (1-800-STAKEIT) A MINIMUM OF TWO WORKING DAYS (48) HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION. THE APPROPRIATE UTILITY COMPANIES SHALL BE NOTIFIED BY THE CONTRACTOR PRIOR TO ANY CONSTRUCTION.
11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE RELOCATION AND/OR SUPPORT OF ALL UTILITIES, POWER POLES, ETC., THAT MAY BE NECESSARY.
12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING TO THE PUBLIC WORKS DEPARTMENT FOR APPROVAL, TRAFFIC CONTROL PLANS PRIOR TO BEGINNING CONSTRUCTION. THE CONTRACTOR SHALL DETERMINE AND SUBMIT FOR APPROVAL THE EXACT SIGNING/TRAFFIC CONTROL DEVICES NECESSARY AND ALL TRAFFIC CONTROL WORK SHALL BE IN ACCORDANCE WITH THE LATEST REVISIONS THEREOF. NO STREET IS TO BE CLOSED, RESTRICTED, OR CONSTRUCTED UPON UNTIL A TRAFFIC CONTROL PLAN IS PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE PUBLIC WORKS DIRECTOR ONE WEEK IN ADVANCE FOR REVIEW AND APPROVAL.
13. APPROPRIATE EMERGENCY AGENCIES SHALL BE NOTIFIED A MINIMUM OF 24 HOURS PRIOR TO ANY CLOSING OF STREETS.
14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION SURVEYING AND LAYOUT WITH CONTROL PROVIDED BY THE DESIGN ENGINEER OR HIS DESIGNEE.
15. THE CONTRACTOR IS RESPONSIBLE FOR QUALITY CONTROL MEASURES SUFFICIENT TO PRODUCE MATERIALS AND WORKMANSHIP OF ACCEPTABLE QUALITY. PRIOR TO CONSTRUCTION THE CONTRACTOR SHALL SUBMIT A QUALITY CONTROL PLAN. THE CONTRACTOR AT HIS OWN EXPENSE SHALL PROVIDE AN INDEPENDENT GEOTECHNICAL FIRM TO PERFORM QUALITY CONTROL TESTING SUCH AS SOILS AND CONCRETE TESTING, AND FULL TIME ASPHALTIC CONCRETE LAYDOWN COMPACTION TESTING AND ADEQUATE PLANT CONTROL FOR EACH PAVING DAY. THE CITY, BY SEPARATE CONTRACT, WILL BE RESPONSIBLE FOR QUALITY ASSURANCE TESTING AS IT MAY DEEM NECESSARY.
16. THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL COSTS OF TESTING AND INSPECTION, INCLUDING THE PRESENCE OF CITY INSPECTORS, REQUIRED AT NIGHT OR ON WEEKENDS.
17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RE-WORK AND/OR REMOVAL AND REPLACEMENT OF ALL MATERIALS REPRESENTED BY FAILING TESTS OR SUBSTANDARD WORKMANSHIP.
18. THE CONTRACTOR SHALL IMPLEMENT BEST-HOUSE-KEEPING MEASURES, AND EROSION AND SEDIMENT CONTROL MEASURES, TO PREVENT THE TRANSPORT OF CONSTRUCTION MATERIALS INTO DRAINAGE INLETS, STORM DRAIN MANHOLES, UTILITY STRUCTURES, OR ONTO ADJACENT STREETS AND PROPERTIES.
19. APPROVAL OF A PORTION OF THE WORK IN PROGRESS DOES NOT GUARANTEE ITS FINAL ACCEPTANCE. TESTING AND EVALUATION MAY CONTINUE UNTIL THE WRITTEN FINAL ACCEPTANCE OF A COMPLETE AND WORKABLE UNIT.
20. THE CITY OF PRESCOTT MAY SUSPEND THE WORK BY WRITTEN NOTICE WHEN, IN ITS JUDGEMENT, PROGRESS IS UNSATISFACTORY, WORK BEING DONE IS UNAUTHORIZED OR DEFECTIVE, WEATHER CONDITIONS ARE UNSTABLE, OR THERE IS A DANGER TO THE PUBLIC HEALTH AND SAFETY.
21. ALL OBSTRUCTIONS IN THE RIGHT OF WAY SHALL BE REMOVED BEFORE ANY CONSTRUCTION IS PERMITTED.
22. REMOVAL OF STRUCTURES AND OBSTRUCTIONS AS NECESSARY TO COMPLETE THE WORK, OTHER THAN SPECIFICALLY SCHEDULED IN THE BID, IS INCIDENTAL TO THE CONTRACT. NO SEPARATE MEASUREMENT OF OR PAYMENT FOR UNSCHEDULED REMOVAL ITEMS WILL BE MADE.
23. CLEARING AND GRUBBING IS CONSIDERED INCIDENTAL TO THE WORK UNLESS SPECIFICALLY IDENTIFIED IN THE BID SCHEDULE. NO SEPARATE MEASUREMENT OF OR PAYMENT FOR CLEARING AND GRUBBING, AND TREE REMOVAL, WILL BE MADE. THE SITE OF ALL EXCAVATION, EMBANKMENTS, AND FILLS SHALL FIRST BE CLEARED OF STUMPS, TRASH, WEEDS, RUBBISH, TOPSOIL, AND LOOSE BOULDERS WHICH SHALL BE REMOVED AND DISPOSED OF. PRIOR TO BIDDING THE CONTRACTOR MUST SATISFY HIMSELF REGARDING THE CHARACTER OF THE SUBSOILS TO INCLUDE THE AMOUNT OF LOAM, CLAY, SAND, QUICKSAND, HARDPAN, GRAVEL, ROCK, WATER, AND ALL OTHER MATERIAL TO BE ENCOUNTERED AND WORK TO BE PERFORMED.
24. THE CONTRACTOR SHALL GUARD AGAINST DAMAGE DURING CONSTRUCTION TO EXISTING PROPERTIES AND IMPROVEMENTS. ANY ITEMS DAMAGED BY THE CONTRACTOR'S ACTIVITIES SHALL BE REPLACED IN KIND OR BETTER AT THE CONTRACTOR'S EXPENSE.
25. THE CONTRACTOR SHALL KEEP SUITABLE EQUIPMENT ON HAND AT THE JOBSITE FOR MAINTENANCE DUST CONTROL, AND SHALL CONTROL DUST AS DIRECTED BY THE APPROPRIATE AGENCY.
26. STREET AND TRAFFIC SIGNS SHALL BE RELOCATED BY THE CONTRACTOR IF NECESSARY, AT THE DIRECTION OF THE PUBLIC WORKS DIRECTOR.
27. BACKFILL COMPACTION SHALL BE TYPE 1 (MAG, SECTION 601) UNLESS OTHERWISE NOTED.
28. AGGREGATE BASE COURSE SHALL NOT BE PLACED ON SUBGRADE UNTIL SUBGRADE REQUIREMENTS HAVE BEEN ACHIEVED.
29. NO PAVING CONSTRUCTION SHALL BE STARTED UNTIL ALL UNDERGROUND UTILITIES WITHIN THE ROADWAY PRISM ARE VERIFIED FOR DETAIL CONFORMANCE, COMPLETED AND TESTED (TO INCLUDE BUT NOT LIMITED TO) SEWER TESTING, LOW AIR TESTING OF MAIN LINE AND SERVICES, TRACE WIRE TESTING, DEFLECTION TESTING AND VERIFICATION OF MANHOLES CONFORMING TO COP DETAIL 420P. WATER TESTING CHLORINATION/DISINFECTING OF MAIN LINE AND SERVICES, PRESSURE TESTING, TRACE WIRE TESTING AND VERIFICATION OF VALVE BOXES CONFORMING TO COP DETAIL 391P.
30. ALL ASPHALT CONCRETE PAVEMENT SHALL BE PER APPLICABLE MAG SPECIFICATIONS AS AMENDED BY THE CITY OF PRESCOTT. ASPHALT CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE PUBLIC WORKS DIRECTOR OR HIS DESIGNEE FOR APPROVAL PRIOR TO START OF CONSTRUCTION.
31. ALL UTILITY FRAMES, COVERS, VALVE BOXES, MANHOLES, ETC. SHALL BE ADJUSTED TO FINISH ASPHALT GRADE AFTER PLACEMENT OF SURFACE COURSE BY THE CONTRACTOR PER COP STANDARD DETAILS.
32. ACCEPTANCE OF THE COMPLETED PAVING STRUCTURES WILL NOT BE GIVEN UNTIL REPRODUCIBLE "AS-BUILT" PLANS HAVE BEEN SUBMITTED BY THE CONTRACTOR AND APPROVED BY THE CITY.
33. ALL CONCRETE TO BE AT LEAST 3000 PSI CLASS "A" PORTLAND CEMENT CONCRETE, UNLESS OTHERWISE SPECIFIED ON THE PLANS, SPECIFICATIONS, OR IN STANDARD DETAILS.
34. EDGES OF CONCRETE STRUCTURES TO HAVE A $\frac{3}{4}$ " CHAMFER, UNLESS OTHERWISE SPECIFIED ON THE PLANS.
35. CONCRETE SURFACES TO HAVE A BROOM FINISH UNLESS OTHERWISE NOTED ON THE PLANS.
36. ALL EXPANSION JOINTS TO BE SEALED WITH $\frac{1}{2}$ " EXPANSION JOINT, PRE-FORMED JOINT FILLER AND SEALER, IN ACCORDANCE WITH MAG SECTION 729.
37. DRIVEWAY ENTRANCES WILL BE LOCATED AS SPECIFIED ON THE PLANS UNLESS MODIFIED IN THE FIELD BY THE ENGINEER. ALL DRIVEWAY ENTRANCES SHALL BE CONSTRUCTED OVER 6" THICK AGGREGATE BASE COURSE PER MAG SPECIFICATION 702 AND COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, UNLESS OTHERWISE NOTED.
38. ALL DISTURBED FENCES SHALL BE REPLACED IN KIND. CONTRACTOR SHALL EXTEND FENCE REPLACEMENT TO THE CLOSEST UPRIGHT SUPPORT NECESSARY FOR STABILITY.
39. MAILBOXES SHALL BE REMOVED AND REINSTALLED AS DIRECTED BY THE U.S. POSTAL SERVICE AND THE CITY OF PRESCOTT TEMPORARY LOCATIONS SHALL BE PER U.S.P.S.
40. NO JOB WILL BE CONSIDERED COMPLETE UNTIL ALL CURBS, PAVEMENT, AND SIDEWALKS HAVE BEEN SWEEP CLEAN OF ALL DIRT AND DEBRIS.
41. THE CONTRACTOR SHALL WARRANT ALL WORK FOR A MINIMUM TWO YEAR PERIOD AFTER FORMAL ACCEPTANCE OF THE WORK BY THE CITY.

COP STANDARD DETAIL

GENERAL NOTES

Charles Andrews
CITY ENGINEER

REVISED: 07/16
DETAIL No. 101P

1. ALL WORK SHALL CONFORM TO MARICOPA ASSOCIATION OF GOVERNMENTS (MAG), & CITY OF PRESCOTT (COP) CONSTRUCTION STANDARDS & SPECIFICATIONS, WHICH ARE ON FILE IN THE OFFICE OF THE CITY ENGINEER.
2. ALL EXISTING FRAMES, COVERS, VALVE BOXES, & MANHOLES SHALL BE EITHER REPLACED OR ADJUSTED TO FINISH GRADE DEPENDING ON PLAN CALL OUT UPON COMPLETION OF PAVING, UTILITY, OR RELATED CONSTRUCTION.
3. ANY QUANTITIES SHOWN ON PLANS ARE NOT VERIFIED BY THE PUBLIC WORKS UTILITIES DIRECTOR.
4. ACCEPTANCE OF THE COMPLETED WORK WILL NOT BE GIVEN UNTIL 3 MIL MYLAR & CAD FORMAT DIGITAL 'AS-BUILT' PLANS ON CITY OF PRESCOTT SURVEY DATUM & COORDINATES HAVE BEEN SUBMITTED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER AND APPROVED BY THE PUBLIC WORKS DEPARTMENT.
5. CITY OF PRESCOTT PUBLIC WORKS UTILITIES SHALL BE NOTIFIED A MINIMUM OF 24 HOURS PRIOR TO THE START OF ANY WORK.
6. ALL WORK & MATERIALS WHICH DO NOT CONFORM TO THE SPECIFICATIONS ARE SUBJECT TO REMOVAL & REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
7. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE OF THE CITY INSPECTOR OR HIS REPRESENTATIVE IS SUBJECT TO REMOVAL & REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
8. THE CONTRACTOR SHALL PROVIDE SUFFICIENT MEN, EQUIPMENT, & MATERIAL ON THE JOB AT ALL TIMES DURING CONSTRUCTION TO COMPLY WITH SPECIFICATIONS & TO COMPLETE THE WORK.
9. CIP INSPECTION TO BE DONE BY THE CITY OF PRESCOTT PUBLIC WORKS DEPARTMENT OR THEIR REPRESENTATIVE. PRIVATE DEVELOPMENTS SHALL PROVIDE FOR INDEPENDENT 3RD PARTY INSPECTIONS.
10. CONTRACTOR TO NOTIFY PROJECT ENGINEER 72 HOURS (3 WORKING DAYS) IN ADVANCE OF CONSTRUCTION TO SCHEDULE CONSTRUCTION CONTROL STAKING.
11. THE CONTRACTOR IS TO UNCOVER ALL EXISTING LINES BEING TIED INTO AND VERIFY GRADES, MATERIAL, SIZE & ELEVATIONS BEFORE COMMENCING CONSTRUCTION & ORDERING MATERIALS.
12. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL UNDERGROUND PIPELINES, TELEPHONE & ELECTRICAL CONDUITS & STRUCTURES IN ADVANCE OF ANY CONSTRUCTION & OBSERVE ALL POSSIBLE PRECAUTIONS TO AVOID ANY DAMAGE TO SUCH. THE ENGINEER &/OR OWNER WILL NOT GUARANTEE ANY LOCATIONS AS SHOWN ON THESE PLANS, OR THOSE OMITTED FROM SAME.
13. CONTRACTOR SHALL NOTIFY 'BLUE STAKE' AT 1-800-STAKEIT (1-800-782-5348) AT LEAST 48 HOURS PRIOR TO CONSTRUCTION.
14. CONTRACTOR SHALL VERIFY ALL QUANTITIES SHOWN & MAKE HIS BID BASED UPON THOSE VERIFICATIONS. IF ANY DISCREPANCY IN QUANTITIES IS FOUND, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AS SUCH.
15. ARIZONA DEPARTMENT OF ENVIRONMENTAL QUALITY REQUIREMENTS MUST BE COMPLIED WITH.
16. ARIZONA DEPARTMENT OF ENVIRONMENTAL QUALITY REQUIREMENTS SHALL APPLY WHEN MORE STRINGENT THAN THE MAG OR CITY OF PRESCOTT STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
17. ALL PLANS SIGNED BY THE CITY ARE NULL & VOID ONE YEAR FROM DATE OF SIGNATURE IF CONSTRUCTION HAS NOT STARTED AND/OR IS NOT ACTIVELY PROGRESSING.
18. PROJECT CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING TRAFFIC CONTROL PLANS WHICH SHALL BE MADE A PART OF THE PLAN REVIEW REQUEST TO THE CITY ENGINEER FOR APPROVAL.
19. ALL WATER LINES & APPURTENANCES SHALL BE PROVIDED WITH TRACE WIRE PER CITY STANDARD DETAIL. TRACE WIRE SHALL BE SUBJECT TO A TRACEABILITY TEST, EASILY ACCESSIBLE, & ANY DEFICIENCIES SHALL BE CORRECTED PRIOR TO PAVING. THE TRACE WIRE SHOULD BE TESTED AND SUBMITTED AS A PACKAGE WITH THE TESTING PACKET.
20. WATER-SEWER SEPARATION SHALL BE PURSUANT TO AAC R-18-5-502C.
21. WATER MAINS SHALL BE SUBJECT TO A PRESSURE & LEAKAGE TEST IN ACCORDANCE WITH AWWA C-600 STANDARD.
22. WATER MAINS SHALL BE DISINFECTED IN ACCORDANCE WITH ADEQ ENGINEERING BULLETIN NO. 8 'DISINFECTION OF WATER SYSTEMS'.
23. OPERATION OF VALVES TO BE DONE BY CITY PERSONNEL ONLY.
24. DUCTILE IRON PIPE TO BE INSTALLED PER MANUFACTURER'S REQUIREMENTS. ALL MATERIALS USED IN THE INSTALLATION OF DUCTILE IRON PIPE SHALL BE PURSUANT TO AAC R-18-4.
25. ALL MATERIALS & PRODUCTS THAT COME INTO CONTACT WITH DRINKING WATER OR DRINKING WATER TREATMENT CHEMICALS MUST COMPLY WITH NSF STANDARD 61. ANY 'OR EQUAL' SUBSTITUTION SHALL ALSO MEET NSF STANDARD 61.
26. ALL TRENCHES & BEDDING SHALL BE PER COP DETAIL 200P & TECHNICAL SPECIFICATIONS.
27. ALL MATERIALS USED IN THE INSTALLATION OF WATER MAINS SHALL BE PURSUANT TO AAC R-18-4 & SHALL BE NSF APPROVED FOR POTABLE WATER.
28. ALL REVISIONS TO ORIGINAL PLANS MUST BE APPROVED BY THE PUBLIC WORKS DIRECTOR PRIOR TO CONSTRUCTION.
29. ALL DUCTILE IRON, COPPER, & BRASS FITTINGS SHALL BE ENCASED IN POLYETHYLENE PROTECTIVE WRAPPING IN ACCORDANCE WITH MAG SECTION 610.5 UNLESS COUNTERINDICATED BY GEOTECHNICAL CORROSIVITY TESTING OF BEDDING AND SHADING MATERIALS & APPROVED BY THE PUBLIC WORKS DIRECTOR.
30. WATER LINES SHALL BE INSTALLED WITH MECHANICAL RESTRAINTS WHERE JOINT RESTRAINTS IS REQUIRED.
31. WATER SERVICE INTERRUPTION NOTICES SHALL BE GIVEN TO AFFECTED RESIDENTS BY THE CONTRACTOR AT HIS EXPENSE. ADVANCE NOTIFICATION REQUIREMENTS MUST BE APPROVED BY THE PUBLIC WORKS DIRECTOR PRIOR TO SCHEDULING A SHUTDOWN.
32. WATER MAIN TAPS, SERVICE TAPS, SHUTDOWN REQUESTS, AND METER REQUESTS MUST BE INITIATED WITH THE CITY INSPECTOR A MINIMUM OF 5 WORKING DAYS IN ADVANCE.

COP STANDARD DETAIL

WATER PLAN GENERAL NOTES

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
103P

1. ALL WORK SHALL CONFORM TO MARICOPA ASSOCIATION OF GOVERNMENTS (MAG), & CITY OF PRESCOTT (COP) CONSTRUCTION STANDARDS & SPECIFICATIONS, WHICH ARE ON FILE IN THE OFFICE OF THE CITY ENGINEER.
2. ALL EXISTING FRAMES, COVERS, VALVE BOXES, & MANHOLES SHALL BE EITHER REPLACED OR ADJUSTED TO FINISH GRADE DEPENDING ON PLAN CALL OUT UPON COMPLETION OF PAVING, UTILITY, OR RELATED CONSTRUCTION.
3. ANY QUANTITIES SHOWN ON PLANS ARE NOT VERIFIED BY THE PUBLIC WORKS UTILITIES DIRECTOR.
4. ACCEPTANCE OF THE COMPLETED WORK WILL NOT BE GIVEN UNTIL 3 MIL MYLAR & CAD FORMAT DIGITAL 'AS-BUILT' PLANS ON CITY OF PRESCOTT SURVEY DATUM & COORDINATES HAVE BEEN SUBMITTED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER AND APPROVED BY THE PUBLIC WORKS DEPARTMENT.
5. CITY OF PRESCOTT PUBLIC WORKS UTILITIES SHALL BE NOTIFIED A MINIMUM OF 24 HOURS PRIOR TO THE START OF ANY WORK.
6. ALL WORK & MATERIALS WHICH DO NOT CONFORM TO THE SPECIFICATIONS ARE SUBJECT TO REMOVAL & REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
7. ANY WORK PERFORMED WITHOUT THE KNOWLEDGE OF THE CITY INSPECTOR OR HIS REPRESENTATIVE IS SUBJECT TO REMOVAL & REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
8. THE CONTRACTOR SHALL PROVIDE SUFFICIENT MEN, EQUIPMENT, & MATERIAL ON THE JOB AT ALL TIMES DURING CONSTRUCTION TO COMPLY WITH SPECIFICATIONS & TO COMPLETE THE WORK.
9. CIP INSPECTION TO BE DONE BY THE CITY OF PRESCOTT PUBLIC WORKS DEPARTMENT OR THEIR REPRESENTATIVE. PRIVATE DEVELOPMENTS SHALL PROVIDE FOR INDEPENDENT 3RD PARTY INSPECTIONS.
10. CONTRACTOR TO NOTIFY PROJECT ENGINEER 72 HOURS (3 WORKING DAYS) IN ADVANCE OF CONSTRUCTION TO SCHEDULE CONSTRUCTION CONTROL STAKING.
11. THE CONTRACTOR IS TO UNCOVER ALL EXISTING LINES BEING TIED INTO AND VERIFY GRADES, MATERIAL, SIZE & ELEVATIONS BEFORE COMMENCING CONSTRUCTION & ORDERING MATERIALS.
12. IT IS THE CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL UNDERGROUND PIPELINES, TELEPHONE & ELECTRICAL CONDUITS & STRUCTURES IN ADVANCE OF ANY CONSTRUCTION & OBSERVE ALL POSSIBLE PRECAUTIONS TO AVOID ANY DAMAGE TO SUCH. THE ENGINEER &/OR OWNER WILL NOT GUARANTEE ANY LOCATIONS AS SHOWN ON THESE PLANS, OR THOSE OMITTED FROM SAME.
13. CONTRACTOR SHALL NOTIFY 'BLUE STAKE' AT 1-800-STAKEIT (1-800-782-5348) AT LEAST 48 HOURS PRIOR TO CONSTRUCTION.
14. CONTRACTOR SHALL VERIFY ALL QUANTITIES SHOWN & MAKE HIS BID BASED UPON THOSE VERIFICATIONS. IF ANY DISCREPANCY IN QUANTITIES IS FOUND, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AS SUCH.
15. ARIZONA DEPARTMENT OF ENVIRONMENTAL QUALITY REQUIREMENTS MUST BE COMPLIED WITH.
16. ARIZONA DEPARTMENT OF ENVIRONMENTAL QUALITY REQUIREMENTS SHALL APPLY WHEN MORE STRINGENT THAN THE MAG OR CITY OF PRESCOTT STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION; MORE SPECIFICALLY WHERE THEY PERTAIN TO MAXIMUM ALLOWABLE SEWER LINE/PRESSURE SEWER LINE EXFILTRATION-INFILTRATION RATES.
17. ALL PLANS SIGNED BY THE CITY ARE NULL & VOID ONE YEAR FROM DATE OF SIGNATURE IF CONSTRUCTION HAS NOT STARTED AND/OR IS NOT ACTIVELY PROGRESSING.
18. PROJECT CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING TRAFFIC CONTROL PLANS WHICH SHALL BE MADE A PART OF THE PLAN REVIEW REQUEST TO THE CITY ENGINEER FOR APPROVAL.
19. WATER-SEWER SEPARATION SHALL BE PURSUANT TO AAC R-18-5-502C.
20. ALL TRENCHES & BEDDING SHALL BE PER COP DETAIL 200P & TECHNICAL SPECIFICATIONS.
21. ALL REVISIONS TO ORIGINAL PLANS MUST BE APPROVED BY THE PUBLIC WORKS DIRECTOR PRIOR TO CONSTRUCTION. ANY UNAPPROVED REVISIONS ARE SUBJECT TO REMOVAL & REPLACEMENT AT CONTRACTOR'S EXPENSE.
22. SEWER FORCE MAIN LINES SHALL BE DESIGNED AND CONSTRUCTED OF A MATERIAL SUITABLE FOR SANITARY SEWER PRESSURE PIPE AS APPROVED BY THE CITY ENGINEER. SEWER LINES SHALL BE PRESSURE TESTED TO A MINIMUM OF 50 PSI ABOVE DESIGN WORKING PRESSURE AT THE LOWEST POINT IN THE SYSTEM FOR A MINIMUM OF 4 HOURS IN ACCORDANCE WITH AAC R18-9.
23. SEWER LINE LOW PRESSURE AIR TESTS SHALL BE DONE ON 100% OF ALL LINES AFTER PLACEMENT OF BACKFILL TO PAVEMENT SUBGRADE. TEST EACH SEGMENT OF THE SEWER LINE FOR LEAKAGE USING THE APPLICABLE METHOD BELOW AND RECORD THE RESULTS:
 - 23A. "STANDARD TEST METHOD FOR INSTALLATION OF ACCEPTANCE OF PLASTIC GRAVITY SEWER LINES USING LOW-PRESSURE AIR, F1417-92(1998)" PUBLISHED BY THE AMERICAN SOCIETY FOR TESTING AND MATERIALS.
24. SEWER MANHOLES EXFILTRATION TESTS SHALL BE DONE ON 100% OF ALL MANHOLES. VACUUM TESTING IN ACCORDANCE WITH CITY STANDARDS MAY BE USED IN LIEU OF EXFILTRATION TEST. THE CONTRACTOR SHALL TEST EACH MANHOLE USING ONE OF THE FOLLOWING TEST PROTOCOLS:
 - 24A. WATERTIGHTNESS TESTING BY FILLING THE MANHOLE WITH WATER. THE CONTRACTOR SHALL ENSURE THAT THE DROP IN WATER LEVEL FOLLOWING PRESOAKING DOES NOT EXCEED 0.00034 OF THE TOTAL MANHOLE VOLUME PER HOUR.
 - 24B. NEGATIVE AIR PRESSURE TESTING USING THE "STANDARD TEST METHOD FOR CONCRETE SEWER MANHOLES BY NEGATIVE AIR PRESSURE" (VACUUM) TEST, C1244-02e1(2002), PUBLISHED BY THE AMERICAN SOCIETY FOR TESTING AND MATERIALS. THIS MATERIAL IS INCORPORATED BY REFERENCE & DOES NOT INCLUDE ANY LATER AMENDMENTS OR EDITIONS OF THE INCORPORATED MATERIAL, & MAY BE VIEWED AT THE ARIZONA DEPARTMENT OF ENVIRONMENTAL QUALITY, 1110 W. WASHINGTON, PHOENIX, AZ. 85007, OR OBTAINED FROM THE AMERICAN SOCIETY FOR TESTING & MATERIALS INTERNATIONAL, 100 BAR HARBOR DRIVE, WEST CONSHOHOCKEN, PA. 19428-2959.
25. SEWER LINE DEFLECTION TESTS WITH AN APPROPRIATELY SIZED MANDREL SHALL BE DONE ON 100% OF ALL NON-RIGID PIPE LINES.
26. THE CONTRACTOR SHALL PERFORM A VISUAL INSPECTION OF ALL SEWER MAINS AND LATERALS INSTALLED WITHIN THE CITY'S COLLECTION SYSTEM UTILIZING A SEWER CCTV SYSTEM AFTER COMPLETE BACKFILL AND COMPACTION BUT BEFORE INSTALLING ANY PORTION OF THE PAVEMENT STRUCTURAL SECTION. THE INSPECTION SHALL COMPLY WITH THE CITY'S VIDEO ACCEPTANCE PROCEDURE. THE CONTRACTOR SHALL PROVIDE 72 HOURS ADVANCE NOTICE FOR CITY STAFF TO BE PRESENT DURING THE VIDEO OPERATION AND SHALL PROVIDE THE CITY A VIDEO DVD AND HARD COPY OF THE INSPECTION REPORT UPON COMPLETION.
27. COVER EACH SEWER LINE WITH AT LEAST 3 FEET OF EARTH COVER MEETING THE REQUIREMENTS "TRENCH EXCAVATION, BACKFILLING, & COMPACTION" (SECTION 601) REVISED 2004, PUBLISHED BY THE MARICOPA ASSOCIATION OF GOVERNMENTS; & "RIGID PIPE BEDDING FOR SANITARY SEWERS" (WMM 104) REVISED JULY 2002, PUBLISHED BY PIMA COUNTY WASTEWATER MANAGEMENT.
28. PRESSURE SEWER MAINS AND SERVICE LATERALS (LPS) SHALL BE SUBJECT TO A PRESSURE & LEAKAGE TEST IN ACCORDANCE WITH AWWA-C-600 STANDARD. TEST PRESSURE SHALL BE A MINIMUM OF 100 PSI, OR 50 PSI OVER WORKING PRESSURE, WHICHEVER IS GREATER. TESTING SHALL BE DONE AFTER BACKFILL TO SUBGRADE.

1. ALL GRADING SHALL CONFORM TO THE CURRENT CITY ADOPTED EDITION OF THE INTERNATIONAL BUILDING CODE, AND CITY OF PRESCOTT LAND DEVELOPMENT CODE (REFERENCE CITY OF PRESCOTT STANDARD SECTIONS).
2. ALL PROVISIONS OF THE PRELIMINARY SOILS REPORT PREPARED BY _____, DATED _____ SHALL BE COMPLIED WITH DURING OPERATIONS.
3. THIS PLAN IS FOR GRADING PURPOSES ONLY. APPROVAL OF THIS PLAN DOES NOT CONSTITUTE APPROVAL OF DRIVEWAY LOCATIONS OR SIZES, PARKING LOT LAYOUT, SEWER AND WATER FACILITIES, BUILDING LOCATIONS, OFF-SITE DRAINAGE FACILITIES OR OTHER ITEMS NOT RELATED DIRECTLY TO THE BASIC GRADING OPERATION.
4. CERTIFICATION FROM THE REGISTERED CIVIL ENGINEER AND SOILS/GEOLOGICAL ENGINEER STATING THAT THE ROUGH GRADING HAS BEEN COMPLETED PER THE APPROVED PLAN, AND A COMPACTION REPORT FROM THE SOILS ENGINEER ON ANY FILL AREAS THAT ARE REQUIRED SHALL BE PROVIDED PRIOR TO BUILDING PERMITS BEING ISSUED.
5. PARTIES NAMED ON ADEQ'S NOTICE OF INTENT (N.O.I.) ARE RESPONSIBLE FOR EROSION, DUST, MUD, SILT, DEBRIS, AND TEMPORARY DRAINAGE CONTROL DURING GRADING OPERATIONS AND MAY BE REQUIRED TO PROVIDE A SWPPP.
6. ANY ON-SITE RETAINING WALLS WILL REQUIRE APPROVAL AS PART OF THESE PLANS. ANY NECESSARY RETAINING WALLS ON THE PERIMETER OF THIS SITE MAY BE REQUIRED TO BE IN PLACE AND APPROVED BY THE CITY BUILDING DEPARTMENT PRIOR TO THE START OF GRADING. A SEPARATE PLAN WITH REQUIRED STRUCTURAL CALCULATIONS MAY BE REQUESTED FOR RETAINING WALLS.
7. ANY INFRASTRUCTURE CONSTRUCTED IN THE PUBLIC RIGHT OF WAY WILL REQUIRE SEPARATE PLAN APPROVAL AND INSPECTION FROM THE CITY ENGINEER.
8. ANY WALLS, FENCES, STRUCTURES AND/OR APPURTENANCES ADJACENT TO THIS PROJECT SHALL BE PROTECTED IN PLACE. IF GRADING OPERATIONS DAMAGE OR ADVERSELY AFFECT SAID ITEMS IN ANY WAY, THE CONTRACTOR AND/OR DEVELOPER IS RESPONSIBLE FOR WORKING OUT AN ACCEPTABLE SOLUTION TO THE SATISFACTION OF THE AFFECTED PROPERTY OWNER(S).
9. THE CONTRACTOR/DEVELOPER IS RESPONSIBLE FOR ENSURING THAT RETAINING WALLS DO NOT INTERFERE WITH PROVISION OF UTILITIES. WALLS MUST BE CONSTRUCTED ON SITE AND OUTSIDE OF THE RIGHT OF WAY. THIS SHALL INCLUDE THE FOOTINGS.
10. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT COMPACTION HAS BEEN ATTAINED ON THE ENTIRE GRADING SITE IN ACCORDANCE WITH THE GENERAL ENGINEERING PLAN, INCLUDING FILL AREAS OUTSIDE THE BUILDING PADS AND ON ALL FILL SLOPES, AND SHALL BE CERTIFIED BY THE SOIL'S ENGINEER.
11. CITY APPROVAL OF PLANS DOES NOT RELIEVE THE DEVELOPER FROM THE RESPONSIBILITY FOR CORRECTION OR ERROR OR OMISSION DISCOVERED DURING CONSTRUCTION. UPON REQUEST, THE REQUIRED PLAN REVISIONS SHALL BE PROMPTLY SUBMITTED TO THE CITY ENGINEER FOR APPROVAL.
12. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CALL THE CITY ENGINEER'S OFFICE AT (928) 777-1140 FOR ANY REQUIRED CIVIL INSPECTION 24 HOURS PRIOR TO PERFORMING ANY WORK. WORK PERFORMED WITHOUT CALLING FOR INSPECTION MAY BE REJECTED AND, IF REJECTED, SHALL BE REMOVED SOLELY AT THE CONTRACTOR'S EXPENSE.
13. NO GRADING SHALL COMMENCE WITHOUT OBTAINING A GRADING PERMIT AND NOTIFYING THE CITY OF PRESCOTT OR DEVELOPER'S GRADING INSPECTOR TO SCHEDULE A PREGRADING MEETING TWO WORKING DAYS PRIOR TO THE START OF WORK.
14. PRIOR TO THE START OF GRADING ALL SWPPP MEASURES SHALL BE IN PLACE, ALL DEBRIS, INCLUDING EXISTING STRUCTURES, FOOTINGS, FOUNDATIONS AND RUBBLE SHALL BE REMOVED FROM THE SITE TO THE SATISFACTION OF THE SOILS ENGINEER.
15. AFTER REMOVAL OF DEBRIS, ANY EXISTING FILL OR DISTURBED NATURAL SOILS SHALL BE EXCAVATED TO THE SATISFACTION OF THE SOILS ENGINEER.
16. THE EXPOSED SOILS SHALL THEN BE INSPECTED BY THE SOILS ENGINEER, AND ANY ADDITIONAL OVER-EXCAVATION SHALL THEN BE MADE IN ACCORDANCE WITH THE SOILS ENGINEER'S RECOMMENDATIONS AND AS CONTAINED IN THE SOIL'S REPORT.
17. THE EXPOSED SOILS SHALL THEN BE SCARIFIED TO PROVIDE A BOND WITH NEW FILL, BROUGHT TO PROPER MOISTURE CONTENT AND COMPACTED TO AT LEAST 90% OF THE MAXIMUM DENSITY, AS DETERMINED BY ASTM D1557-78 OR EQUIVALENT COMPACTION SHALL BE OBTAINED BY METHODS SPECIFIED BY THE SOILS ENGINEER. ROAD PRISM SUBGRADE SHALL BE COMPACTED TO AT LEAST 95% STANDARD OR MODIFIED PER SOILS ENGINEER'S RECOMMENDATIONS.
18. THE SOILS AND DESIGN ENGINEER OF RECORD SHALL ALSO BE RESPONSIBLE TO INSPECT, VERIFY AND REPORT THAT PROPER COMPACTION HAS BEEN OBTAINED BY EARTHWORK CONTRACTOR OR SUBCONTRACTOR AND PRIVATE UTILITY FRANCHISES CONCERNING UTILITY LINE BACKFILL, TO INCLUDE ELECTRICAL, GAS, CABLE, FIBEROPTIC AND LANDSCAPE IRRIGATION LINES. ADDITIONALLY, WATER AND SEWER LINES TO BE BACKFILLED AND COMPACTED IN ACCORDANCE WITH GENERAL ENGINEERING REQUIREMENTS SECTION AND DETAIL.
19. AN AS-GRADED GRADING PLAN AND THE CERTIFICATION OF COMPLIANCE FORMS FOR SAID GRADING PLAN WITH THE PROPER STAMPS AND SIGNATURES ARE TO BE SUBMITTED TO THE CITY ENGINEER PRIOR TO RELEASE OF GRADING BOND AND PRIOR TO FINAL GRADING INSPECTION. BUILDING PAD CERTIFICATION SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT WHEN REQUESTED.
20. NO FILL SHALL BE PLACED UNTIL STRIPPING OF VEGETATION, REMOVAL OF UNSUITABLE SOILS, AND INSTALLATION OF SUBDRAINS (IF ANY) HAVE BEEN INSPECTED AND APPROVED BY THE SOILS ENGINEER.
21. ENGINEER MUST SET GRADE STAKES FOR ALL DRAINAGE DEVICES AND OBTAIN INSPECTION BEFORE POURING.
22. GRADING SHALL NOT BE STARTED WITHOUT FIRST NOTIFYING CITY PUBLIC WORKS INSPECTION DEPARTMENT. A PRE-GRADING MEETING ON THE SITE IS REQUIRED BEFORE BEGINNING GRADING ACTIVITIES BY THE FOLLOWING PEOPLE PRESENT: OWNER, GRADING CONTRACTOR, DESIGN CIVIL ENGINEER, SOIL ENGINEER/GEOLOGIST, PUBLIC WORKS INSPECTOR, AND WHEN REQUIRED, THE ARCHAEOLOGIST AND PALEONTOLOGIST. THE REQUIRED INSPECTIONS FOR GRADING WILL BE EXPLAINED AT THE PRE-CONSTRUCTION MEETING.
23. ALL EXISTING FILLS SHALL BE APPROVED AND CERTIFIED BY THE SOILS ENGINEER OR REMOVED PRIOR TO PLACING ADDITIONAL FILLS.
24. ALL TRENCH BACKFILLS SHALL BE TESTED AND APPROVED BY THE SOIL ENGINEER.
25. THE COMPACTION REPORT AND APPROVAL FROM THE SOIL ENGINEER SHALL INDICATE THE TYPE OF FIELD TESTING PERFORMED. EACH TEST SHALL BE IDENTIFIED WITH THE METHOD OF OBTAINING THE IN-PLACE DENSITY, WHETHER SAND CONE OR NUCLEAR GAUGE, AND SHALL BE SO NOTED FOR EACH TEST.
26. EXPORT SOIL MUST BE TRANSPORTED TO A LEGAL DUMP OR TO A PERMITTED SITE SHOWN CLEARLY ON APPROVED PLANS.
27. ALL EXISTING DRAINAGE COURSES THROUGH THIS SITE SHALL REMAIN OPEN UNTIL FACILITIES TO HANDLE STORM WATER ARE APPROVED AND FUNCTIONAL; HOWEVER, IN ANY CASE, THE PERMITTEE SHALL BE HELD LIABLE FOR ANY DAMAGE DUE TO OBSTRUCTING NATURAL DRAINAGE PATTERNS.

COP STANDARD DETAIL

GRADING AND DRAINAGE
NOTES

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
105P-1

1. A COPY OF THE APPROVED GRADING AND DRAINAGE PLAN FOR THIS PROJECT AND EROSION AND SEDIMENT CONTROL (ESC) PLAN OR STORM WATER POLLUTION PREVENTION PLAN (SWPPP) SHALL BE MAINTAINED ON THE SITE AND AVAILABLE FOR REVIEW. THOSE ELEMENTS OF THE GRADING AND DRAINAGE PLAN PERTINENT TO OR REFERENCED ON THE SWPPP SHALL BE CONSIDERED A PART OF THE SWPPP.
2. THE ESC/SWPPP AND RELATED RECORDS MUST BE MADE AVAILABLE UPON REQUEST TO ADEQ AND THE CITY OF PRESCOTT.
3. THE IMPLEMENTATION OF THESE PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND UPGRADING OF THESE FACILITIES IS THE RESPONSIBILITY OF THE PERMITTEE/CONTRACTOR UNTIL ALL CONSTRUCTION IS APPROVED AND A NOTICE OF TERMINATION HAS BEEN SUBMITTED.
4. THE SCHEMATIC EROSION CONTROL MEASURES SHOWN ON THE PLANS ARE A MINIMUM. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEANS TO PROTECT EXISTING FACILITIES AND ADJACENT PROPERTIES FROM NOISE, DUST, AND STORM WATER RUNOFF THROUGHOUT CONSTRUCTION OF THE PROJECT AND BUILDINGS ON LOTS, AND SHALL CONDUCT HIS OPERATIONS IN SUCH A MANNER THAT STORM WATER WILL BE CONTAINED ON SITE OR CHANNELED INTO A STORM DRAIN SYSTEM, PROVIDED THAT IT IS FREE FROM POLLUTANTS AND DEBRIS.
5. CONTRACTOR SHALL PERMANENTLY STABILIZE ALL DISTURBED SLOPES AS STATED ON APPROVED CONSTRUCTION PLANS. ALL EROSION CONTROL STRUCTURES SHALL REMAIN IN PLACE UNTIL EXPOSED SLOPES HAVE BEEN PERMANENTLY STABILIZED.
6. CONTRACTOR SHALL TAKE MEASURES TO PREVENT OR MINIMIZE THE GENERATION, EMISSION AND/OR TRANSPORT OF FUGITIVE DUST FROM CONSTRUCTION ACTIVITIES.
7. THIS PLAN SHALL BE IN EFFECT UNTIL ALL DISTURBED AREAS ARE PERMANENTLY STABILIZED, TRANSFERRED TO NEW OWNERSHIP, OR DEVELOPED UNDER FUTURE PLANS WITH A NEW NOTICE OF INTENT (NOI), SWPPP, AND PERMIT. ONCE THE CONSTRUCTION ACTIVITIES HAVE BEEN COMPLETED AND THE SITE HAS MET THE FINAL STABILIZATION REQUIREMENTS OF THE PERMIT, THE AUTHORIZED SITE REPRESENTATIVE MAY FILE A NOTICE OF TERMINATION (NOT) WITH ADEQ, WITH A COPY SUBMITTED TO THE CITY OF PRESCOTT ENGINEERING DIVISION TO TERMINATE COVERAGE UNDER THE PERMIT.
8. A CONCRETE WASHOUT SHALL BE INSTALLED FOR ALL PROJECTS THAT PROPOSE CONCRETE TO BE MIXED ON SITE OR BE DELIVERED FROM A BATCH PLANT. THE CONCRETE WASHOUT SHALL BE LOCATED A MINIMUM OF FIFTY (50) FEET FROM ANY DRAINAGE INFRASTRUCTURE OR NATURAL DRAINAGE FEATURES OR WATER BODIES AND INCORPORATE AN IMPERMEABLE LINER TO CONTAIN THE REQUIRED VOLUME. ALL DRIED CONCRETE WASTE SHALL BE BROKEN INTO MANAGEABLE PIECES AND DISPOSED OF OFF-SITE AT AN APPROVED FACILITY.

1. THE CONTRACTOR SHALL SPOT LAYOUT THE ENTIRE PROJECT AND CONTACT THE CITY INSPECTOR TO MAKE ARRANGEMENTS FOR INSPECTION PRIOR TO INSTALLING TRAFFIC SIGNS OR PAVEMENT MARKINGS. ANY SIGNING OR STRIPING INSTALLED BEFORE LAYOUT APPROVAL SHALL BE SUBJECT TO REMOVAL AND REINSTALLATION AT THE CONTRACTOR'S EXPENSE.
2. TRAFFIC SIGN DIMENSIONS, COLORS AND LETTERING SHALL CONFORM TO THE LATEST MUTCD SPECIFICATIONS. TRAFFIC SIGN SIZE SHALL BE STANDARD UNLESS OTHERWISE SPECIFIED ON THE PLANS.
3. SIGN LOCATION SHALL BE COORDINATED WITH LANDSCAPING PLANS TO ENSURE SIGN VISIBILITY PER AASHTO STANDARDS.
4. ALL R1-1 "STOP" SIGNS AND PEDESTRIAN WARNING SIGNS SHALL BE RETRO-REFLECTIVE WITH SHEETING MATERIAL TO BE DIAMOND VIP GRADE, MEETING OR EXCEEDING ASTM 4956-04.
5. ALL OTHER SIGNS ARE TO BE RETRO-REFLECTIVE WITH SHEETING MATERIAL TO BE HIGH INTENSITY PRISMATIC MEETING OR EXCEEDING ASTM 4956-04.
6. SIGN BLANKS SHALL BE 5052-H38 ALLOY TREATED ALUMINUM WITH ALODINE 1200 CONVERSION COATING, 0.080" THICK WITH ROUNDED CORNERS.
7. SIGNS SHALL BE MOUNTED ON STREET LIGHT POLES WHENEVER FEASIBLE.
8. STRIPING SHALL CONFORM TO THE MOST RECENT EDITION OF THE MUTCD WITH REGARD TO SIZE, COLOR, REFLECTIVITY AND PLACEMENT UNLESS OTHERWISE SPECIFIED ON THE PLANS.
9. ALL THERMOPLASTIC APPLICATIONS SHALL CONFORM TO ADOT SPECIFICATION 704. TRANSVERSE MARKINGS, SYMBOLS AND LEGENDS SHALL BE 90 MIL (0.090 INCH) THICK, LONGITUDINAL MARKINGS SHALL BE 60 MIL (0.060 INCH) THICK ALKYD EXTRUDED THERMOPLASTIC.
10. ALL PAINT APPLICATION SHALL CONFORM TO ADOT SPECIFICATION 708.
11. ALL CONFLICTING STRIPING, PAVEMENT MARKINGS, AND CURB PAINT SHALL BE REMOVED BY WET SANDBLASTING OR OTHER APPROVED METHOD PRIOR TO THE INSTALLATION OF NEW STRIPING. SLURRY OR PAINT SHALL NOT BE USED TO COVER EXISTING PAINT. PAVEMENT THAT IS DAMAGED DUE TO THE REMOVAL OF MARKERS OR STRIPING SHALL BE REPAIRED TO THE SATISFACTION OF THE CITY ENGINEER OR HIS DESIGNEE.

COP STANDARD DETAIL

SIGNING AND
STRIPING NOTES

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
106P-1

1. ALL MATERIAL, EQUIPMENT AND INSTALLATION SHALL CONFORM TO THE LATEST MUTCD SPECIFICATIONS; THE ADOT TRAFFIC SIGNAL AND LIGHTING STANDARD DRAWINGS, ADOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND THE CITY OF PRESCOTT STANDARD DETAILS.
2. ALL EQUIPMENT SHALL BE APPROVED BY THE CITY OF PRESCOTT THROUGH THE ELECTRICAL EQUIPMENT SUBMITTAL PROCESS PRIOR TO THE ORDERING OF EQUIPMENT.
3. THE LOCATION OF EACH NEW POLE FOUNDATION, PULLBOX, CONTROLLER CABINET FOUNDATION, UPS CABINET FOUNDATION AND ELECTRICAL SERVICES PEDESTAL FOUNDATION SHALL BE MARKED IN THE FIELD AS SHOWN ON THE PLANS. THE EXACT LOCATION SHALL BE APPROVED BY THE CITY TRAFFIC ENGINEER AND/OR APPROVED CITY REPRESENTATIVE BY CONTACTING (928)777-1130 PRIOR TO WORK.
4. ALL VEHICLE AND PEDESTRIAN COUNTDOWN INDICATIONS SHALL BE LED.
5. PEDESTRIAN COUNTDOWN HEADS SHALL BE PROVIDED AT ALL VEHICULAR SIGNAL LOCATIONS WHENEVER SIDEWALK CONNECTIONS EXIST OR ARE INSTALLED.
6. ALL PEDESTRIAN PUSH BUTTON ASSEMBLIES SHALL CONFORM TO ADOT STANDARD DRAWINGS TS11-1 EXCEPT THAT THE PUSH BUTTON SHALL BE A MINIMUM OF TWO INCH IN DIAMETER. PUSH BUTTONS MUST MEET ADA REQUIREMENTS AND BE MOUNTED AT ADA HEIGHTS. ACCESSIBLE PEDESTRIAN SIGNAL SYSTEMS SHALL BE REQUIRED WHEN DIRECTED BY THE CITY TRAFFIC ENGINEER AND/OR APPROVED CITY REPRESENTATIVE.
7. CONTROLLER CABINET SHALL BE TYPE IV ECONOLITE TS2, TYPE 1 WITH ELEVATOR BASE. PROVISION FOR BATTERY BACK-UP SHALL BE PROVIDED IN ALL TRAFFIC SIGNAL CABINETS.
8. METER PEDESTAL CABINET SHALL BE MYERS PBM 2000 OR 1250 UPS W/FOUNDATION OR APPROVED EQUAL.
9. TRAFFIC SIGNAL PULL BOXES SHALL BE NO. 7 OR NO. 5 AS CALLED FOR ON THE PLANS AND CONFORM TO THE TS 1-4, 1-5 AND 1-5 OF THE ADOT TRAFFIC SIGNALS AND LIGHTING STANDARD DRAWINGS. BOX LIDS SHALL BE LOCKING AND LABELED WITH "TRAFFIC SIGNAL" UNLESS OTHERWISE SPECIFIED BY THE CITY TRAFFIC ENGINEER AND/OR THE APPROVED CITY REPRESENTATIVE.
10. ALL CONDUITS SHALL BE SCHEDULE 40 PVC, OF A DIAMETER AS CALLED FOR ON THE APPROVED PLANS.
11. LOOP DETECTOR SHALL BE SIX (6') FEET BY FIFTY (50') FEET QUADRUPOLE UNLESS OTHERWISE SPECIFIED BY THE CITY TRAFFIC ENGINEER AND/OR THE APPROVED CITY REPRESENTATIVE. ALL VEHICLE DETECTION LOOP CABLES SHALL BE #14 AWG IMSA 51-5-1985 CABLE. LEAD-IN CABLES SHALL BE #14 AWG IMSA 50-2-1984 CABLE. NO SPLICE SHALL BE ALLOWED IN THE DETECTION LOOP CABLE EXCEPT AT THE PULLBOX ADJACENT TO THE LOOP. THE DETECTOR LEAD-IN SHALL NOT BE SPLICED.
12. THE TOP OF THE POLE FOUNDATION SHALL BE LEVEL WITH THE FINISHED GRADE. IF THE SLOPE OR SHOULDER DROPS OFF FROM FINISHED GRADE, THE CONTRACTOR SHALL GRADE AROUND THE POLE FOUNDATION. THE TOP OF THE FOUNDATION SHALL EXTEND NO MORE THAN 4 INCHES ABOVE THE ADJACENT ULTIMATE GRADE.
13. ALL CONCRETE USED FOR TRAFFIC SIGNAL POLE AND CABINET FOUNDATIONS SHALL BE CLASS "A".
14. THE CONTRACTOR SHALL CONTACT THE CITY TRAFFIC ENGINEER AND/OR THE APPROVED CITY REPRESENTATIVE TO ARRANGE FOR METER AND ELECTRICAL SERVICE CONNECTION FROM ARIZONA PUBLIC SERVICE (APS). THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING AND INSTALLING THE SERVICE CONDUIT RUN FROM THE POWER COMPANY SOURCE TO THE SIGNAL METER PEDESTAL.
15. EMERGENCY VEHICLE PREEMPTION SHALL BE INSTALLED AT ALL TRAFFIC SIGNAL INTERSECTIONS. THE CONTRACTOR SHALL PROVIDE AND INSTALL AN OPTICOM 700 SERIES SYSTEM OR APPROVED EQUAL TO INCLUDE, SENSORS, PROCESSORS, CONFIRMATION AND CONTROLLER HARDWARE, MOUNTING HARDWARE, INTERFACE CABLES, OPTICAL CABLES, AND ANY OTHER EQUIPMENT REQUIRED FOR A FULLY FUNCTIONING PRE-EMPTION SYSTEM.
16. CONTRACTOR SHALL PERFORM A GROUND RESISTANCE TEST FOR EACH INSTALLED GROUND ROD AND POLE FOUNDATION GROUNDING COIL IN ACCORDANCE WITH ADOT SPEC. 723-3.03.
17. CONTRACTOR SHALL BAG ALL NEWLY INSTALLED VEHICULAR AND/OR PEDESTRIAN TRAFFIC SIGNAL HEADS WITH BURLAP OR OTHER APPROVED MATERIAL UNTIL FINAL INSPECTION AND ACCEPTANCE BY THE CITY TRAFFIC ENGINEER AND/OR THE APPROVED CITY REPRESENTATIVE.
18. CONTRACTOR SHALL REPLACE ALL LANDSCAPING AND/OR IRRIGATION FACILITIES THAT MAY BE DISTURBED OR DAMAGED DURING TRAFFIC SIGNAL CONSTRUCTION AT HIS EXPENSE CONTACT THE PROPERTY OWNER FOR INFORMATION ON THE LOCATION OF IRRIGATION EQUIPMENT.
19. CONTRACTOR SHALL PROVIDE AND INSTALL "TRAFFIC CONTROL CHANGE" SIGNS WITH FLAGS FOR 30 DAYS FOLLOWING TURN-ON.
20. CONTRACTOR SHALL RETURN ALL REMOVED/UNUSED TRAFFIC SIGNAL EQUIPMENT TO THE CITY OF PRESCOTT PUBLIC WORKS DEPARTMENT. FOR ADDITIONAL INFORMATION, PLEASE CONTACT THE TRAFFIC SIGNAL SUPERVISOR AT (928)777-1683
21. CONTRACTOR SHALL PROVIDE A SIX-FOOT COILED CONTROL CORD IN THE POLICE PANEL OF THE CONTROLLER CABINET.

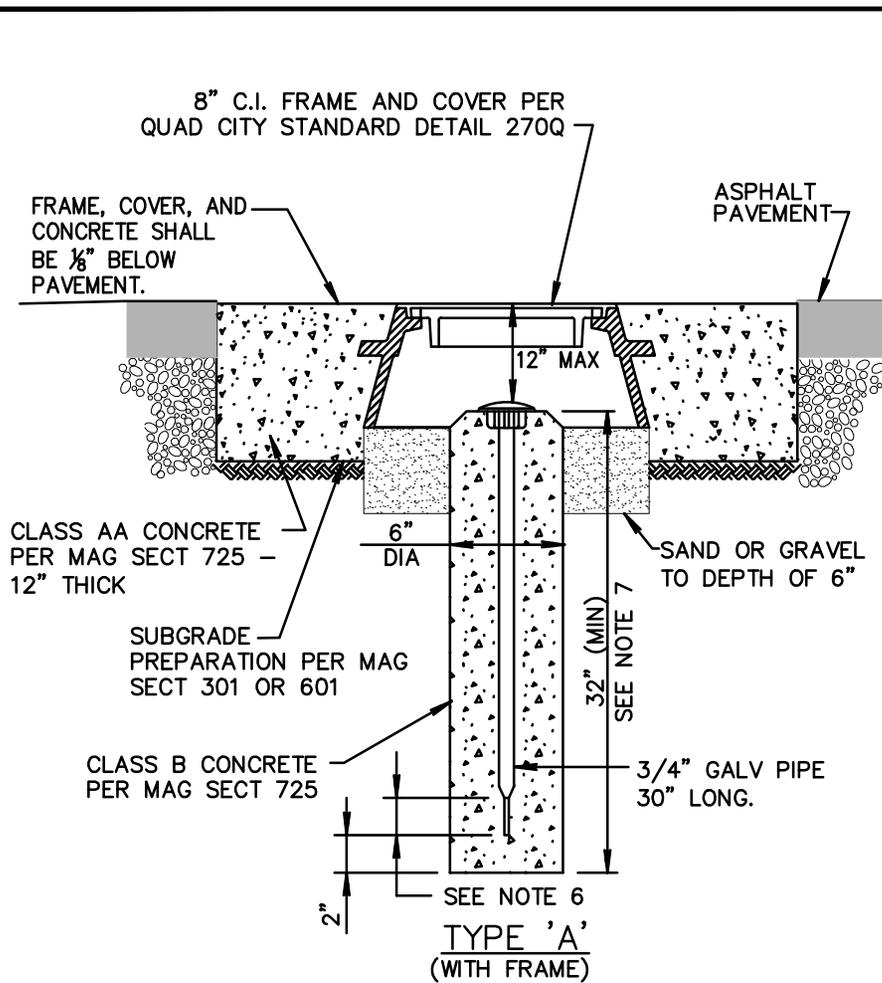
COP STANDARD DETAIL

TRAFFIC SIGNAL NOTES

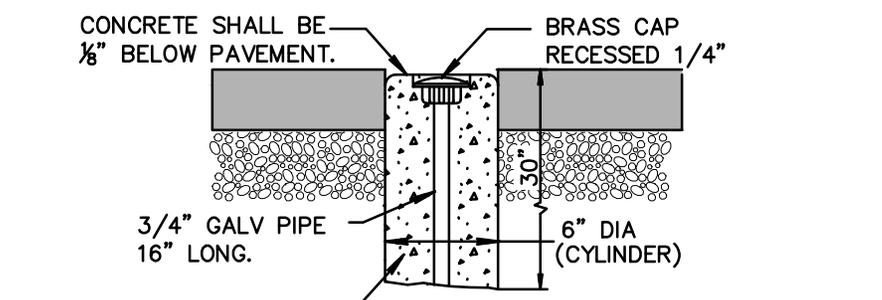
Charles Andrews
CITY ENGINEER

REVISED:
07/16

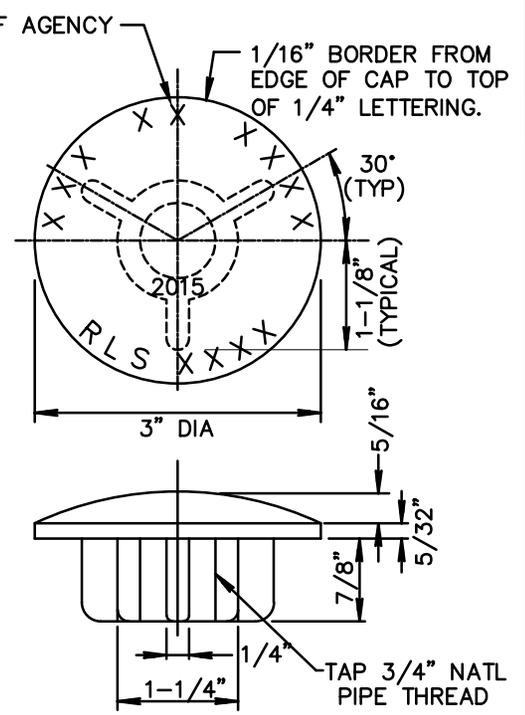
DETAIL No.
106P-2



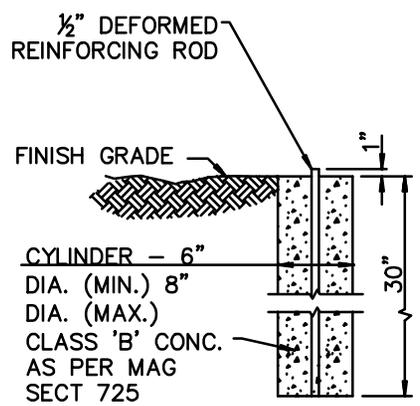
TYPE 'A'
(WITH FRAME)



TYPE 'B'
(WITHOUT FRAME)



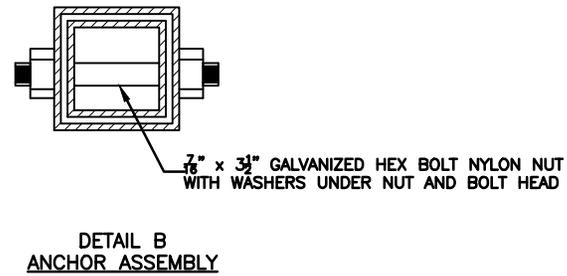
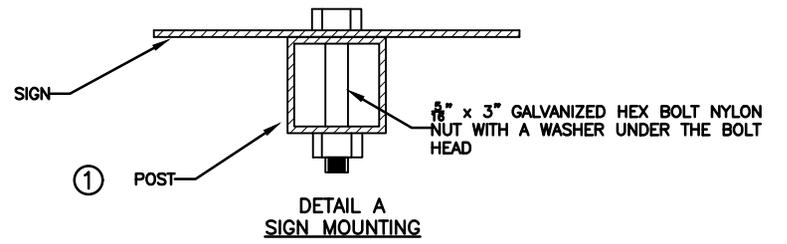
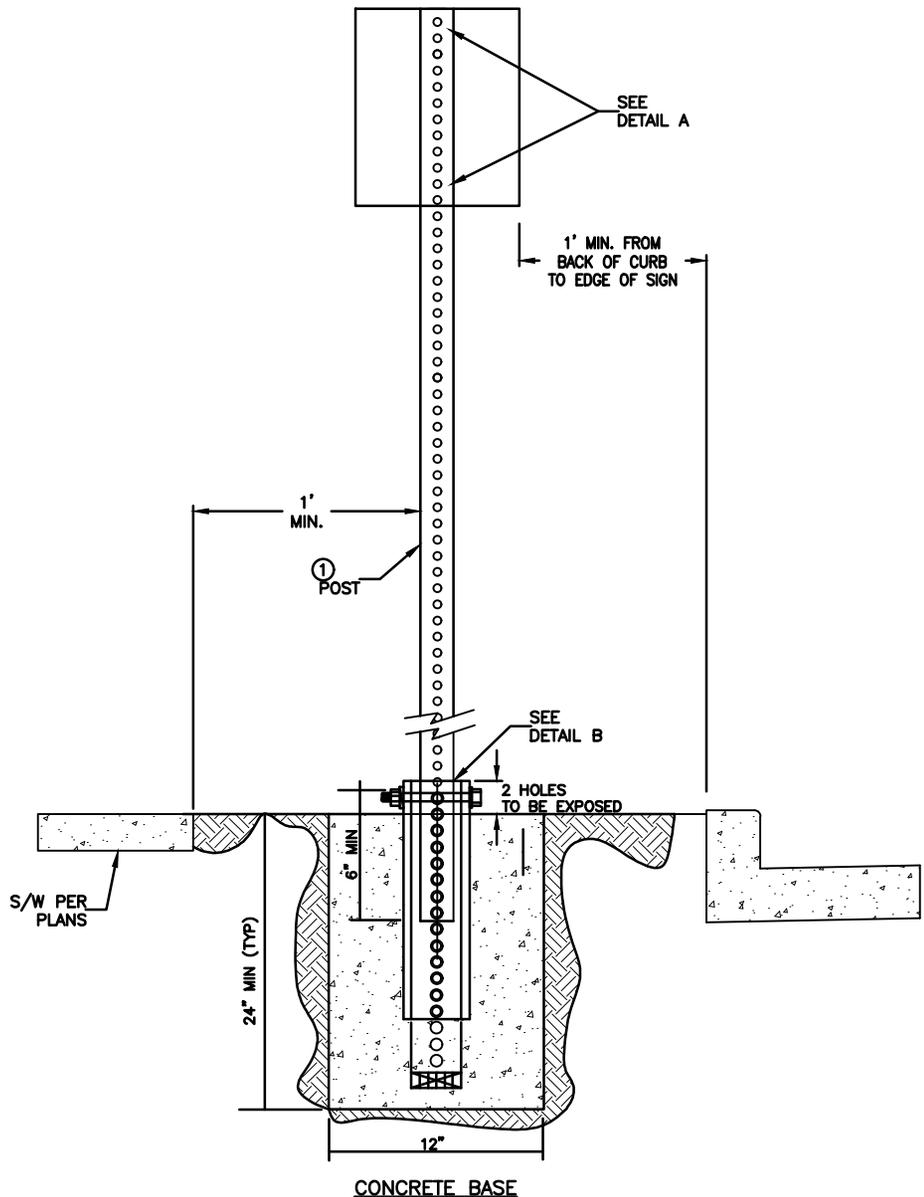
CAP DETAIL



TYPE 'C'

NOTES:

1. TYPE "A" TO BE USED AT INTERSECTION OF STREET CENTERLINES; SECTION AND ALIQUOT SECTION CORNERS; ANGLE POINTS; PC'S AND PT'S OF CURVES; LYING WITHIN ARTERIAL AND COLLECTOR STREETS.
2. TYPE "B" TO BE USED AT INTERSECTION OF STREET CENTERLINES; SECTION AND ALIQUOT SECTION CORNERS; ANGLE POINTS; PC'S AND PT'S OF CURVES; SUBDIVISION CORNERS & CHANGES IN ALIGNMENT OF SUBDIVISION BOUNDARIES; LYING WITHIN STREETS NOT DESIGNATED ARTERIAL OR COLLECTOR.
3. TYPE "C" TO BE USED AT CORNERS OF AND CHANGES IN ALIGNMENT OF SUBDIVISION BOUNDARIES; LYING IN UNPAVED AREAS.
4. CAP TO BE CONSTRUCTED OF RED BRASS OR BRONZE.
5. LETTERS TO BE APPROX. 1/32" WIDE & 1/32" DEEP.
6. FLATTENING THE BOTTOM 2" OF THE GALVANIZED PIPE IS OPTIONAL.
7. TOP OF CONCRETE POST IS CHAMFERED 3/4" EXCEPT WHEN SET FLUSH WITH PAVEMENT.
8. THE CAP SHALL SHOW THE POINT SURVEYED BY A PUNCH MARK OR SCRIBED CROSS AND THE CAP SHALL BE STAMPED WITH THE YEAR AND THE REGISTERED LAND SURVEYOR'S (RLS) REGISTRATION NUMBER.
9. WHEN APPLICABLE, THE CAP SHALL BE STAMPED WITH THE APPROPRIATE PUBLIC LAND SURVEY SYSTEM MARKING PER CURRENT MANUAL OF INSTRUCTIONS FOR THE SURVEY OF PUBLIC LANDS OF THE UNITED STATES, PREPARED BY THE BUREAU OF LAND MANAGEMENT.
10. SUBMIT TO THE AGENCY SURVEYOR A COPY OF THE RECORDED CORNER RECORD OR RESULTS OF SURVEY TO DOCUMENT COMPLIANCE WITH THE ARIZONA BOARD OF TECHNICAL REGISTRATION REQUIREMENTS.
11. THE MAXIMUM DISTANCE FROM TOP OF COVER TO TOP OF BRASS CAP SHALL NOT EXCEED 12 INCHES. SUBSEQUENT PAVEMENT LIFTS OR STREET CONSTRUCTION THAT CAUSES THIS DISTANCE TO BE EXCEEDED SHALL CAUSE THE STREET MONUMENT TO BE RECONSTRUCTED.
12. AT THE TIME OF CONSTRUCTION, THE BRASS CAP SHALL BE PLACED 6" BELOW THE COVER.



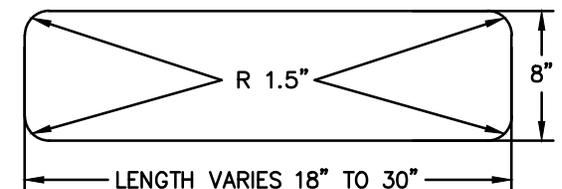
- NOTES:**
1. INSTALL ALL TRAFFIC SIGNS ON 12 GAUGE 2" SQUARE STEEL TUBING.
 2. CONCRETE BASE 2' DEEP MIN. x 12" WIDE, 18" LONG ANCHOR & 12" SLEEVE COMPLETELY TAPED TO PREVENT SEEPAGE OF CONCRETE.
 3. POST ANCHOR SHALL HAVE 2 HOLES EXPOSED AT FINISHED GRADE.
 4. ALL TRAFFIC SIGNS, WITH THE EXCEPTION OF R6-1 & DELINEATORS, SHALL BE SET AT A HEIGHT OF 7' TO BOTTOM OF SIGN. POSTS WITH DUAL SIGN ASSEMBLIES SHALL BE SET AT A HEIGHT OF 6' TO BOTTOM OF SIGN. DELINEATORS SHALL BE MOUNTED AT A MINIMUM OF 4' TO THE BOTTOM OF THE SIGN. ALTERNATE HEIGHTS MUST BE APPROVED BY THE TRAFFIC ENGINEER PRIOR TO INSTALLATION.
 5. BOLT FOR THE BASE TO BE PERPENDICULAR TO THE FLOW OF TRAFFIC.

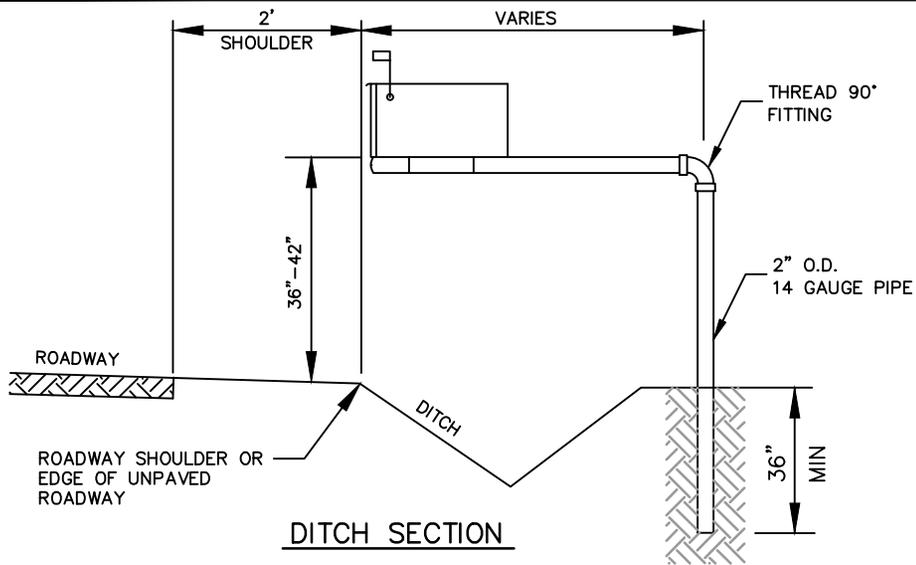


NOTES:

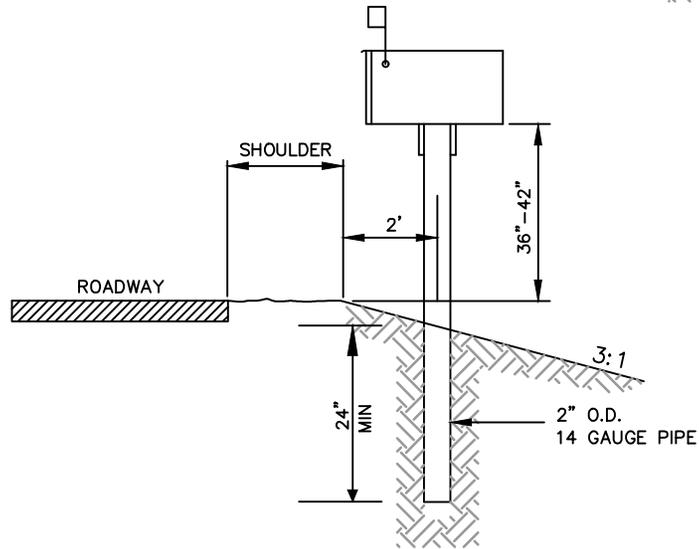
1. ALL REFLECTIVE SHEETING MATERIAL(S) SHALL BE HIGH INTENSITY PRISMATIC.
2. ALL TRANSPARENT ACRYLIC, PRESSURE-SENSITIVE FILM SHALL BE 3M #177 GREEN ELECTRO CUT FILM OR APPROVED EQUAL.
3. LETTER FONT SHALL BE UPPERCASE AND LOWERCASE FONT HWY C OR SIMILAR.
4. SEE DETAIL "A" FOR 8" STREET SIGN BLANK DIMENSIONS.
5. THESE SIGNS ARE CONSTRUCTED BY APPLYING WHITE HIP SHEETING TO THE ENTIRE BLANK. ON TOP OF THIS SHEETING A GREEN TRANSLUCENT PRESSURE-SENSITIVE FILM FROM WHICH THE LEGEND HAS BEEN CUT AND REMOVED IS APPLIED. THUS THE GREEN BACKGROUND IS APPLIED ON TOP OF THE WHITE SHEETING RESULTING IN A SIGN WITH A WHITE LEGEND AND A GREEN BACKGROUND.
6. SIGN BLANKS SHALL BE 5052-H38 ALLOY TREATED ALUMINUM WITH ALODINE 1200 CONVERSION COATING.
7. SIGN BLANK SHALL BE 0.100" THICK WITH ROUNDED CORNERS AS NOTED.
8. BLOCK NUMBERS ARE IN INCREMENTS OF 100 GOING UP IN NUMERICAL VALUE.
9. SIGNS THAT ARE LOCATED IN THE CENTER OF THE BLOCK SHALL HAVE "< ->".
10. SIGNS THAT ARE AT THE BEGINNING AND END OF THE BLOCK HAVE "->" POINTING UP THE BLOCK.

DETAIL "A" - BLANK DIMENSIONS





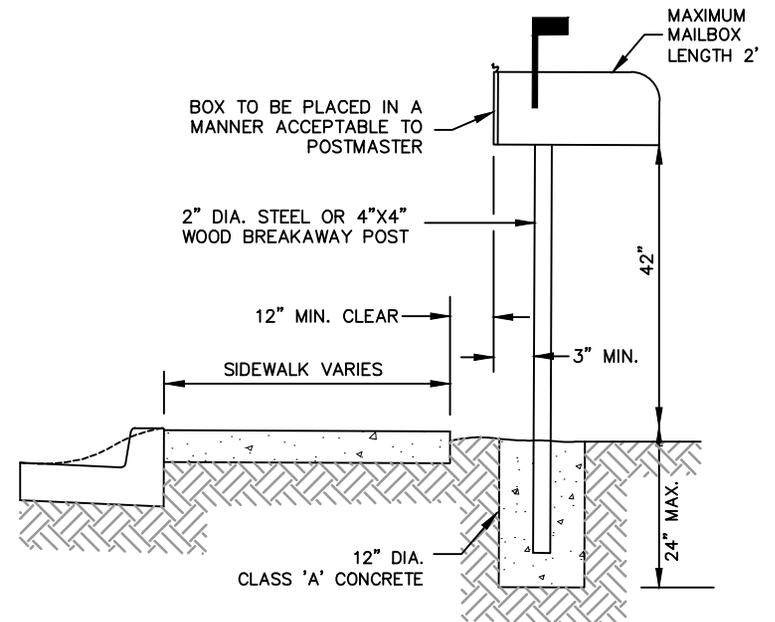
DITCH SECTION



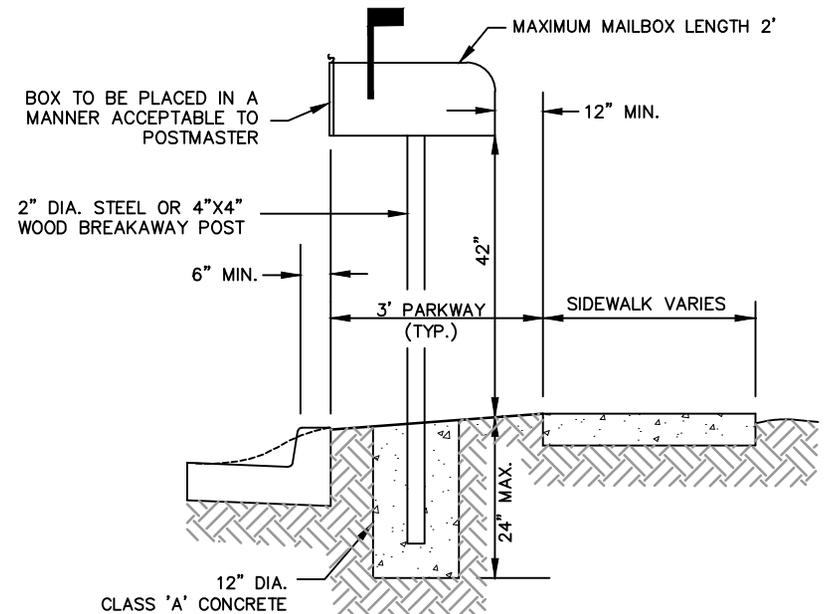
SHOULDER SECTION

NOTES:

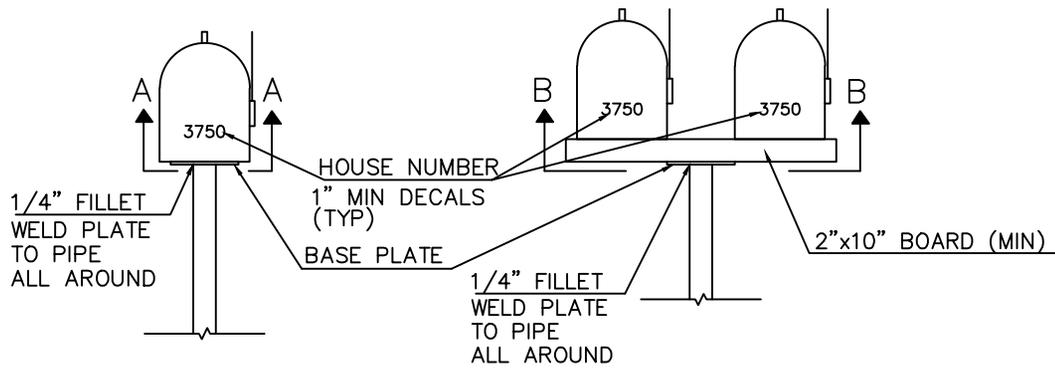
1. SOLID OR ANCHORED STRUCTURES IN RIGHT-OF-WAY SHALL BE BREAKAWAY CONSTRUCTION TO LIMIT DAMAGE & INJURIES.
2. ALTERNATE POST MOUNTED MAILBOX DESIGNS MEETING UNITED STATES POSTAL SERVICE SPECIFICATIONS AND REQUIREMENTS WILL BE CONSIDERED.
3. ANY DEVIATION FROM THIS DETAIL OR STANDARD MAILBOX MUST BE APPROVED BY THE AGENCY ENGINEER.



MAILBOX BEHIND SIDEWALK

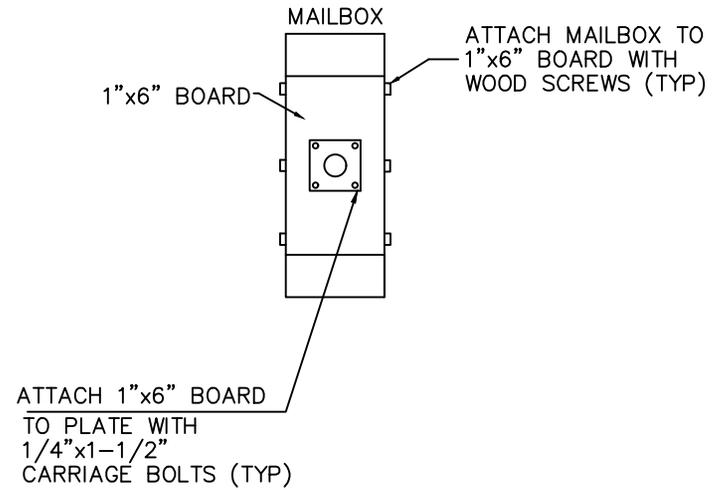


MAILBOX WITH PARKWAY

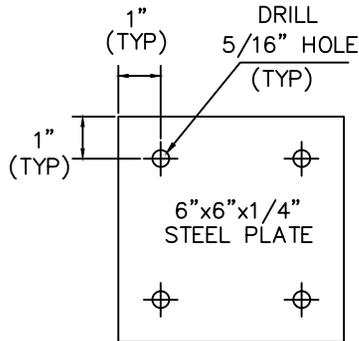


SINGLE MAILBOX INSTALLATION

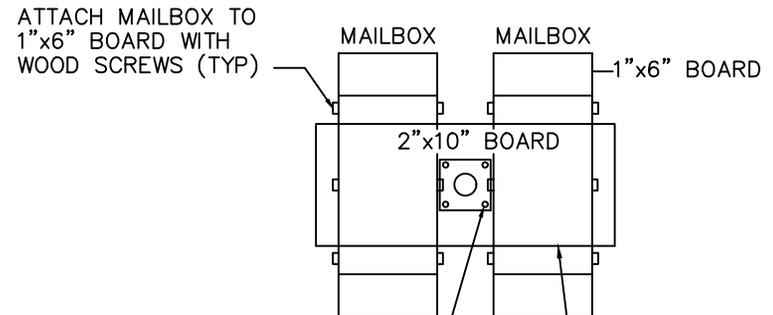
DOUBLE MAILBOX INSTALLATION



SECTION A-A



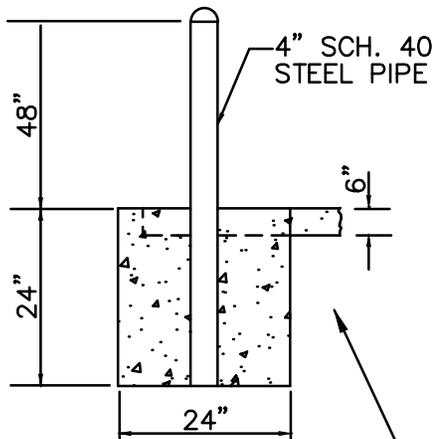
BASE PLATE



SECTION B-B

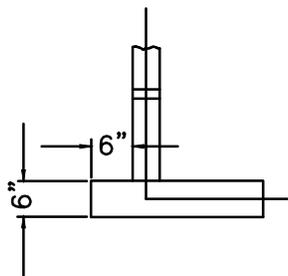
NOTE:

1. ALTERNATE POST MOUNTED MAILBOX DESIGNS MEETING UNITED STATES POSTAL SERVICE SPECIFICATIONS AND REQUIREMENTS WILL BE CONSIDERED.

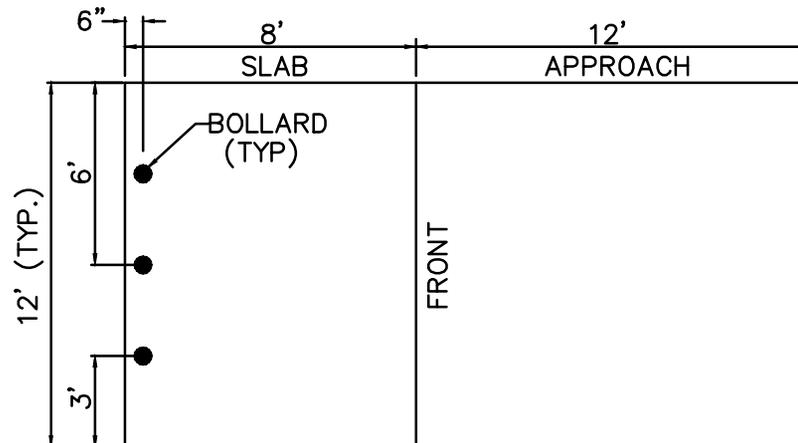


4' HIGH ABOVE PAD

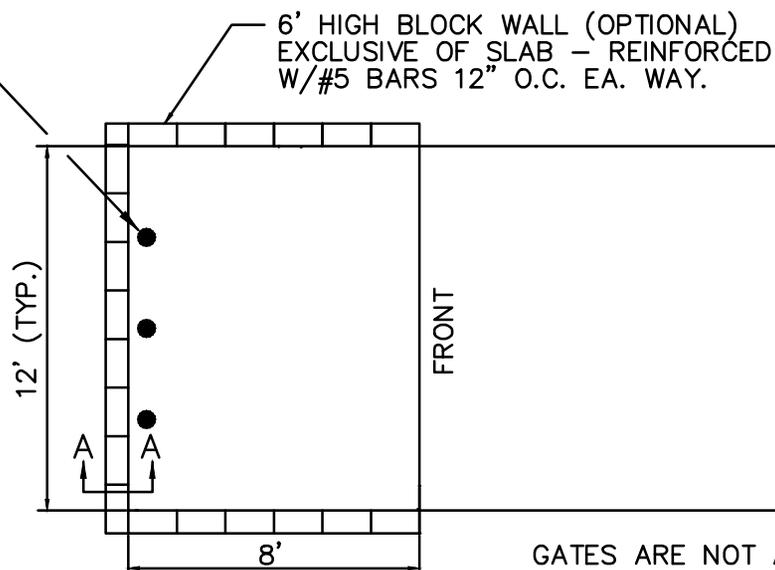
4' HIGH CONC. FILLED BOLLARD - BURY 2' W/24" COLLAR



SECTION A-A



DETAIL "A"

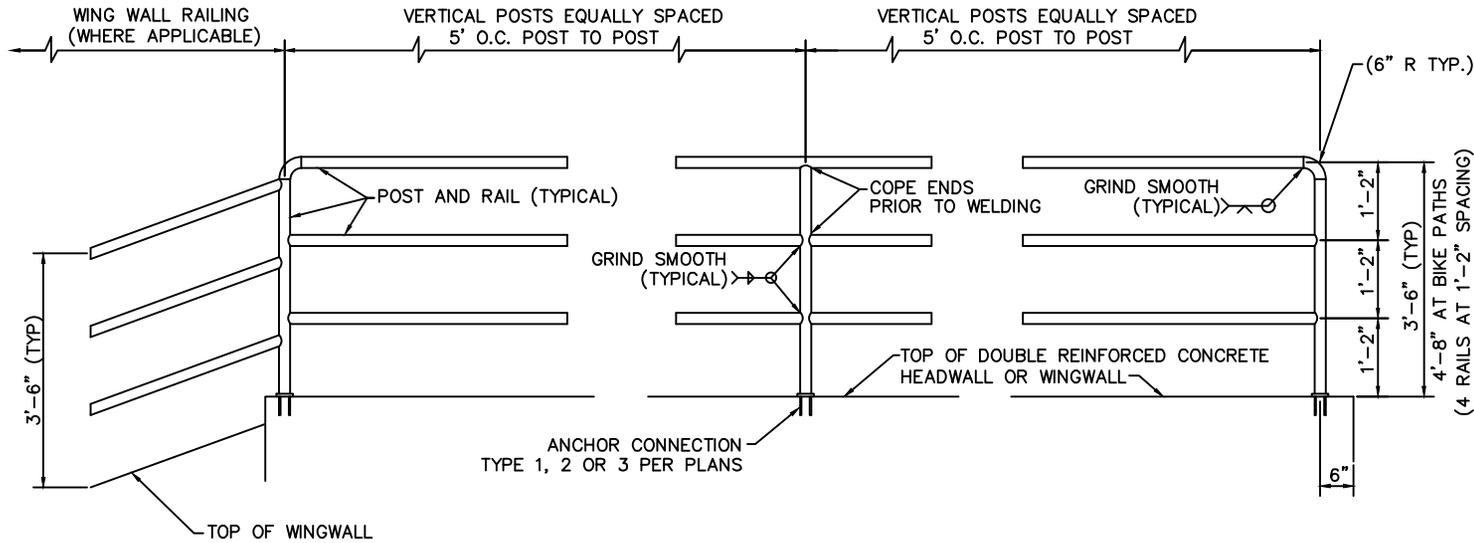


DETAIL "B"

GATES ARE NOT ALLOWED

SLAB DATA

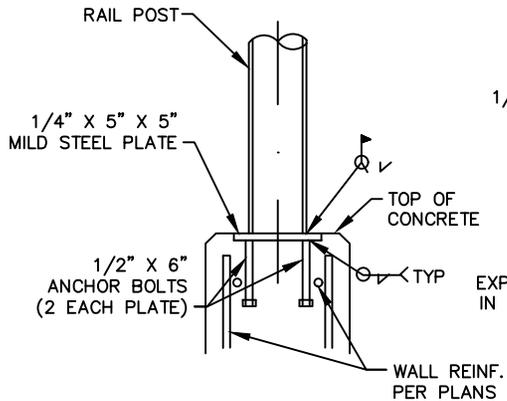
1. MIN. 6" THICK CLASS "A" CONCRETE ON 4" A.B.C. 95% COMP. AS PER MAG SECTIONS 340 & 725
2. SLAB TO BE REINFORCED W/ 6x6 - 8/8 W.W.F.
3. SLAB TO BE POURED TO THE SAME ELEV. AS THE APPROACH.



ELEVATION

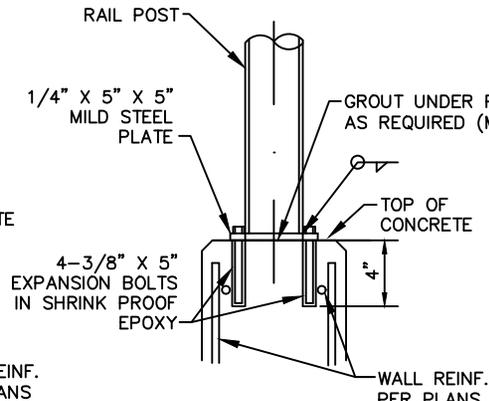
NOTES:

1. POSTS AND RAILS SHALL BE 2.0" SCHEDULE 40 STEEL PIPE ASTM A 53, GRADE B
2. PAINT RAIL PER MAG SPECIFICATIONS MAG SECTION 530 WHEN REQUIRED BY PLANS. SHOP PRIME WITH RUST INHIBITING PRIMER (FIELD REPAIR PRIMER AS NEEDED). COLOR TO BE PER AGENCY SPECIFICATIONS.
3. VERTICAL POSTS TO BE EVENLY SPACED PER DETAIL.
4. REMOVE ALL SHARP EDGES.
5. THE EMBEDMENT FOR ANCHOR TYPES 1, 2 AND 3 SHALL BE LOCATED INSIDE THE WALL REINFORCEMENT CAGE.
6. HANDRAIL IS REQUIRED WHEREVER THERE IS A SLOPE STEEPER THAN 2:1 WITHIN 3' OF A SIDEWALK.



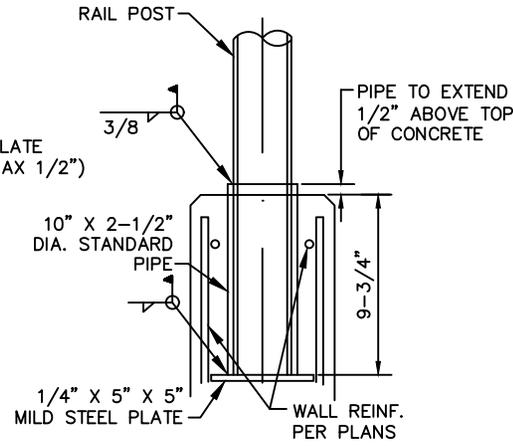
TYPE 1

ANCHOR PLATE DETAIL



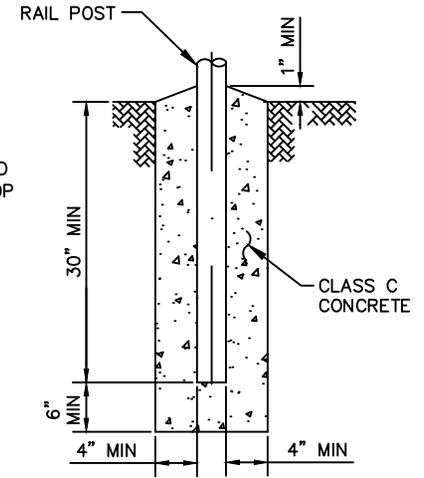
TYPE 2

EXPANSION BOLT DETAIL



TYPE 3

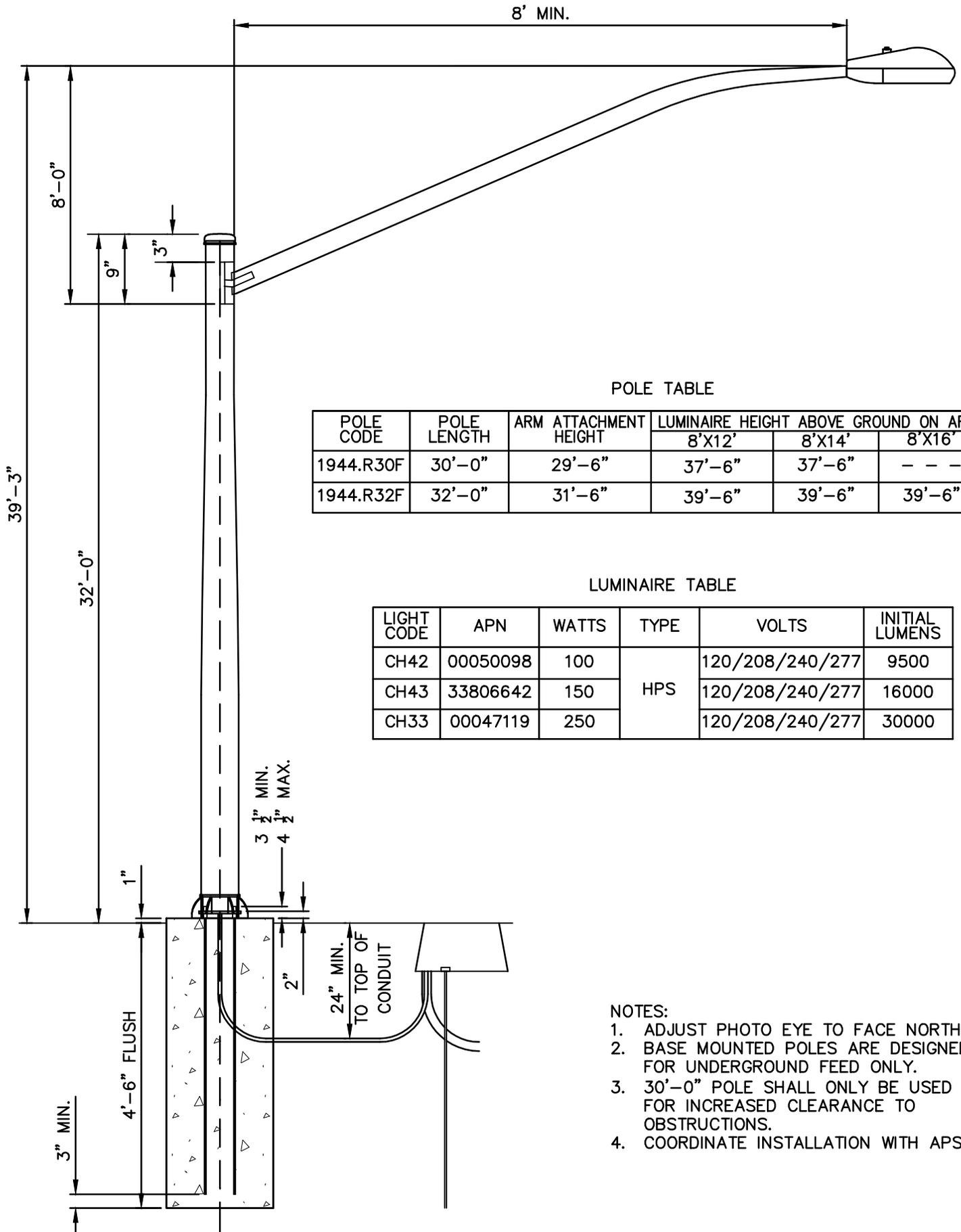
PIPE SLEEVE DETAIL



TYPE 4

GROUND INSTALLATION DETAIL

NOTE: SEE PLANS FOR ANCHORAGE DETAILS FOR ATTACHMENT TO SINGULARLY REINFORCED AND NON-REINFORCED WALLS.



POLE TABLE

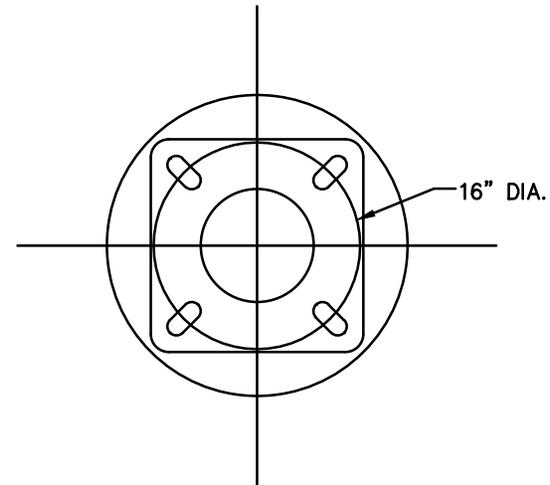
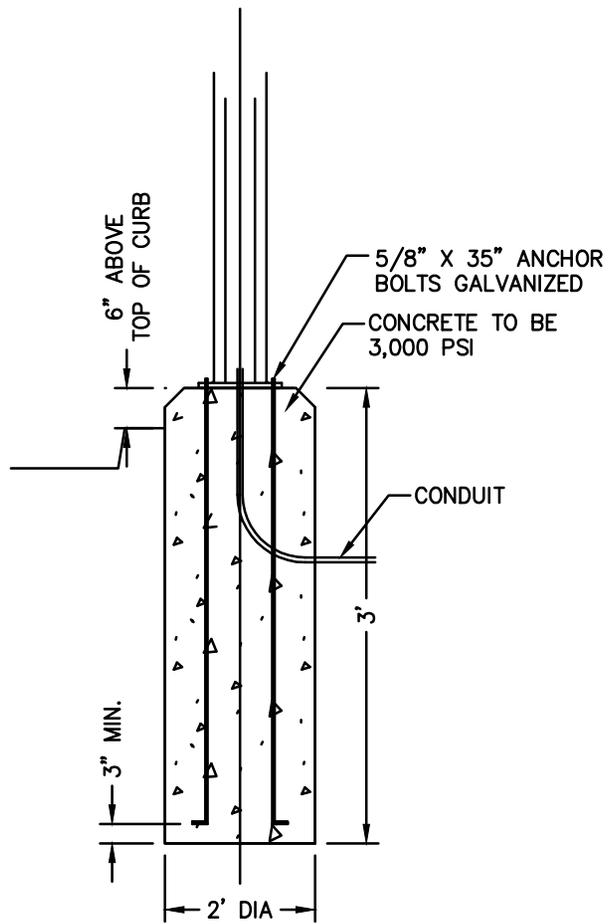
| POLE CODE | POLE LENGTH | ARM ATTACHMENT HEIGHT | LUMINAIRE HEIGHT ABOVE GROUND ON ARM | | |
|-----------|-------------|-----------------------|--------------------------------------|--------|--------|
| | | | 8'X12' | 8'X14' | 8'X16' |
| 1944.R30F | 30'-0" | 29'-6" | 37'-6" | 37'-6" | - - - |
| 1944.R32F | 32'-0" | 31'-6" | 39'-6" | 39'-6" | 39'-6" |

LUMINAIRE TABLE

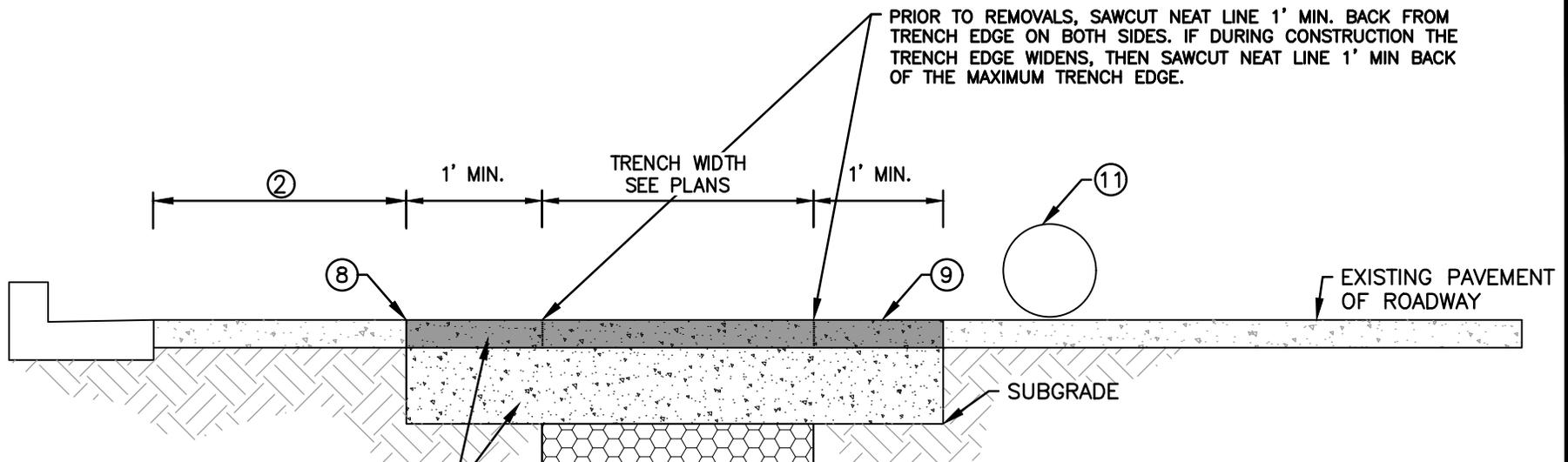
| LIGHT CODE | APN | WATTS | TYPE | VOLTS | INITIAL LUMENS |
|------------|----------|-------|------|-----------------|----------------|
| CH42 | 00050098 | 100 | HPS | 120/208/240/277 | 9500 |
| CH43 | 33806642 | 150 | | 120/208/240/277 | 16000 |
| CH33 | 00047119 | 250 | | 120/208/240/277 | 30000 |

NOTES:

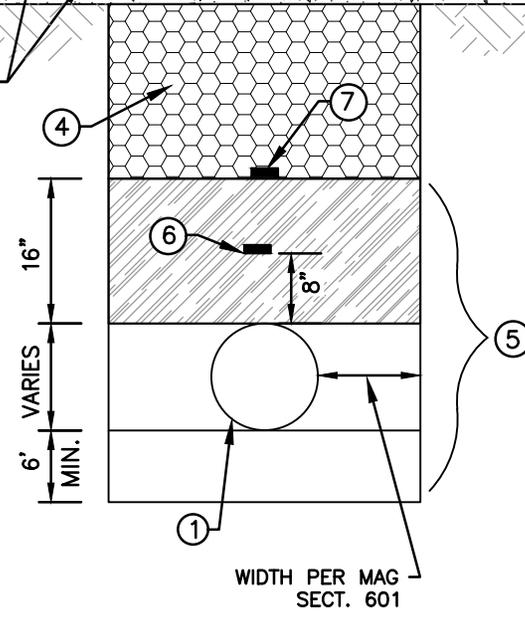
1. ADJUST PHOTO EYE TO FACE NORTH.
2. BASE MOUNTED POLES ARE DESIGNED FOR UNDERGROUND FEED ONLY.
3. 30'-0" POLE SHALL ONLY BE USED FOR INCREASED CLEARANCE TO OBSTRUCTIONS.
4. COORDINATE INSTALLATION WITH APS.



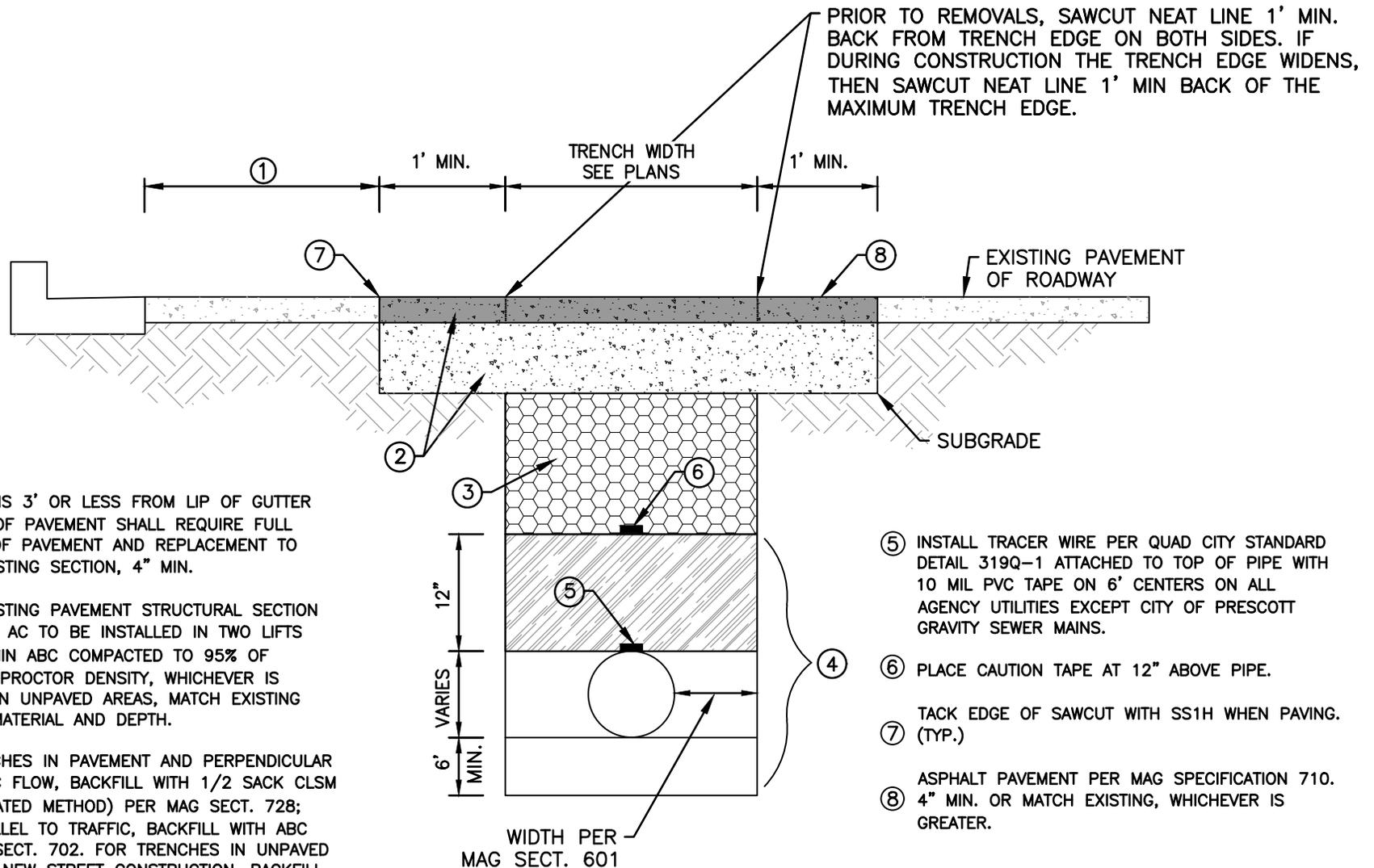
NOT TO SCALE



1. ALL WATER MAIN INSTALLATION OF PVC SHALL BE DR14.
2. EXCAVATIONS 3' OR LESS FROM LIP OF GUTTER OR EDGE OF PAVEMENT SHALL REQUIRE FULL REMOVAL OF PAVEMENT AND REPLACEMENT TO MATCH EXISTING SECTION, 4" MIN.
3. MATCH EXISTING PAVEMENT STRUCTURAL SECTION OR 4" MIN AC TO BE INSTALLED IN TWO LIFTS OVER 8" MIN ABC COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, WHICHEVER IS GREATER. IN UNPAVED AREAS, MATCH EXISTING SURFACE MATERIAL AND DEPTH.
4. FOR TRENCHES IN PAVEMENT AND PERPENDICULAR TO TRAFFIC FLOW, BACKFILL WITH 1/2 SACK CLSM (CONSOLIDATED METHOD) PER MAG SECT. 728; AND PARALLEL TO TRAFFIC, BACKFILL WITH ABC PER MAG SECT. 702. FOR TRENCHES IN UNPAVED AREAS OR NEW STREET CONSTRUCTION, BACKFILL TRENCH TO SURFACE WITH 3" MINUS SCREENED NATIVE MATERIAL.
5. GRANULAR BEDDING/SHADING MATERIAL SHALL BE PER MAG SECT. 601 WITH 100% PASSING 1" SIEVE, MAXIMUM 10 P.I.. THE COMBINATION OF PERCENT MATERIAL PASSING THROUGH #200 PLUS P.I. SHALL NOT EXCEED 22.
6. INSTALL 8 GAUGE TRACER WIRE, 8" DISTANCE ABOVE PVC PIPE.
7. PLACE CAUTION TAPE AT 16" ABOVE PIPE.
8. TACK EDGE OF SAWCUT WITH SS1H WHEN PAVING. (TYP.)
9. ASPHALT PAVEMENT PER MAG SPECIFICATION 710. 4" MIN. OR MATCH EXISTING, WHICHEVER IS GREATER.



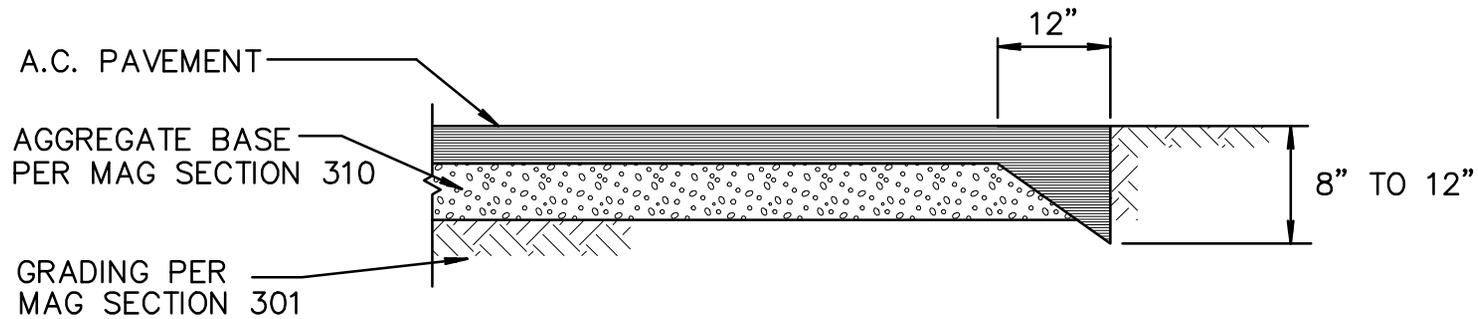
10. BEDDING, SHADING, AND BACKFILL SHALL BE COMPACTED TO 95% PROCTOR DENSITY PER ASTM D-698. MIN. 1 TEST PER LIFT PER 100'. EXPOSED SUBGRADE TO BE COMPACTED TO 95% PROCTOR DENSITY PER ASTM D-698. PAVED SURFACE TO BE COMPACTED TO 95% MARSHALL DENSITY PER ASTM D-6927-15.
11. ALL PVC PIPE WILL BE INSPECTED BY THE CITY JUST BEFORE INSTALLATION INTO THE TRENCH TO ENSURE THE PIPE IS NOT DAMAGED.
12. ALL VERTICAL REALIGNMENT OF PVC PIPE SHALL BE PER QUAD CITY STANDARD DETAIL 370Q AND LENGTH OF RESTRAINT SHALL BE PER QUAD CITY STANDARD DETAIL 303Q.
13. ALL HORIZONTAL DEFLECTION OF PVC PIPE MUST BE DIP FITTINGS (UNLESS JOINT DEFLECTION ALLOWED BY CITY ENGINEER) AND LENGTH OF RESTRAINT SHALL BE PER QUAD CITY STANDARD DETAIL 303Q.
14. INSTALLATION OF ALL PVC SHALL ADHERE TO ASTM D2774 AND F1668 (LATEST EDITIONS TO ENSURE THE SOIL/PIPE SYSTEM WILL SUPPORT DESIGN LOADS WITHOUT EXCESSIVE STRAINS FROM DEFLECTIONS OR FROM LOCALIZED PIPE WALL DISTORTIONS).
15. FREQUENCY AND DURATION OF INSPECTIONS WILL BE DETERMINED BY THE CITY. INSPECTIONS SHALL INCLUDE AT A MINIMUM:
 - A. ALL TRENCH BEDDING PRIOR TO PIPE INSTALLATION.
 - B. ALL PIPE INSTALLATION DURING SHADING PLACEMENT.
 - C. ALL UTILITY CROSSINGS OF PVC AFTER INSTALL.



- ① EXCAVATIONS 3' OR LESS FROM LIP OF GUTTER OR EDGE OF PAVEMENT SHALL REQUIRE FULL REMOVAL OF PAVEMENT AND REPLACEMENT TO MATCH EXISTING SECTION, 4" MIN.
- ② MATCH EXISTING PAVEMENT STRUCTURAL SECTION OR 4" MIN AC TO BE INSTALLED IN TWO LIFTS OVER 8" MIN ABC COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, WHICHEVER IS GREATER. IN UNPAVED AREAS, MATCH EXISTING SURFACE MATERIAL AND DEPTH.
- ③ FOR TRENCHES IN PAVEMENT AND PERPENDICULAR TO TRAFFIC FLOW, BACKFILL WITH 1/2 SACK CLSM (CONSOLIDATED METHOD) PER MAG SECT. 728; AND PARALLEL TO TRAFFIC, BACKFILL WITH ABC PER MAG SECT. 702. FOR TRENCHES IN UNPAVED AREAS OR NEW STREET CONSTRUCTION, BACKFILL TRENCH TO SURFACE WITH 3" MINUS SCREENED NATIVE MATERIAL.
- ④ GRANULAR BEDDING/SHADING MATERIAL SHALL BE PER MAG SECT. 601 WITH 100% PASSING 1" SIEVE, MAXIMUM 10 P.I.. THE COMBINATION OF PERCENT MATERIAL PASSING THROUGH #200 PLUS P.I. SHALL NOT EXCEED 22.

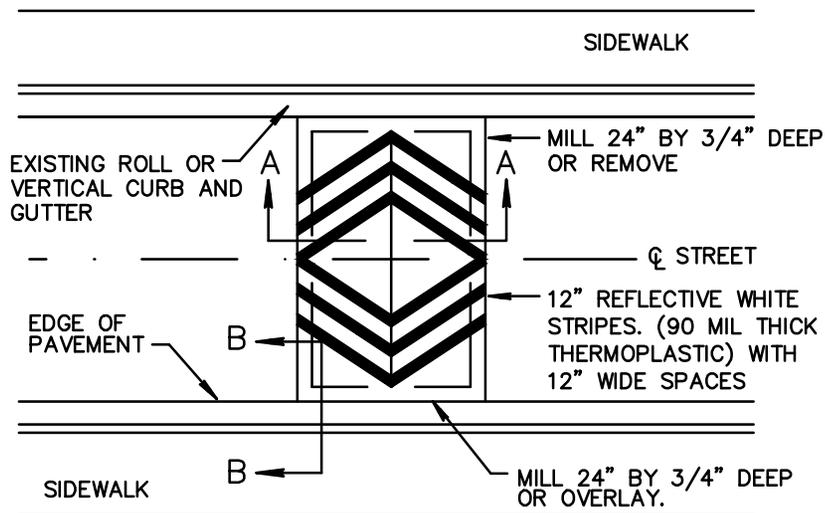
- ⑤ INSTALL TRACER WIRE PER QUAD CITY STANDARD DETAIL 319Q-1 ATTACHED TO TOP OF PIPE WITH 10 MIL PVC TAPE ON 6' CENTERS ON ALL AGENCY UTILITIES EXCEPT CITY OF PRESCOTT GRAVITY SEWER MAINS.
- ⑥ PLACE CAUTION TAPE AT 12" ABOVE PIPE.
- ⑦ TACK EDGE OF SAWCUT WITH SS1H WHEN PAVING. (TYP.)
- ⑧ ASPHALT PAVEMENT PER MAG SPECIFICATION 710. 4" MIN. OR MATCH EXISTING, WHICHEVER IS GREATER.

NOTE: BEDDING, SHADING, AND BACKFILL SHALL BE COMPACTED TO 95% PROCTOR DENSITY PER ASTM D-698. MIN. 1 TEST PER LIFT PER 200'. EXPOSED SUBGRADE TO BE COMPACTED TO 95% PROCTOR DENSITY PER ASTM D-698. PAVED SURFACE TO BE COMPACTED TO 95% MARSHALL DENSITY PER ASTM D-6927-15.

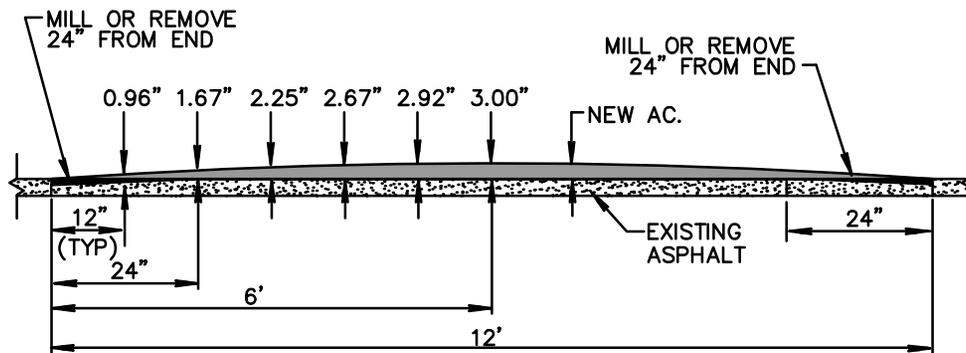


NOTES:

1. THICKENED EDGE REQUIRED AT ALL TRANSVERSE TERMINATIONS OF PAVING AND AT LONGITUDINAL EDGES FOR ASPHALT PAVEMENTS UNCONFINED BY CURB & GUTTER.
2. MATCHLINE TO EXISTING PAVEMENTS SHALL BE SKEWED OR OFFSET FOR SMOOTHER TRANSITION. LOCATION OF SAWCUT SHALL BE AS DIRECTED BY THE AGENCY ENGINEER OR HIS DESIGNEE.
3. EXISTING EDGES SHALL BE UNDISTURBED NATIVE, FIRM, & UNYIELDING SOIL OR FORMED TO PROVIDE FULLY COMPACTED STRAIGHT EDGES.

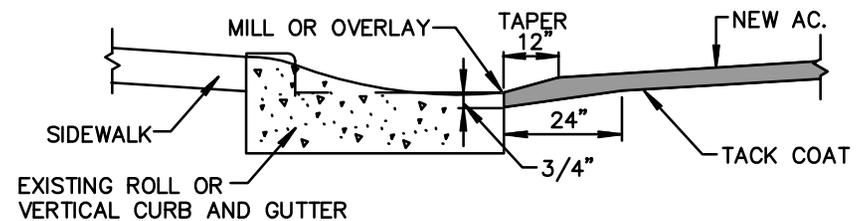


PLAN VIEW



SECTION A-A

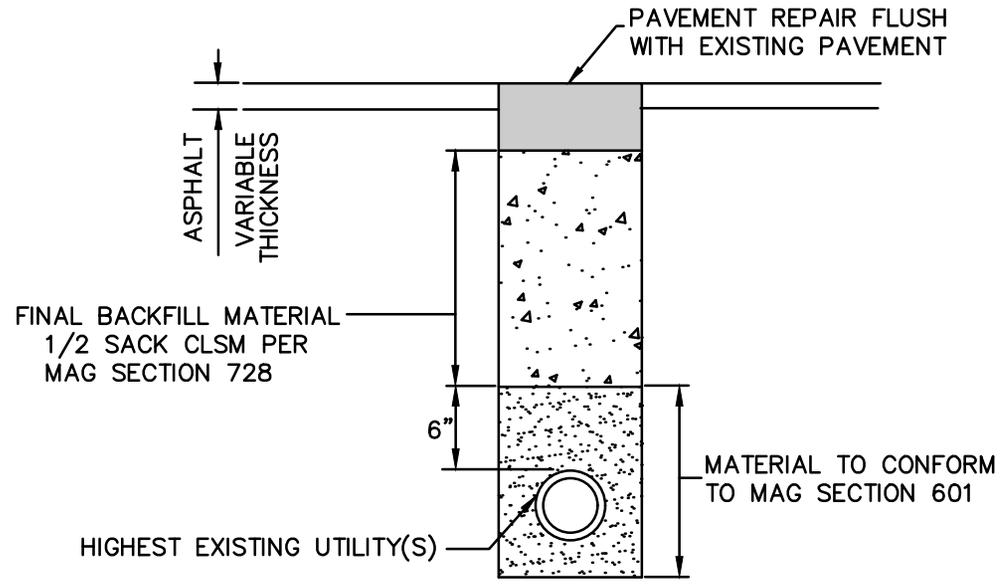
IMPORTANT:
TO GAIN MAXIMUM EFFECT, HUMPS MUST BE THE FULL 3". CONTRACTORS
MUST NOT EXCEED THIS HEIGHT BASED ON CONSIDERATION FOR EMERGENCY POLICE
AND FIRE DEPARTMENT VEHICLES.



SECTION B-B

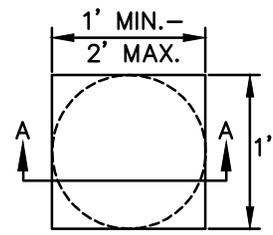
NOTES:

1. SPEED HUMPS SHALL NOT BE PLACED OVER MANHOLES, WATER VALVES, SURVEY MONUMENTS OR ANY SUBSURFACE INFRASTRUCTURE INDICATOR.
2. SPEED HUMPS SHALL NOT BE INSTALLED IN A LOCATION SUCH THAT ROADWAY DRAINAGE IS COMPROMISED.
3. SPEED HUMPS TO BE CONSTRUCTED WITH TYPE R-1.5 (D-1/2) COURSE, M.A.G. SPECIFICATIONS SECTION 710. A TACK COAT SHALL BE APPLIED PRIOR TO APPLICATION OF PAVEMENT MATERIAL.
4. CONTACT THE AGENCY ENGINEER ONE WEEK PRIOR TO INSTALLATION TO COORDINATE MARKINGS & SIGNING.



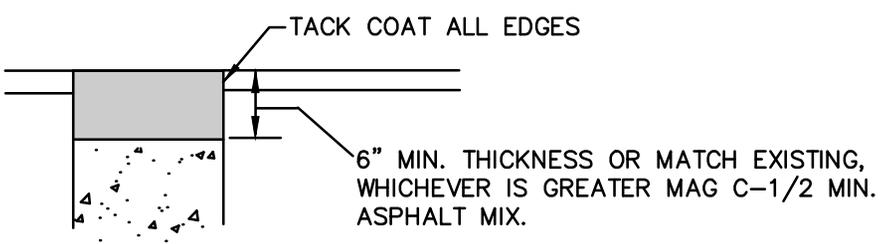
SECTION VIEW

PAVEMENT REPAIR



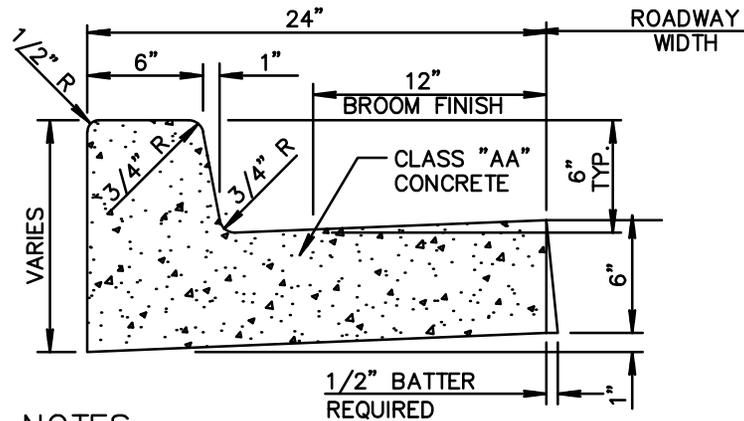
PLAN VIEW

- NOTES:
1. DIMENSIONS ARE NOMINAL.
 2. EDGES SHALL BE CUT TO A NEAT VERTICAL FACE.
 3. PLACE CLSM BACKFILL IN ACCORDANCE WITH MAG SECTION 604.
 4. PLACE ASPHALT CONCRETE IN MAXIMUM 2" LIFTS.

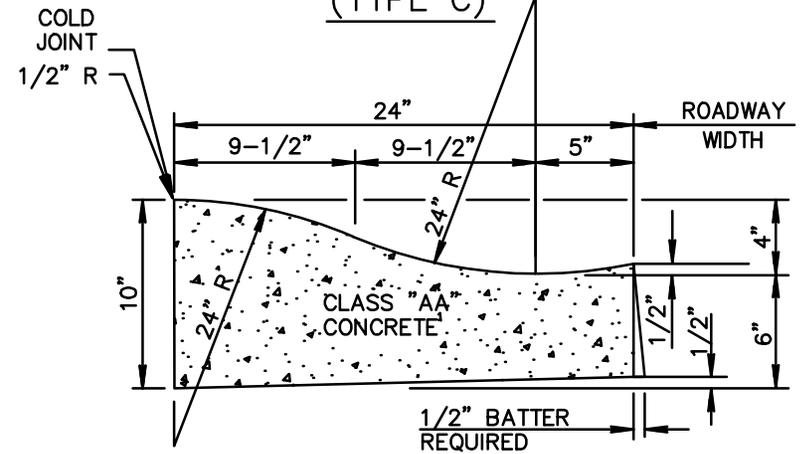


SECTION A-A

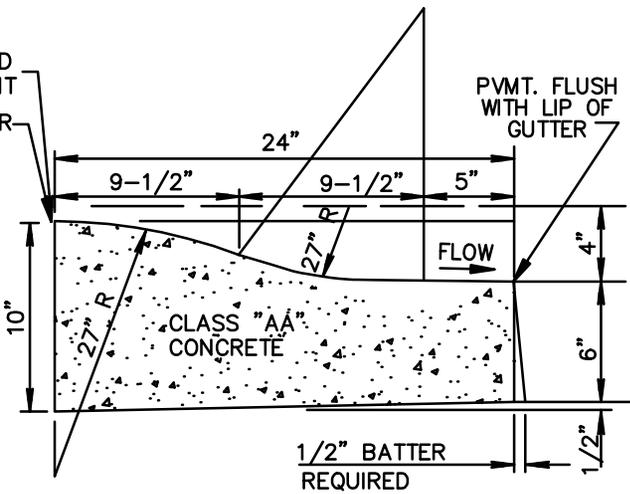
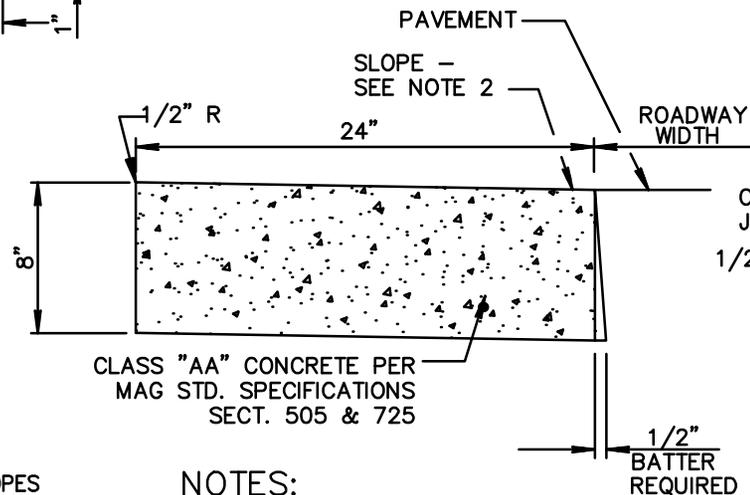
VERTICAL CURB AND GUTTER (TYPE A)



ROLL CURB AND GUTTER (TYPE C)



RIBBON CURB (TYPE B)



NOTES:

1. ALL EXPOSED SURFACES TO BE TROWEL FINISHED EXCEPT AS SHOWN. SEE MAG SECT. 340 & 505 & 725
2. HEIGHT OF TYPE 'A' CURB SHALL BE 6" UNLESS SPECIFIED ON PLANS
3. CONTRACTION JOINT SPACING 10' MAXIMUM.
4. EXPANSION JOINTS AS PER MAG SECT. 340 50' MAX. AND AT P.C., DWY., ALLEYS & STRUCTURES
5. 4" MINIMUM OF ABC COMPACTED 95% PROCTOR SHALL EXTEND TO BACK OF CURB BELOW ALL CURB & GUTTER.
6. WHEN THE ADJACENT PAVEMENT SECTION SLOPES AWAY FROM THE GUTTER, THE SLOPE OF THE GUTTER PAN SHALL MATCH PAVEMENT CROSS SLOPE.
7. 1/4" ROUND FINISH SHALL BE REQUIRED AT LIP OF GUTTER, ALL DETAILS.
8. FINAL PAVEMENT SURFACE SHALL BE 1/4" ABOVE GUTTERS EXCEPT TYPE D AND WHERE GUTTER SLOPES TOWARD PAVEMENT.
9. 3 - 1/2"x12" DOWELS SHALL BE PLACED WHEN ATTACHING NEW CURB TO EXISTING CURB.

NOTES:

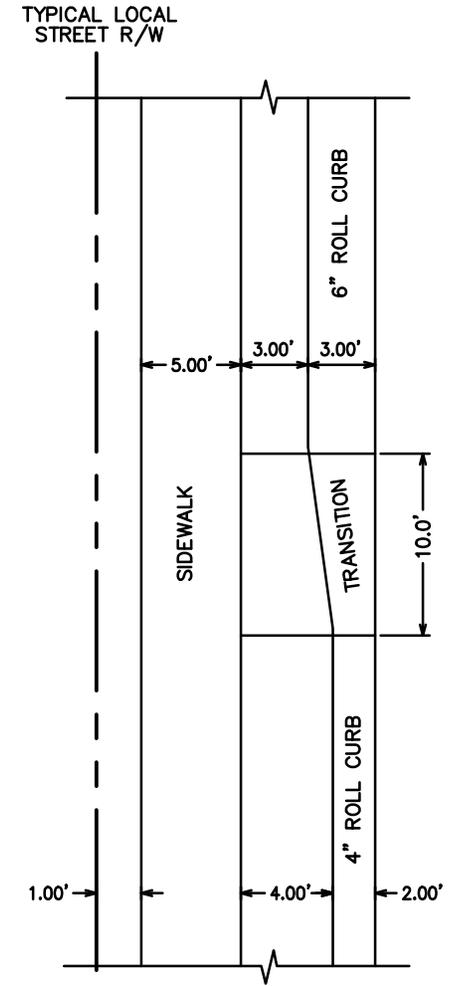
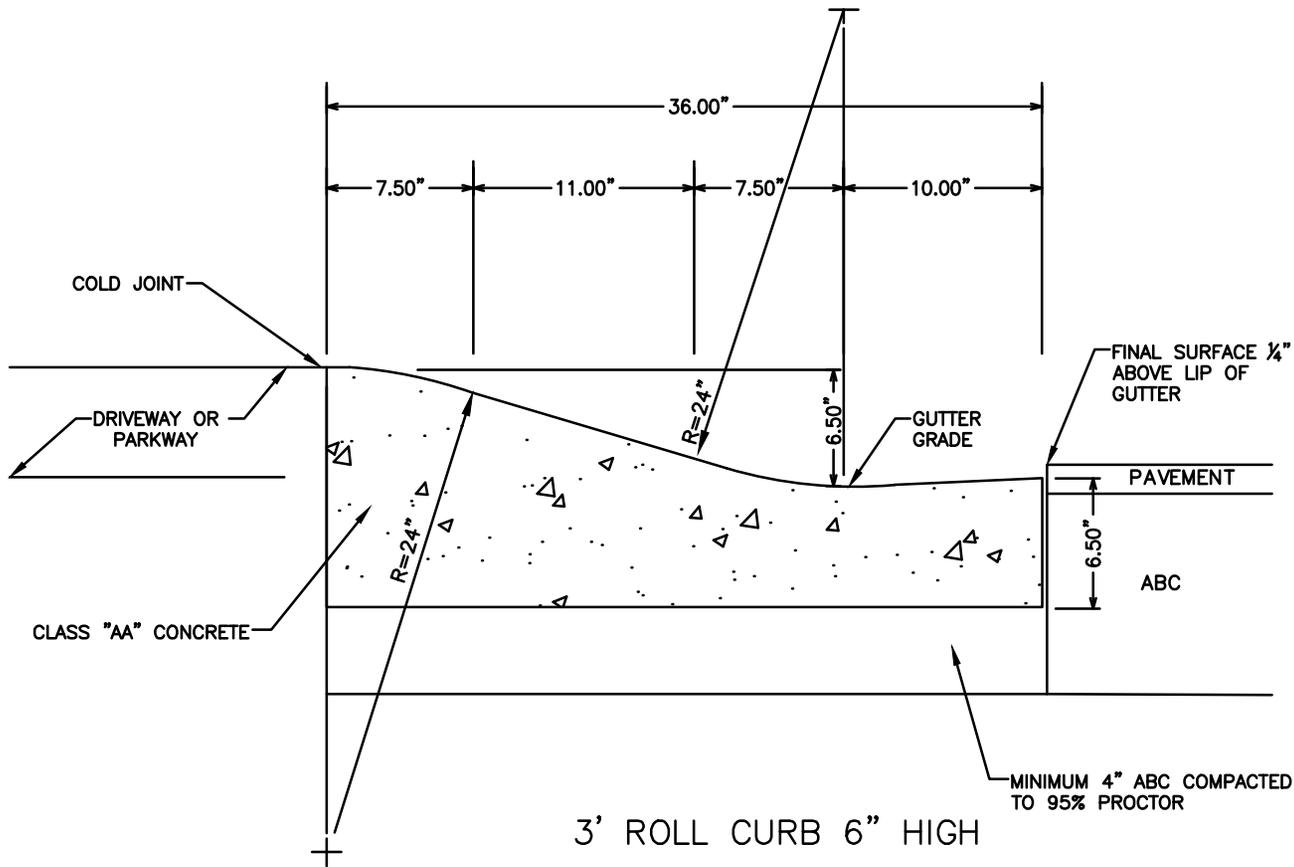
1. CONSTRUCT CURB AND INSTALL 1/2" MASTIC EXPANSION JOINTS, A.S.T.M. D-1751 AND MAG SECT. 340.
2. RIBBON CURB MAY SLOPE TOWARDS PAVEMENT OR PARKWAY AS INDICATED ON PLANS.

(TYPE D)

SPECIAL SECT. USE FOR HIGH SIDE CURB WITH SHEET DRAINAGE REVERSE FLOW ACROSS STREET

TRANSITION

3' ROLL CURB, 6" HIGH
TO
2' ROLL CURB, 4" HIGH

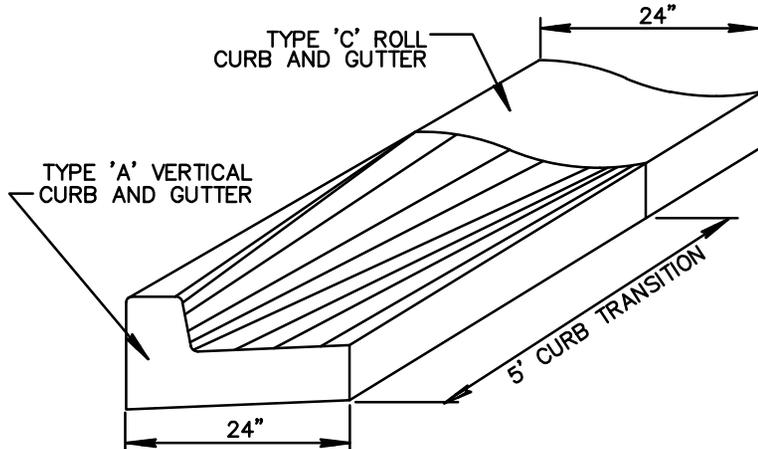


PLAN VIEW

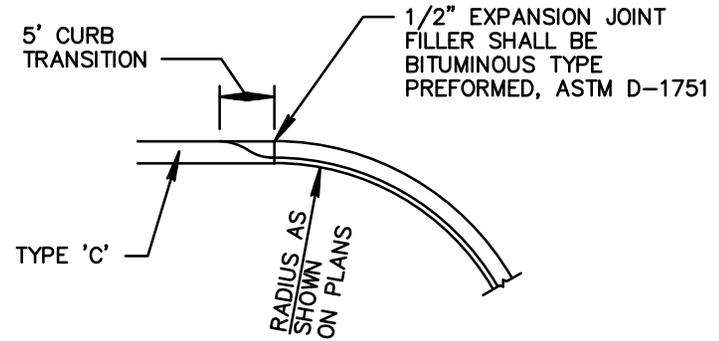
NOTES:

1. ALL WORK AND MATERIALS SHALL CONFORM TO MAG STANDARD SPECIFICATIONS, SECTION 340, 505, AND 725 BROOM FINISH EXPOSED SURFACE.
2. CONTRACTION JOINT SPACING SHALL BE A MAXIMUM OF 10 FEET.
3. EXPANSION JOINTS SHALL BE CONSTRUCTED AS PER MAG STANDARD SPECIFICATIONS SECTION 340.

CURB TRANSITION TYPE 'A' TO TYPE 'C'



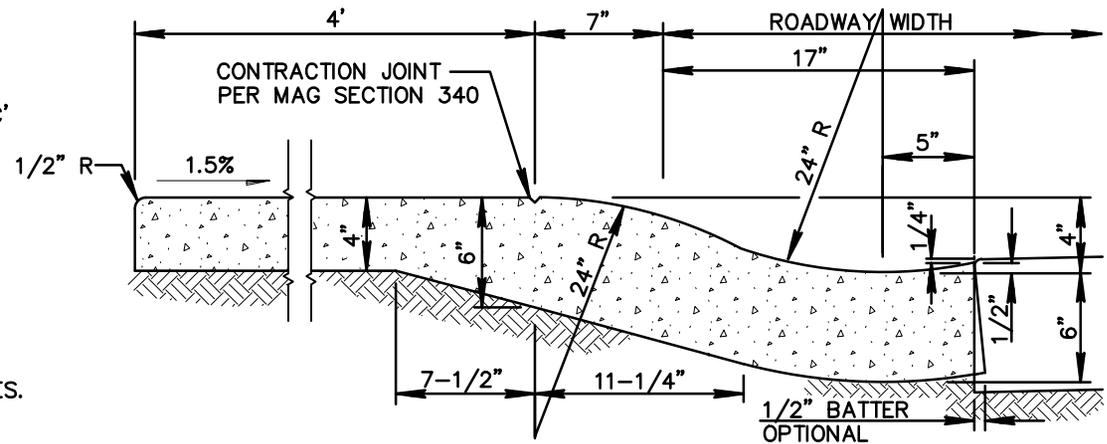
CURB AND GUTTER TRANSITION



NOTES: (CURB AND GUTTER TRANSITIONS)

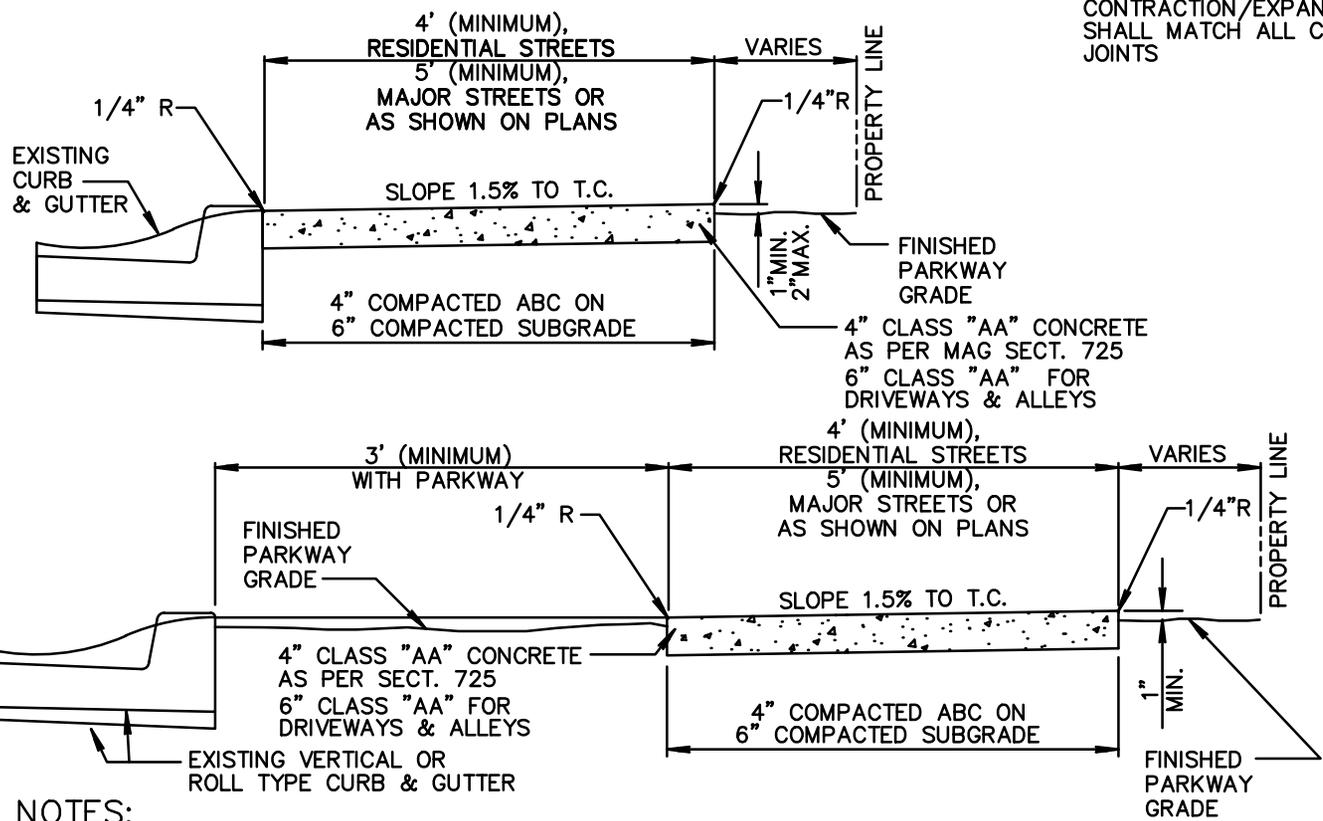
- TRANSITIONS WILL BE PAID FOR AS THE PREDOMINANT TYPE OF CURB AND GUTTER BEING TRANSITIONED. WHEN TYPE 'A' CURB AND GUTTER ARE USED AT CURB RETURNS AND TYPE 'C' CURB AND GUTTER IS PREDOMINANTLY USED ELSEWHERE, THE TYPE 'A' TO TYPE 'C' TRANSITIONS SHALL BE MEASURED AND PAID FOR AS TYPE 'C' CURB AND GUTTER.
- WHERE PROPOSED CONSTRUCTION IS TO BE CONNECTED TO EXISTING CURB AND GUTTER, THE TRANSITION SHALL BE INDICATED ON PLANS.
- CLASS 'AA' CONCRETE PER MAG SECTION 725.
- TRANSITION BETWEEN TYPICAL SECTIONS SHALL BE ACCOMPLISHED BY THE USE OF DIRECT STRAIGHT LINE TRANSITIONS OF THE FLOW LINE AND OTHER SURFACE FEATURES.
- 4" MINIMUM OF ABC COMPACTED TO 95% PROCTOR BELOW ALL CURB & GUTTER.

INTEGRAL ROLL CURB, GUTTER AND SIDEWALK

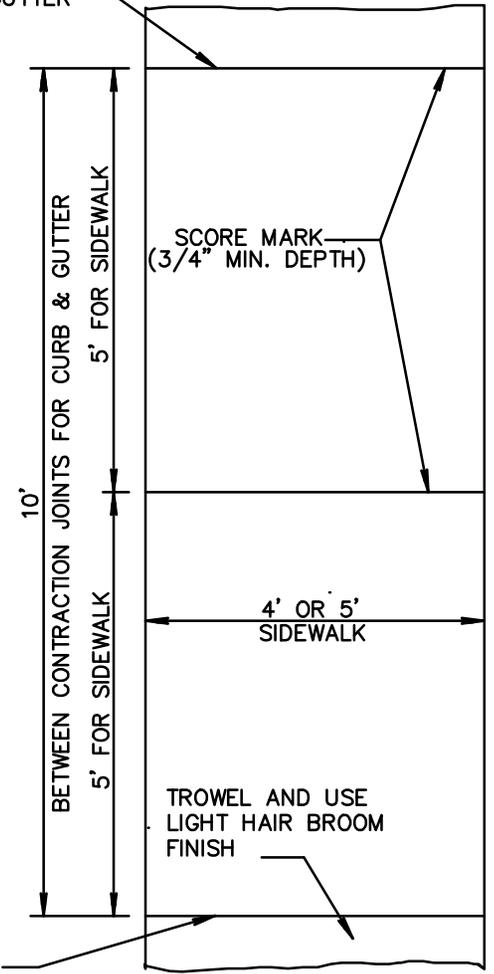


NOTES: (INTEGRAL ROLL CURB, GUTTER AND SIDEWALK)

- CONCRETE TO BE MONOLITHIC POUR. EXPOSED SURFACE FINISH AS PER SIDEWALK AND GUTTER DETAIL.
- CONTRACTION JOINT SPACING 5' MAXIMUM.
- EXPANSION JOINTS PER MAG SECTION 340.
- CLASS 'AA' CONCRETE PER MAG SECTION 725.
- 4" MINIMUM OF ABC COMPACTED TO 95% PROCTOR BELOW ALL CURB & GUTTER.



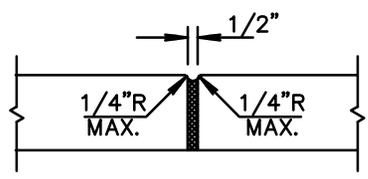
CONTRACTION/EXPANSION JOINTS SHALL MATCH ALL CURB & GUTTER JOINTS



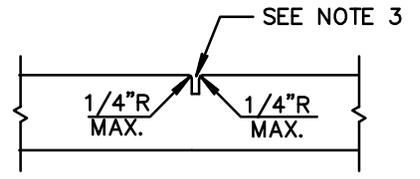
CONTRACTION/EXPANSION JOINTS SHALL MATCH ALL CURB & GUTTER JOINTS

NOTES:

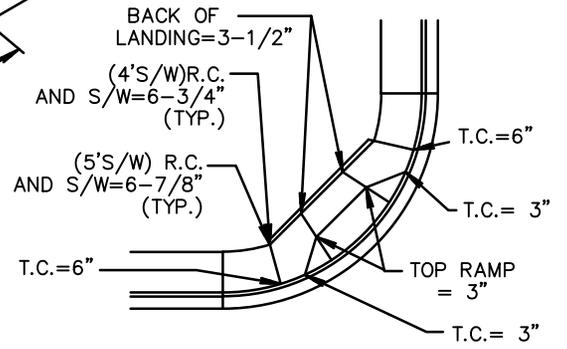
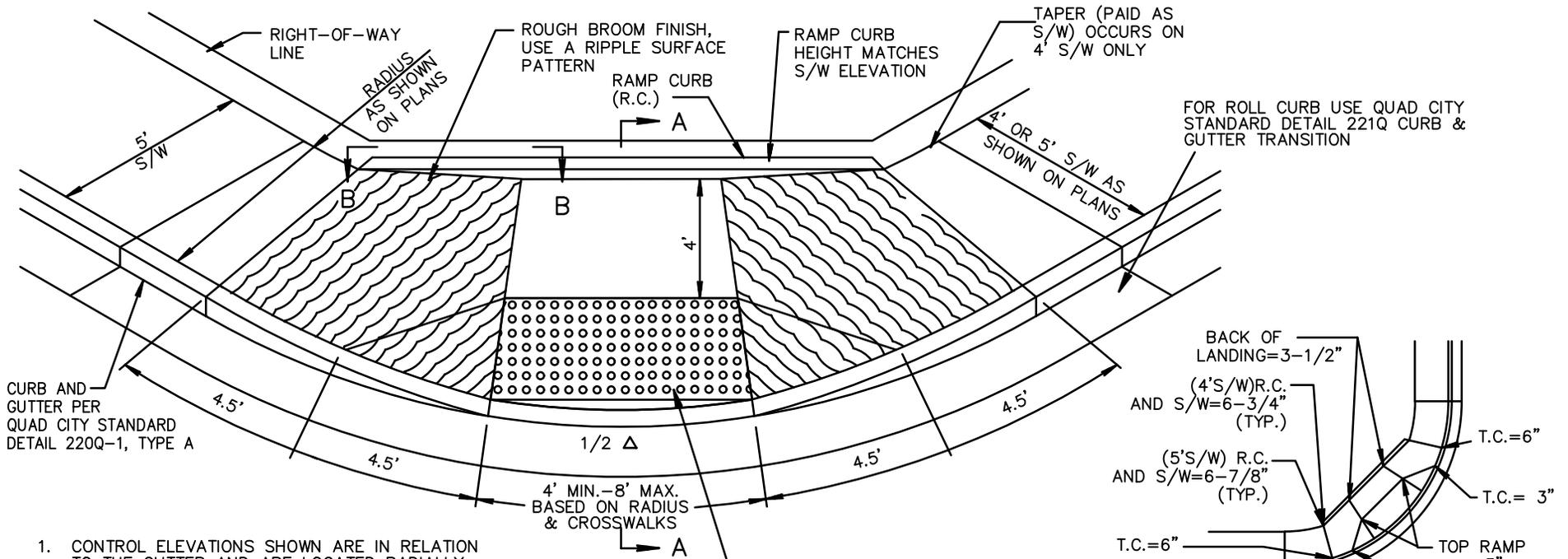
1. SIDEWALK CONSTRUCTION SHALL CONFORM TO MAG SECTIONS 725 & 340.
2. EXPANSION JOINT FILLER SHALL BE 1/2" BITUMINOUS TYPE PREFORMED EXPANSION JOINT FILLER, A.S.T.M. D-1751.
3. LARGE AGGREGATE, IN CONTRACTION JOINT, SHALL BE SEPARATED TO A DEPTH OF 1". FINISH DEPTH SHALL BE A MINIMUM OF 3/4".
4. EXPANSION JOINT 50' MAX. SPACING PER MAG SECT. 340 AND AT ALL P.C.'S, DRIVEWAYS, ALLEYS, AND STRUCTURES.
5. SUBGRADE TO BE COMPACTED TO 95% OF MAXIMUM PER MAG SECT. 301. MOISTURE TO BE 2% OVER OPTIMUM IN COHESIVE SOILS.
6. 4" MINIMUM OF ABC COMPACTED TO 95% PROCTOR SHALL EXTEND TO BACK OF SIDEWALK.



EXPANSION JOINT



CONTRACTION JOINT

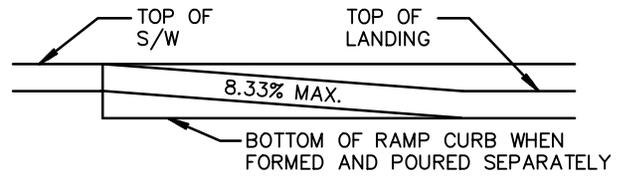


1. CONTROL ELEVATIONS SHOWN ARE IN RELATION TO THE GUTTER AND ARE LOCATED RADIALLY. GUTTER ELEVATION=0.
2. CLASS "AA" CONCRETE CONSTRUCTION AS PER MAG SECTIONS 725 & 340.
3. NON-DIRECTIONAL RAMP MAY BE USED ON LOCAL STREET/LOCAL STREET INTERSECTIONS ONLY.
4. SIDEWALK AND RAMP SHALL BE 6" CLASS "AA" CONCRETE TO 10' ON BOTH SIDES OF RAMP.
5. LENGTH OF WINGS VARY WITH RUNNING SLOPE OF ROADS. SEE WING LENGTH TABLE. MAXIMUM SLOPE OF RAMPS SHALL NOT EXCEED A.D.A. REQUIRED 8.33%.
6. 4" MINIMUM OF ABC COMPACTED TO 95% PROCTOR SHALL EXTEND TO BACK OF RAMP.

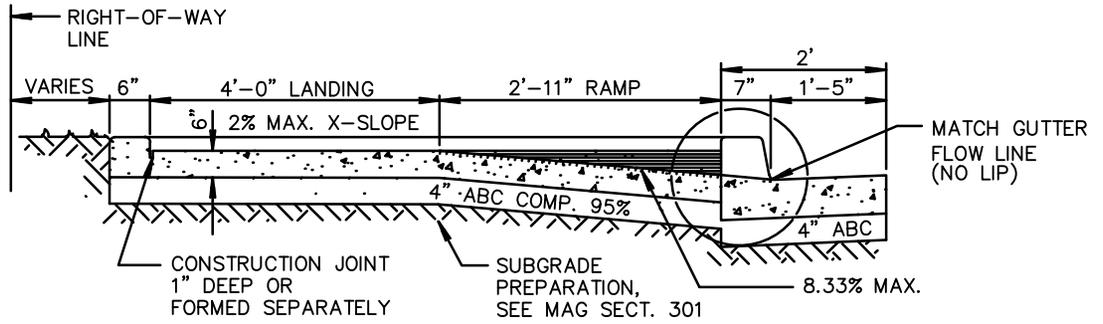
DETECTABLE WARNING PANEL OF TRUNCATED DOMES: MASCO CASTINTACT OR APPROVED EQUAL; COLOR SALEM RED MINIMUM 2' X 4' PANEL.

WING LENGTH TABLE

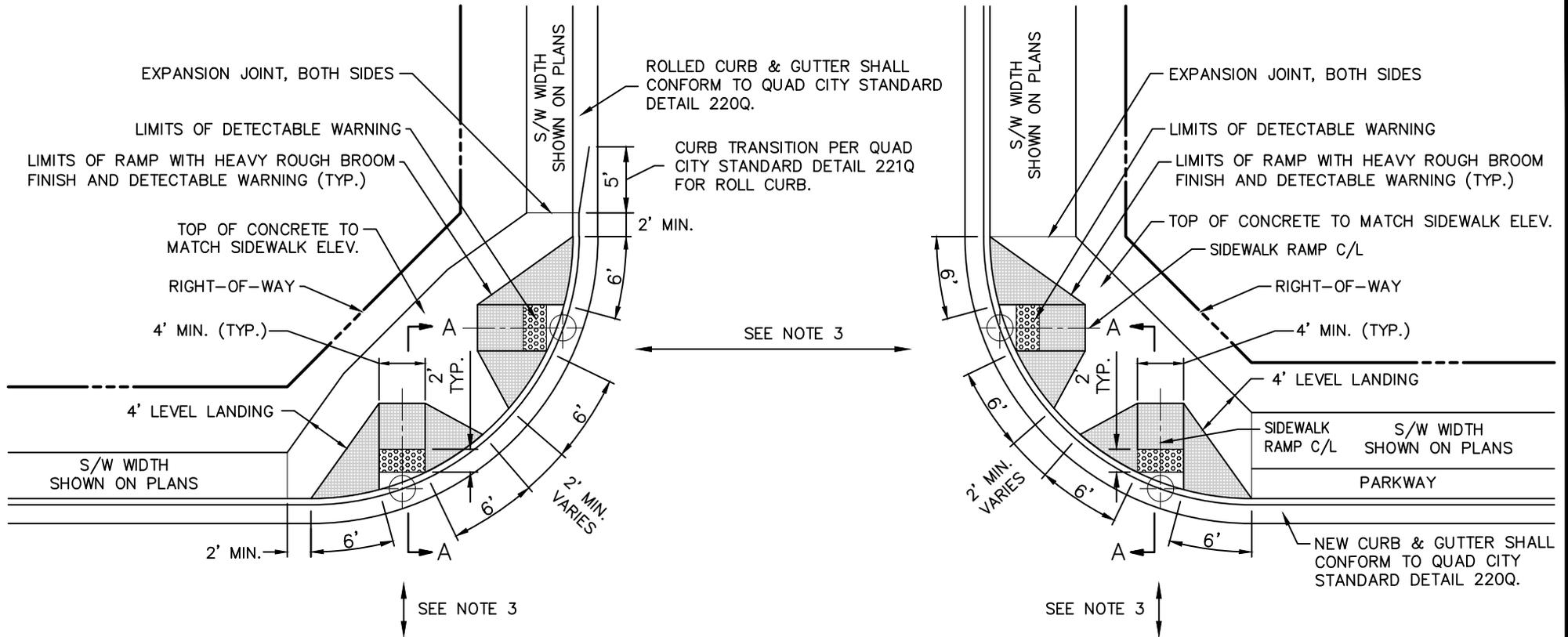
| CURB FACE | RADIUS (FT) | SIDE SLOPE | X | T.C. GRADE (ALONG CURB RETURN) | | | | | |
|-----------|-------------|------------|----------------|--------------------------------|------|------|-------|-------|-------|
| | | | | 1% | 2% | 3% | 4% | 5% | 6% |
| 6" | 25' | 8.33% | X _S | 5.4' | 4.9' | 4.5' | 4.1' | 3.8' | 3.5' |
| | | | X _L | 6.9' | 7.9' | 9.4' | 11.6' | 15.0' | 15.0' |



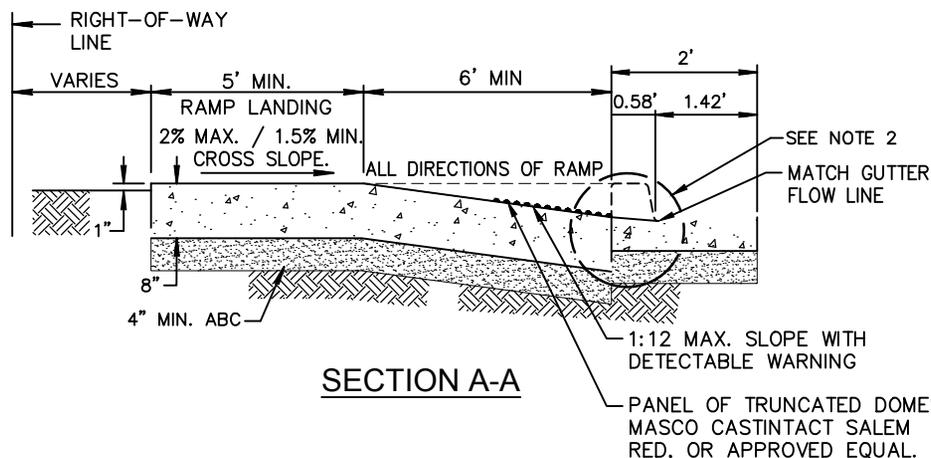
SECTION B-B



SECTION A-A

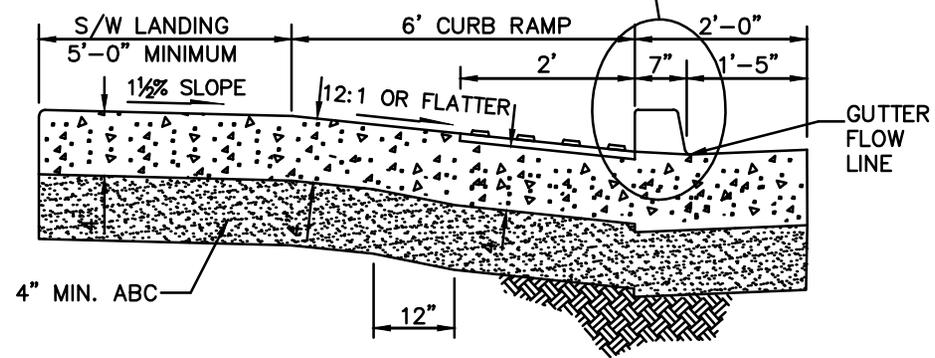
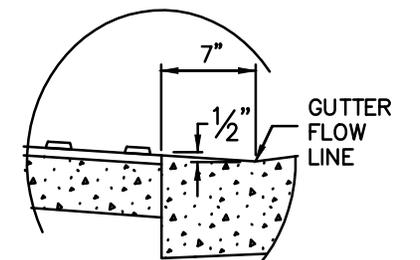
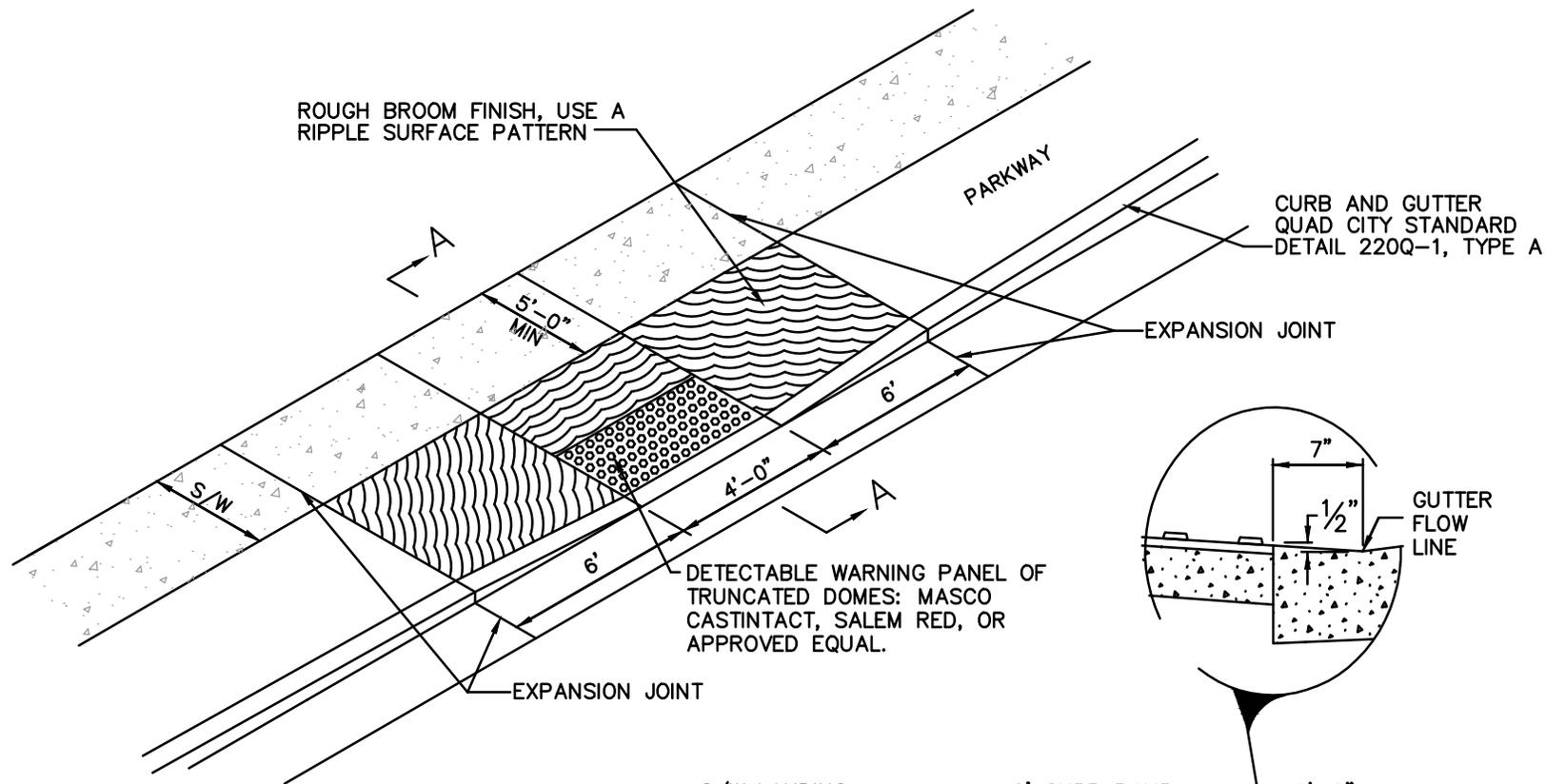


LEGEND
 RAMP CONTROL POINT (TYP.) SEE PLANS



NOTES:

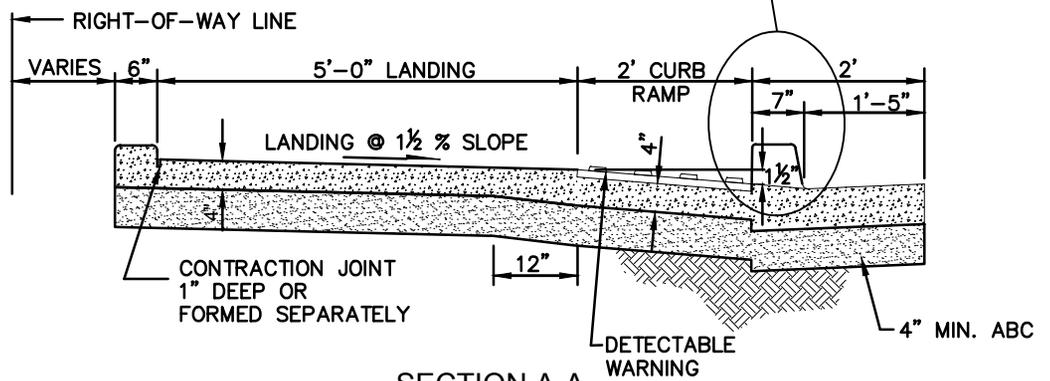
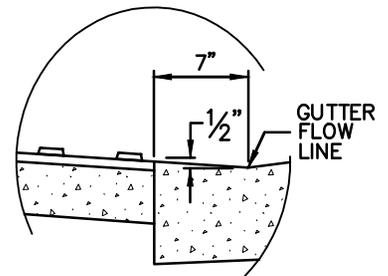
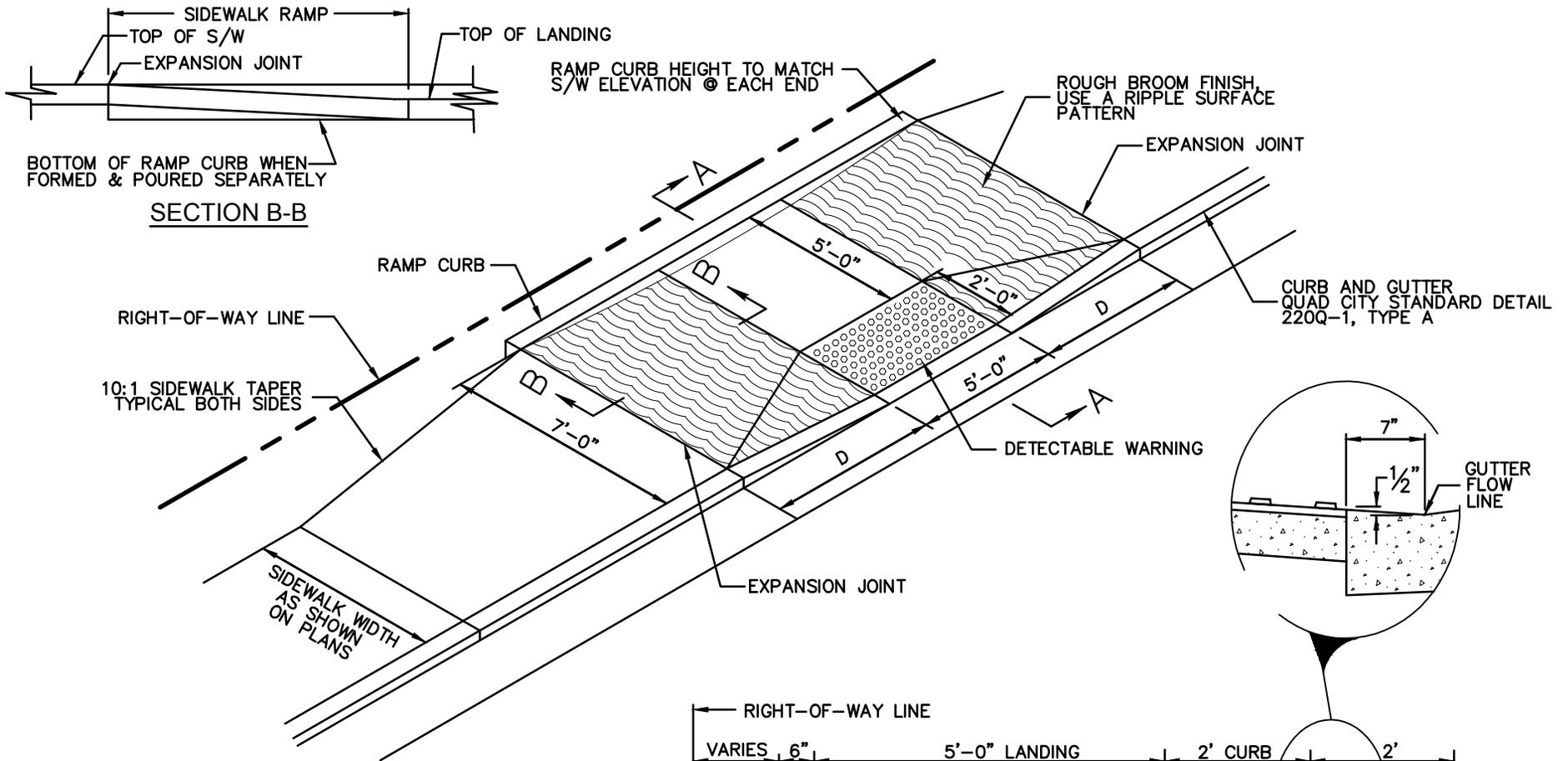
1. ALL CONCRETE TO BE CLASS "AA", MAG SECTION 725.
2. ALL RAMPS AND DETECTABLE WARNING SHALL BE ALIGNED IN THE DIRECTION OF PEDESTRIAN TRAVEL AND DIRECTED TOWARD RAMP ON OPPOSITE SIDE OF STREET.
3. THE USE OF ANY SIDEWALK RAMP OTHER THAN DIRECTIONAL SIDEWALK RAMPS ON COLLECTOR OR ARTERIAL STREETS REQUIRES APPROVAL FROM AGENCY ENGINEER.
4. SEE PLANS FOR LOCATION OF SIDEWALK RAMP CENTERLINE.
5. NO PORTION OF THE PEDESTRIAN ROUTE CROSSING THE STREET MAY EXCEED 2% CROSS SLOPE.
6. A MINIMUM OF 4" ABC COMPACTED TO 95% SHALL EXTEND TO BACK OF RAMP LANDING.



SECTION A-A

NOTES:

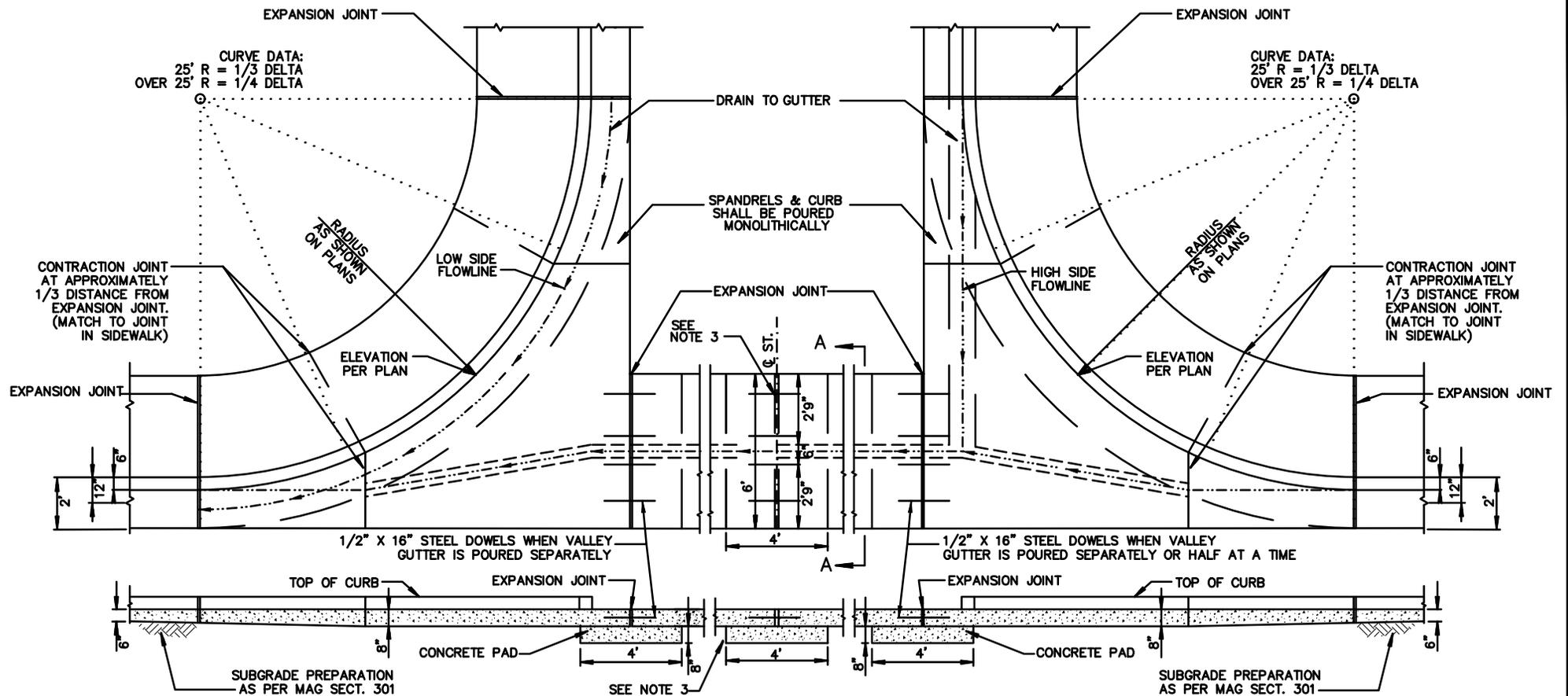
1. CLASS "AA" CONCRETE PER MAG SECTION 725.
2. EXPANSION JOINTS SHALL CONFORM TO MAG SECTION 340.
3. SIDEWALK SURFACE TO MATCH 1 1/2 % SLOPE FROM TOP OF CURB.
4. DETAIL IS ADA COMPLIANT FOR $S_G \leq 2\%$.
5. A MINIMUM OF 4" ABC COMPACTED TO 95% PROCTOR SHALL EXTEND TO BACK OF RAMP LANDING.



NOTES:

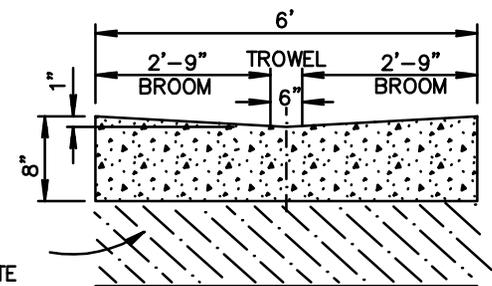
1. CLASS "AA" CONCRETE PER MAG SECTION 725.
2. EXPANSION JOINTS SHALL CONFORM TO MAG SECTION 340.
3. DETECTABLE WARNING PANEL OF TRUNCATED DOMES: MASCO CASTINTACT SALEM RED, OR APPROVED EQUAL.
4. DETAIL IS ADA COMPLIANT FOR $S_G \leq 2\%$.
5. A MINIMUM OF 4" ABC COMPACTED TO 95% PROCTOR SHALL EXTEND TO BACK OF RAMP LANDING.

SECTION A-A



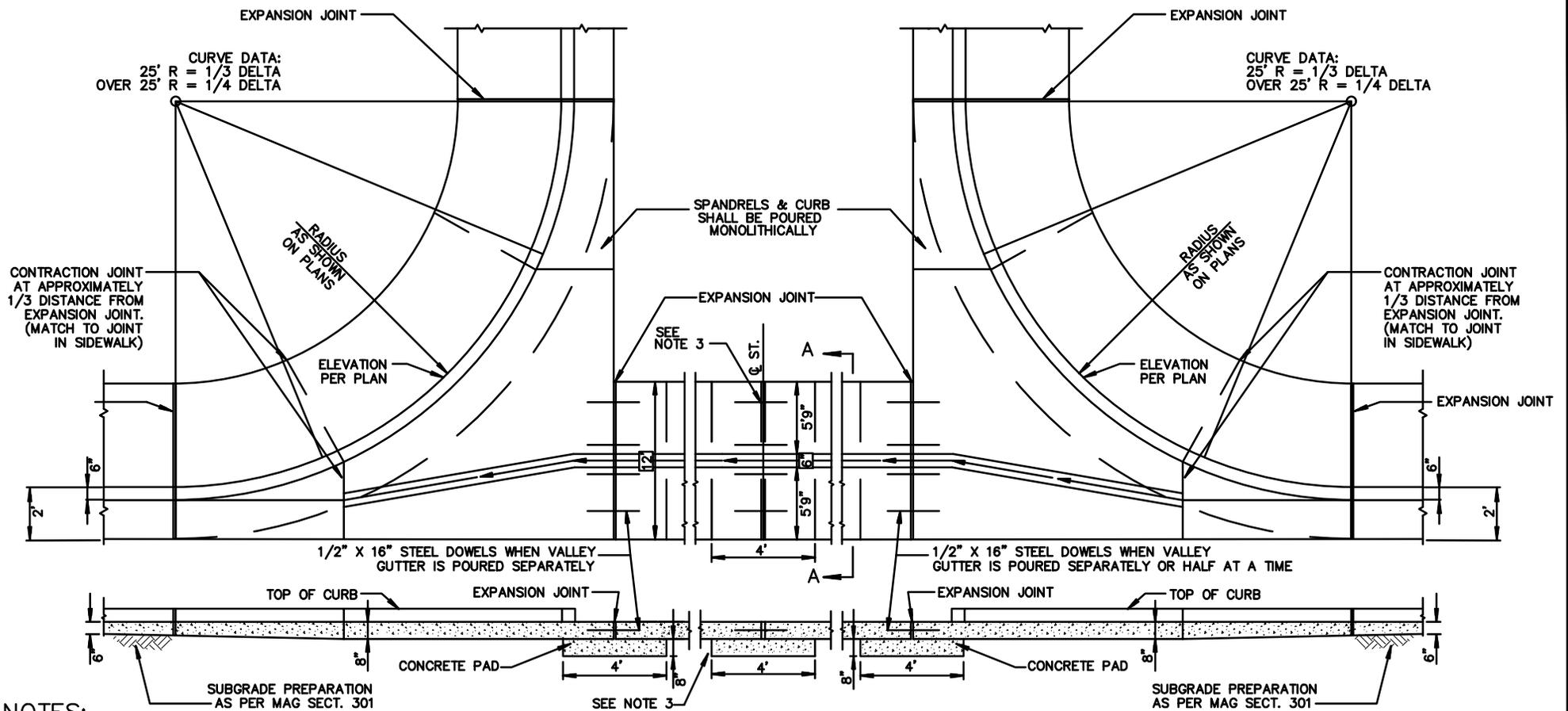
NOTES:

1. CONCRETE SHALL BE CLASS "AA" WITH 5%-7% ENTRAINED AIR PORTLAND CEMENT CONCRETE 600 LBS. WITH 3/4" AGGREGATE MAX., 4000 P.S.I. AT 28 DAYS & SHALL HAVE A SLUMP OF NOT MORE THAN 4 INCHES, PER MAG SECT. 340 & 725.
2. EITHER CONSTRUCTION JOINT OR CONTRACTION JOINT IS REQUIRED AT CENTERLINE OF STREET.
3. A SEPARATE CONCRETE PAD IS REQUIRED WHEN VALLEY GUTTER IS POURED SEPARATELY OR HALF AT A TIME.
4. EXPANSION JOINTS SHALL CONFORM TO MAG SECT. 340.
5. NO CONCRETE SHALL BE PLACED PRIOR TO FORM INSPECTION BY THE AGENCY ENGINEER OR DESIGNEE.
6. A MINIMUM OF 8" ABC (OR PER GEOTECH ANALYSIS) COMPACTED TO 95% PROCTOR SHALL EXTEND BELOW GUTTER & SPANDRELS.



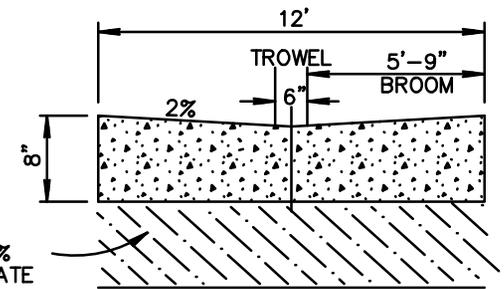
ABC COMP. 95%
(NOT A SEPARATE
PAY ITEM)

**SECTION A-A
VALLEY GUTTER**



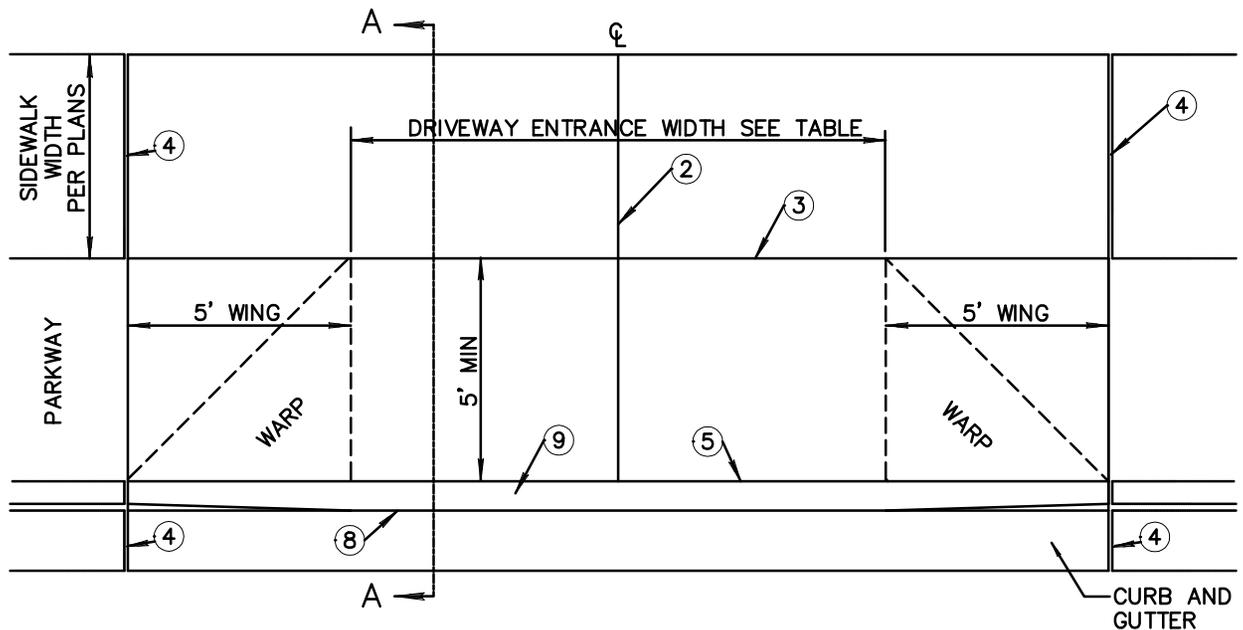
NOTES:

1. CONCRETE SHALL BE CLASS "AA" WITH 5% ± 1% ENTRAINED AIR PORTLAND CEMENT CONCRETE 600 LBS. WITH 3/4" AGGREGATE MAX., 4000 P.S.I. AT 28 DAYS & SHALL HAVE A SLUMP OF NOT MORE THAN 4 INCHES, PER MAG SECT. 340 & 725.
2. EITHER CONSTRUCTION JOINT OR CONTRACTION JOINT IS REQUIRED AT CENTERLINE OF STREET.
3. A SEPARATE CONCRETE PAD IS REQUIRED WHEN VALLEY GUTTER IS POURED SEPARATELY OR HALF AT A TIME.
4. EXPANSION JOINTS SHALL CONFORM TO MAG SECT. 340.
5. NO CONCRETE SHALL BE PLACED PRIOR TO FORM INSPECTION BY THE AGENCY ENGINEER OR DESIGNEE.
6. A MINIMUM OF 8" ABC (OR PER GEOTECH ANALYSIS) COMPACTED TO 95% PROCTOR SHALL EXTEND BELOW GUTTER AND SPANDRELS.

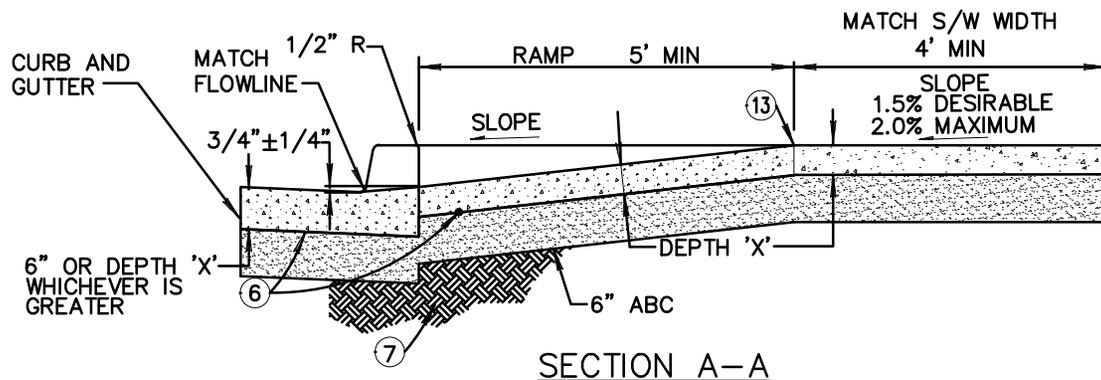
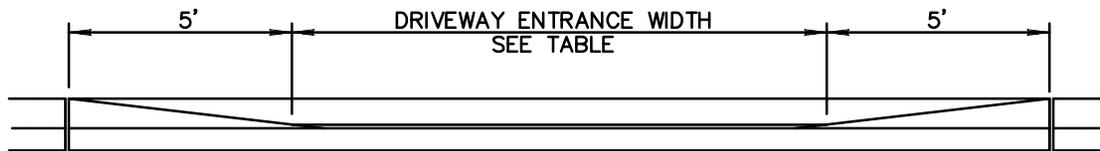


ABC COMP. 95%
(NOT A SEPARATE
PAY ITEM)

**SECTION A-A
VALLEY GUTTER**



DRIVEWAY WITH DETACHED SIDEWALK

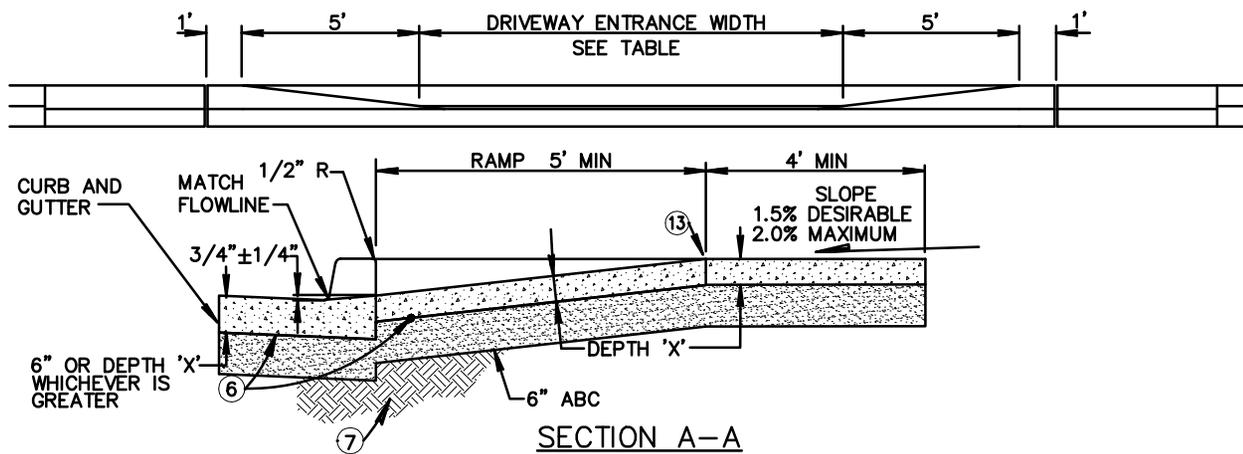
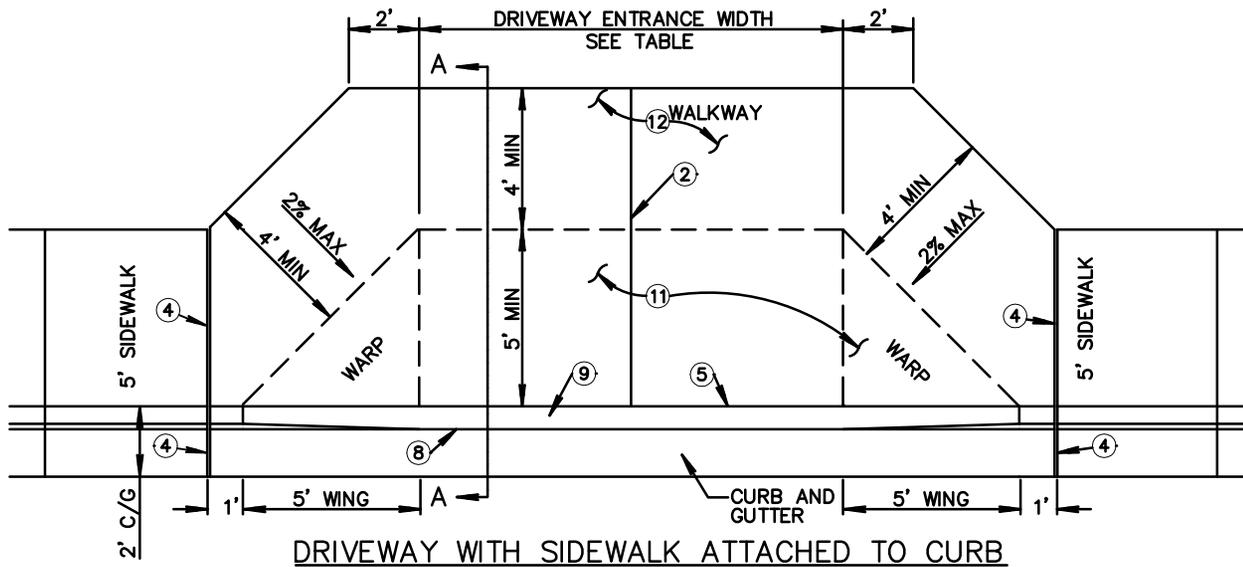


SECTION A-A

NOTES:

1. DEPRESSED CURB SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE TYPE OF CURB USED AT THAT LOCATION.
2. CONTRACTION JOINT ON D/W CENTERLINE.
3. CONTRACTION JOINT.
4. 1/2-INCH EXPANSION JOINTS SHALL COMPLY WITH MAG SECTION 340.
5. BACK OF CURB - CONSTRUCTION JOINT.
6. CLASS "AA" CONCRETE PER MAG SECTION 725.
7. SUBGRADE PREPARATION, MAG SECT. 301.
8. FLOW LINE OF GUTTER.
9. DEPRESSED CURB.
10. SECT. A-A AND ELEVATION: D/W SHOWN WITH VERTICAL CURB AND GUTTER, ROLL TYPE CURB AND GUTTER TREATED SIMILARLY.
11. ROUGH BROOM FINISH FULL WIDTH OF RAMP AND WINGS.
12. TROWEL AND USE LIGHT HAIR BROOM FINISH FOR WALKWAY AREA.
13. ELEVATION AT TOP OF DRIVEWAY RAMP SHALL BE EQUAL TO OR HIGHER THAN NORMAL CURB ELEVATION.

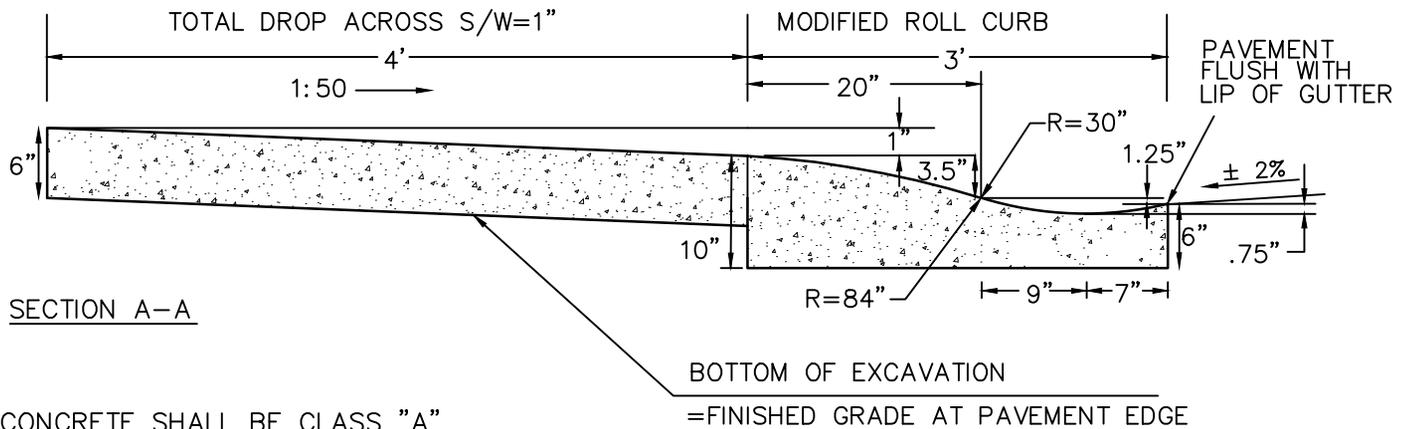
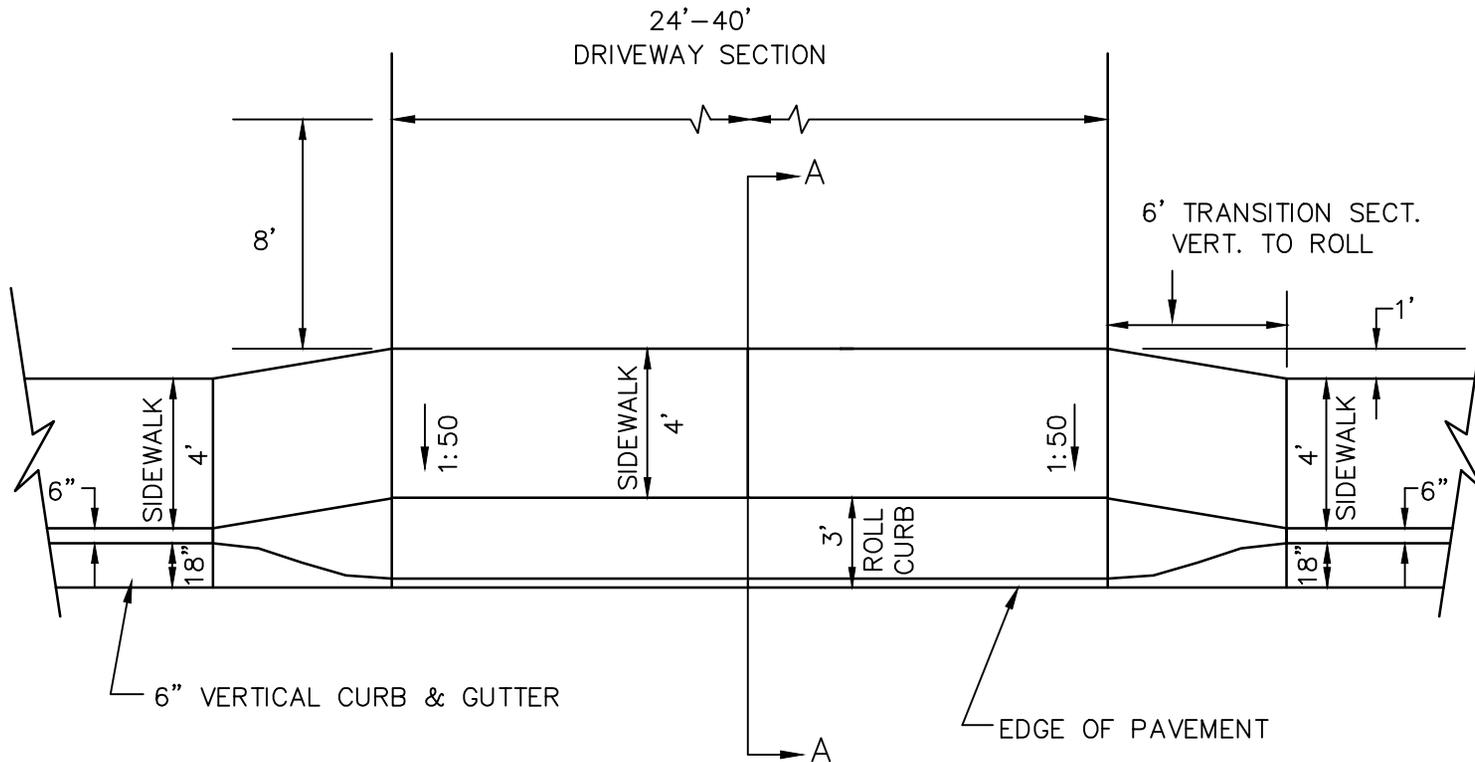
| COMMERCIAL AND INDUSTRIAL | | | | |
|-------------------------------|-------|------|-------|-----------|
| DRIVEWAY ENTRANCE WIDTH | MIN. | MAX. | CLASS | DEPTH 'X' |
| COMMERCIAL | * 16' | 40' | A | 9" |
| INDUSTRIAL | * 16' | 40' | A | 9" |
| *24' MIN. FOR TWO WAY TRAFFIC | | | | |
| RESIDENTIAL | | | | |
| DRIVEWAY ENTRANCE WIDTH | MIN. | MAX. | CLASS | DEPTH 'X' |
| ARTERIAL STREET | 16' | 30' | A | 6" |
| COLLECTOR STREET | * 12' | 30' | A | 6" |
| LOCAL STREET | 12' | 30' | A | 6" |
| *16' DESIRABLE | | | | |



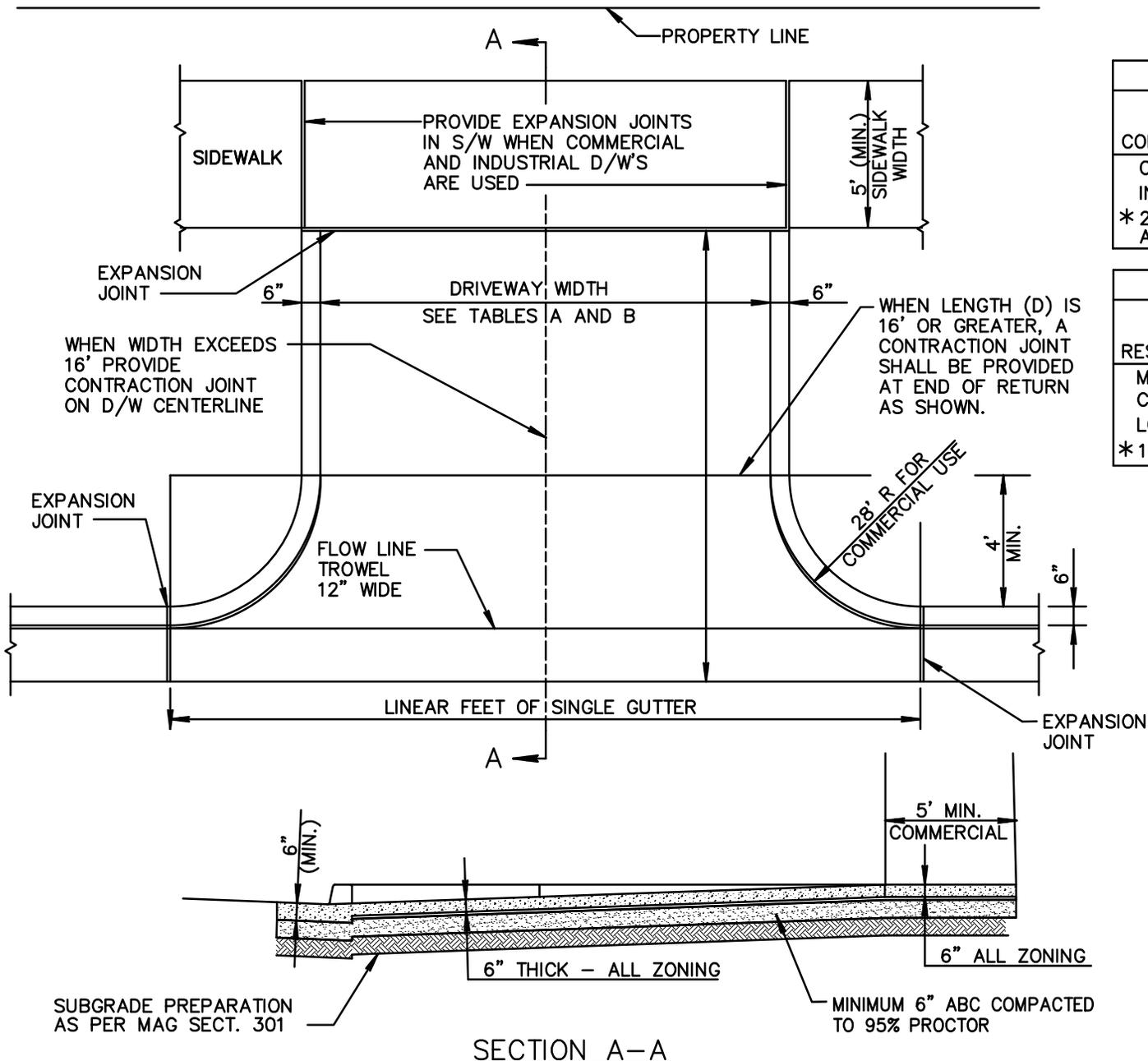
NOTES:

1. DEPRESSED CURB SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE TYPE OF CURB USED AT THAT LOCATION.
2. CONTRACTION JOINT(S) FOR DRIVEWAY ENTRANCE: WIDTH LESS THAN 22' NONE REQUIRED; WIDTH GREATER THAN 22' AND LESS THAN 30' LOCATE SINGLE JOINT ON D/W CENTERLINE; WIDTH OF 30' OR GREATER LOCATE TWO JOINTS TO EQUALLY DIVIDE THE DRIVEWAY ENTRANCE WIDTH.
3. DETAIL GEOMETRICS ARE BASED ON A CURB HEIGHT OF SIX INCHES (6"), AN ATTACHED SIDEWALK WIDTH OF FIVE FEET (5'), AND A DRIVEWAY RAMP LENGTH NOT EXCEEDING SIX FEET (6'). GEOMETRIC MODIFICATIONS MAY BE REQUIRED WHEN CONDITIONS ARE MODIFIED.
4. 1/2-INCH EXPANSION JOINTS SHALL COMPLY WITH MAG SECTION 340.
5. BACK OF CURB - CONSTRUCTION JOINT.
6. CLASS "AA" CONCRETE PER MAG SECTION 725.
7. SUBGRADE PREPARATION, MAG SECT. 301.
8. FLOW LINE OF GUTTER.
9. DEPRESSED CURB.
10. SECT. A-A AND ELEVATION: D/W SHOWN WITH VERTICAL CURB AND GUTTER, ROLL TYPE CURB AND GUTTER TREATED SIMILARLY.
11. ROUGH BROOM FINISH FULL WIDTH OF RAMP AND WINGS.
12. TROWEL AND USE LIGHT HAIR BROOM FINISH FOR WALKWAY AREA.
13. ELEVATION AT TOP OF DRIVEWAY RAMP SHALL BE EQUAL TO OR HIGHER THAN NORMAL CURB ELEVATION.

| COMMERCIAL AND INDUSTRIAL | | | | | RESIDENTIAL | | | | |
|-------------------------------|-------|------|-------|-----------|-------------------------|-------|------|-------|-----------|
| DRIVEWAY ENTRANCE WIDTH | MIN. | MAX. | CLASS | DEPTH 'X' | DRIVEWAY ENTRANCE WIDTH | MIN. | MAX. | CLASS | DEPTH 'X' |
| COMMERCIAL | * 16' | 40' | A | 9" | ARTERIAL STREET | 16' | 30' | A | 6" |
| INDUSTRIAL | * 16' | 40' | A | 9" | COLLECTOR STREET | * 12' | 30' | A | 6" |
| *24' MIN. FOR TWO WAY TRAFFIC | | | | | LOCAL STREET | 12' | 30' | A | 6" |
| | | | | | *16' DESIRABLE | | | | |



NOTE: ALL CONCRETE SHALL BE CLASS "A"

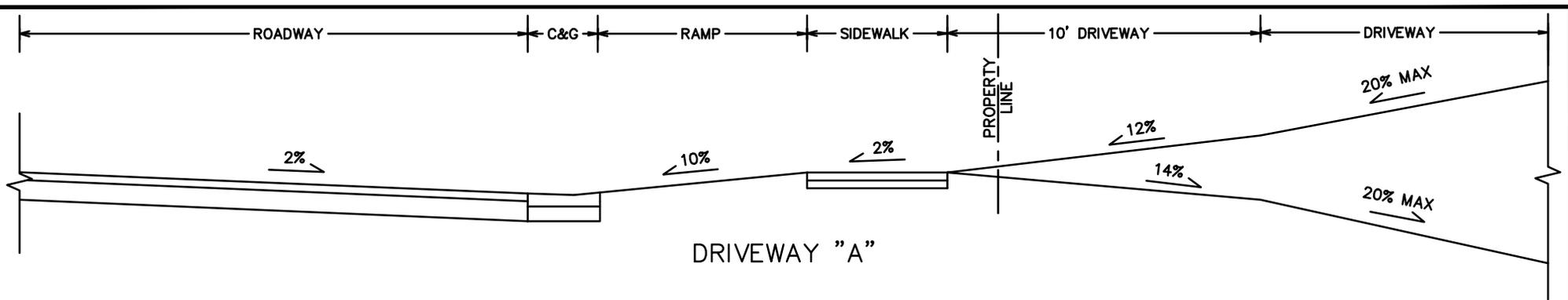


| TABLE A | | |
|--|----------------|------|
| ZONING | DRIVEWAY WIDTH | |
| | MIN.* | MAX. |
| COMMERCIAL AND INDUSTRIAL | 16' | 40' |
| COMMERCIAL | 16' | 40' |
| INDUSTRIAL | 16' | 40' |
| * 24' WHERE 2-WAY TRAFFIC IS ANTICIPATED | | |

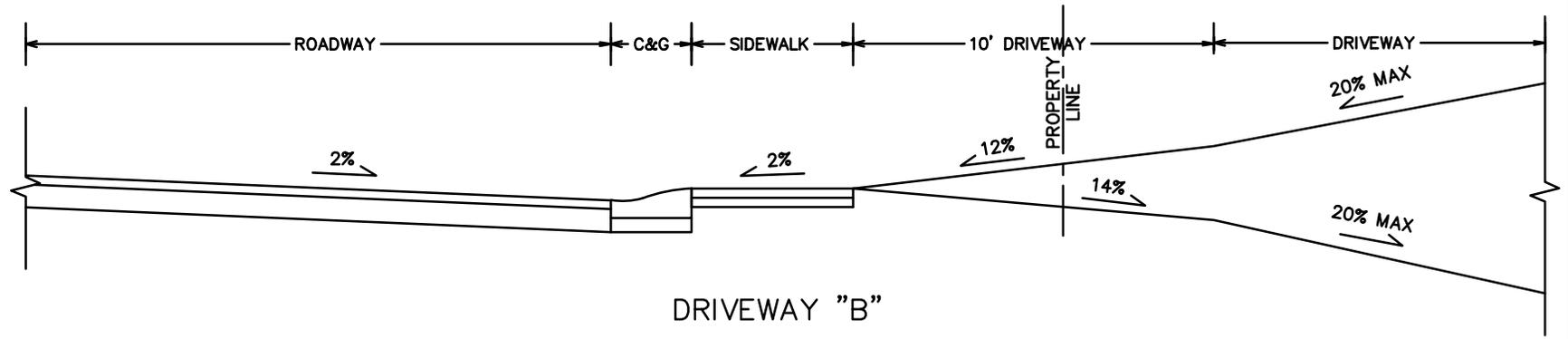
| TABLE B | | |
|--------------------------|----------------|------|
| ZONING | DRIVEWAY WIDTH | |
| | MIN.* | MAX. |
| RESIDENTIAL | 16' | 30' |
| MAJOR STREET | 12' | 30' |
| COLLECTOR STREET | 12' | 30' |
| LOCAL STREET | 12' | 30' |
| * 16' WIDTH IS DESIRABLE | | |

NOTES:

1. EXPANSION JOINTS SHALL COMPLY TO MAG SECTION 340.
2. THIS TYPE D/W TO BE USED ONLY UPON APPROVAL OF ENGINEER.
3. CLASS "AA" CONCRETE CONSTRUCTION AS PER MAG SECTION 725.
4. SUBGRADE TO BE COMPACTED TO 95% OF MAX PER MAG SECTION 301

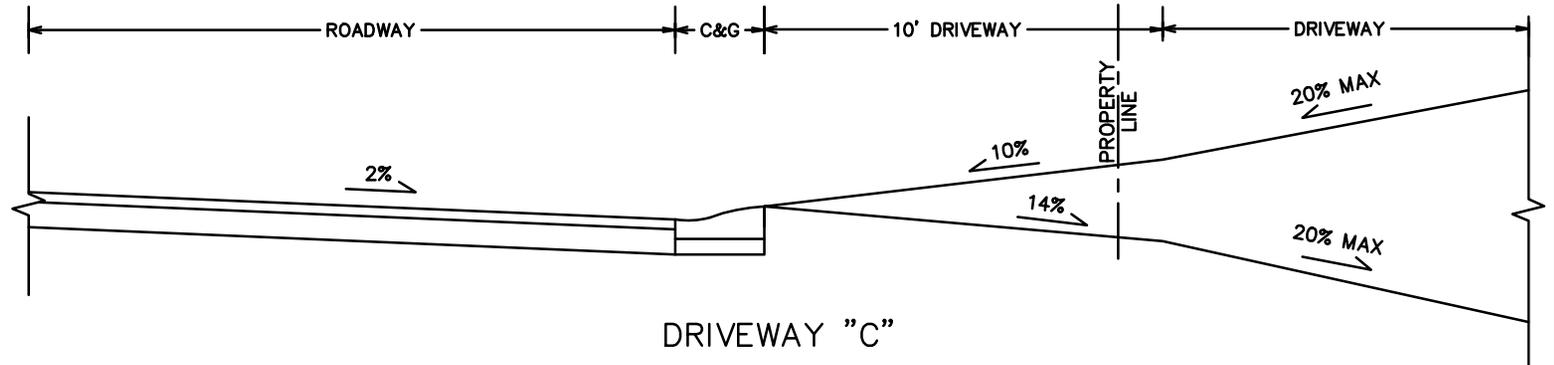


DRIVEWAY "A"

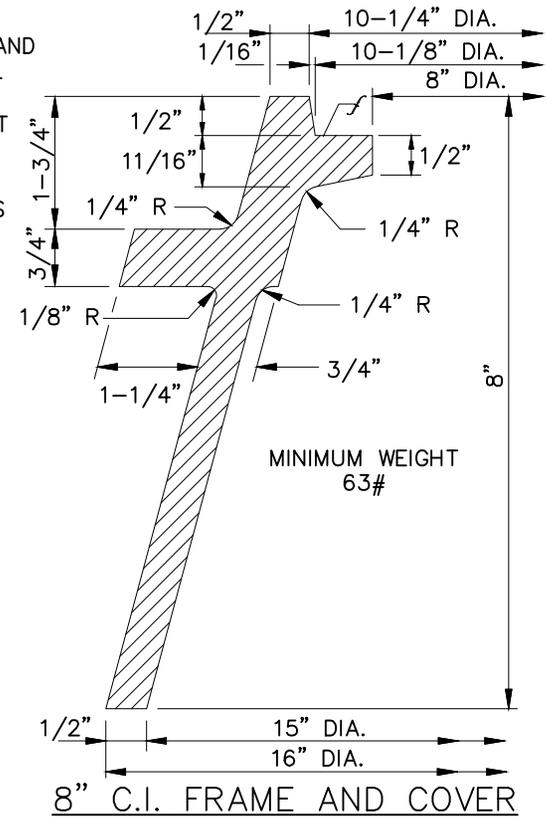
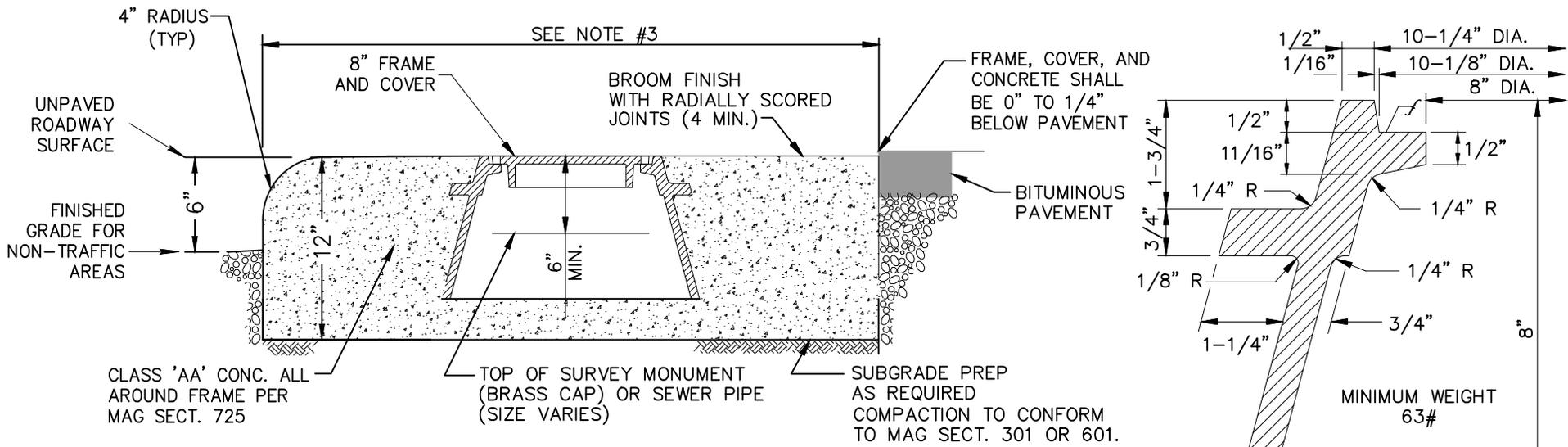


DRIVEWAY "B"

NOTE:
ALL SLOPE PERCENTAGES
ARE THE MAXIMUM
ALLOWED.



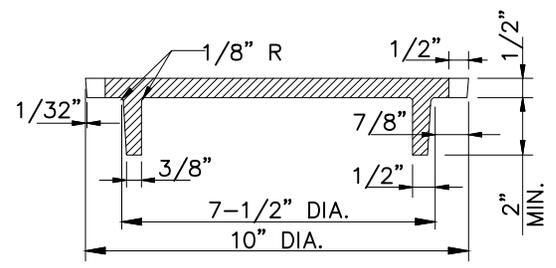
DRIVEWAY "C"



SURVEY MONUMENT OR SEWER
CLEAN OUT FRAME & GRADE ADJUSTMENT

NOTES:

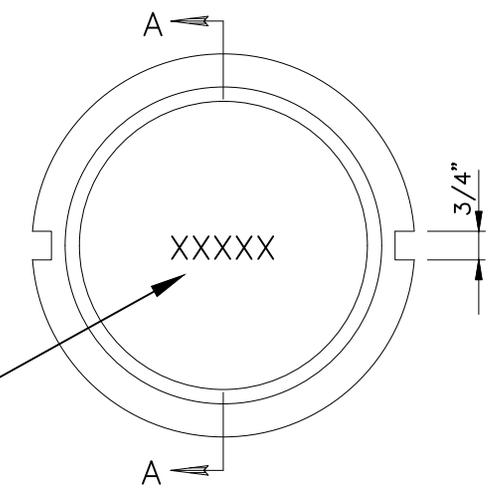
- LETTERS ON COVER TO BE AS FOLLOWS: "SEWER", "PRIVATE SEWER", OR "SURVEY" AS DIRECTED TOTAL WIDTH OF WORD "SEWER" 3-3/4". TOTAL WIDTH OF WORD "SURVEY" 4-1/2". LETTER SIZE 5/8" x 3/4", RAISED 1/16" ABOVE LEVEL OF COVER, TYPE OF LETTERS TO BE SUBMITTED FOR APPROVAL.
- U.S. MANUFACTURED IRON ONLY.
- CONCRETE COLLAR SHALL BE 40" DIAMETER IN VEHICULAR TRAFFIC AREAS INCLUDING ALL EASEMENTS, AND 24" DIAMETER IN NON TRAFFIC AREAS.



COVER SECTION A-A

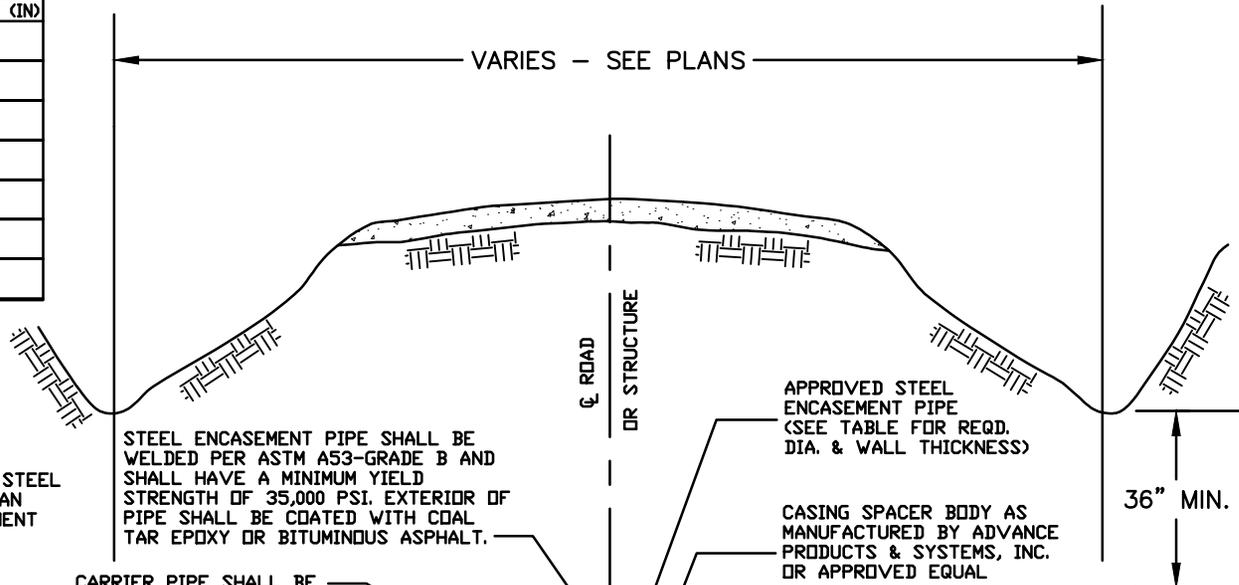
DETAIL TYPICAL FOR BOTH FRAME AND COVER

SEWER, SURVEY, PRIVATE SEWER, PER PLAN



| CARRIER PIPE SIZE (IN) | STEEL ENCASEMENT O.D. (IN MIN) | STEEL ENCASEMENT WALL THICKNESS (IN) |
|------------------------|--------------------------------|--------------------------------------|
| 6 | 14 | 1/4 |
| 8 | 18 | 1/4 |
| 12 | 21 | 1/4 |
| 18 | 27 | 3/8 |
| 21 | 30 | 3/8 |
| 24 | 36 | 3/8 |
| 27 | 39 | 1/2 |

1. FOR ALL CARRIER PIPE OVER 27" THE STEEL ENCASEMENT PIPE SHALL BE 12" LARGER THAN THE CARRIER PIPE AND THE STEEL ENCASEMENT WALL THICKNESS SHALL BE 1/2"

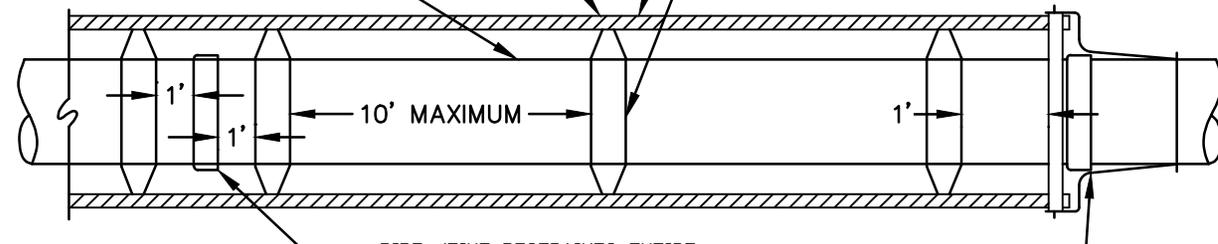


STEEL ENCASEMENT PIPE SHALL BE WELDED PER ASTM A53-GRADE B AND SHALL HAVE A MINIMUM YIELD STRENGTH OF 35,000 PSI. EXTERIOR OF PIPE SHALL BE COATED WITH COAL TAR EPOXY OR BITUMINOUS ASPHALT.

CARRIER PIPE SHALL BE DUCTILE IRON PIPE (DIP)

APPROVED STEEL ENCASEMENT PIPE (SEE TABLE FOR REQ. DIA. & WALL THICKNESS)

CASING SPACER BODY AS MANUFACTURED BY ADVANCE PRODUCTS & SYSTEMS, INC. OR APPROVED EQUAL



CARRIER PIPE SHALL BE DUCTILE IRON PIPE (DIP)

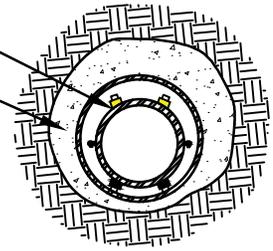
10' MAXIMUM

PIPE JOINT RESTRAINTS ENTIRE LENGTH OF ENCASEMENT

STAINLESS STEEL BAND CASING SPACERS MODEL SSI BY ADVANCE PRODUCTS & SYSTEMS, INC. OR APPROVED EQUAL

SEAL ENDS OF PIPE CASING WITH ADVANCE PRODUCTS & SYSTEMS, INC MODEL AC SEALS OR APPROVED EQUAL

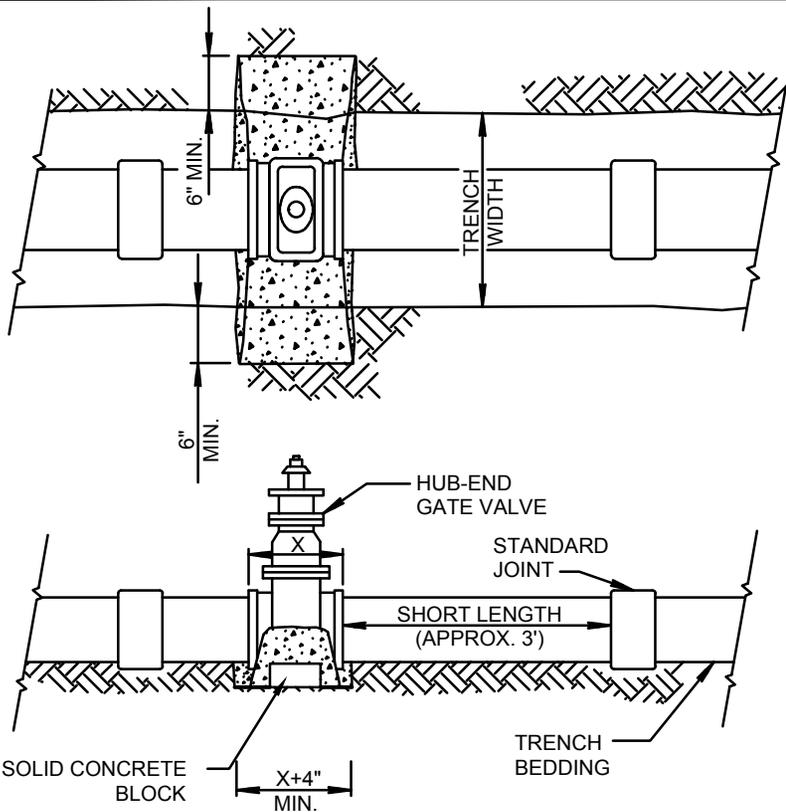
PRESSURE GROUT BETWEEN CASING AND SURROUNDING EARTH



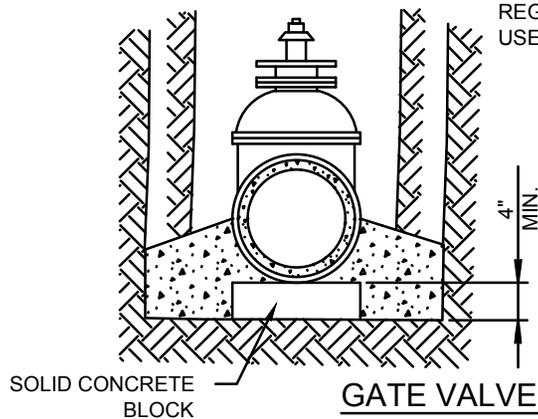
END VIEW

NOTES:

1. ALL FITTINGS SHALL BE RESTRAINED WITH FIELD LOC GASKETS OR APPROVED EQUAL
2. ALL BELLS SHOULD BE PROTECTED WITH MEGA-STOP SERIES 5000 BELL PROTECTION OR APPROVED EQUAL.
3. 1" MAXIMUM BETWEEN SPACERS AND CASING PIPE
4. INSTALLATION SHALL BE IN ACCORDANCE WITH ADVANCE PRODUCTS & SYSTEMS, INC. RECOMMENDATIONS OR APPROVED EQUAL.
5. TRACE WIRE PER AGENCY SPECIFICATIONS.
6. NO PIPE SHALL BE SUPPORTED BY BELL



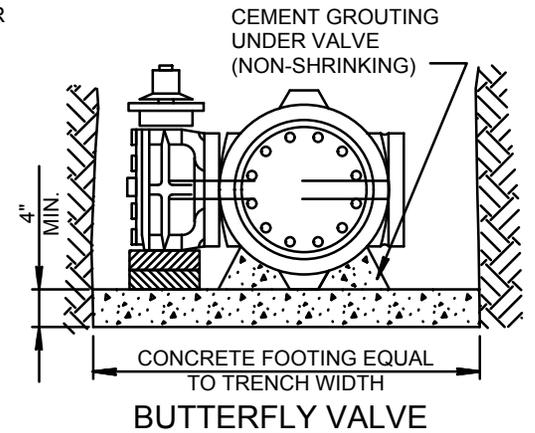
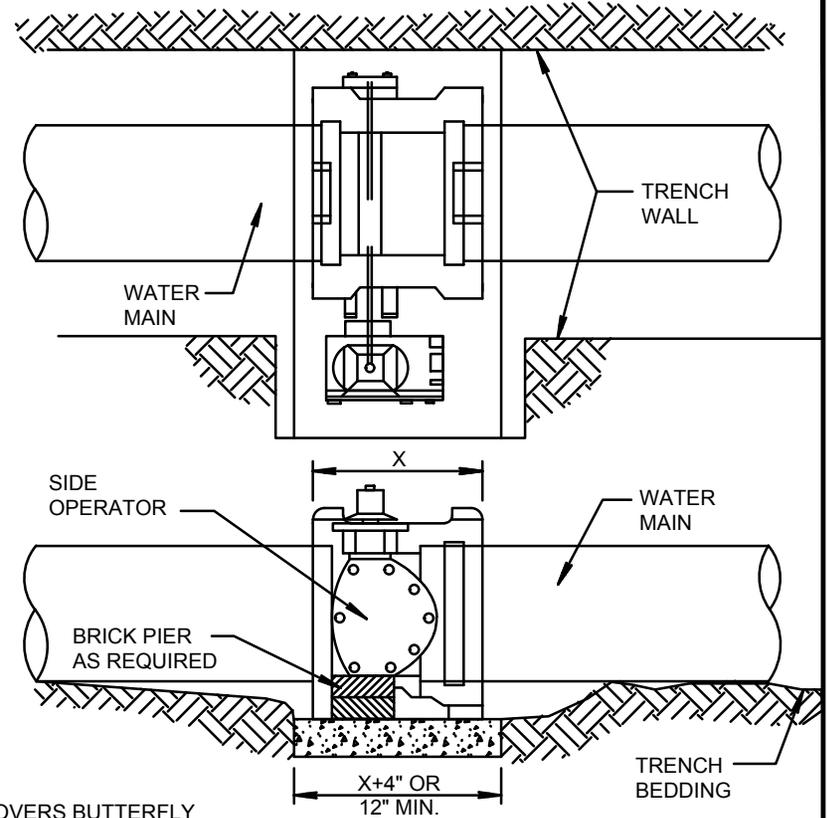
THIS DETAIL COVERS RESILIENT SEATED GATE VALVES, REGARDLESS OF TYPE OF PIPE USED.

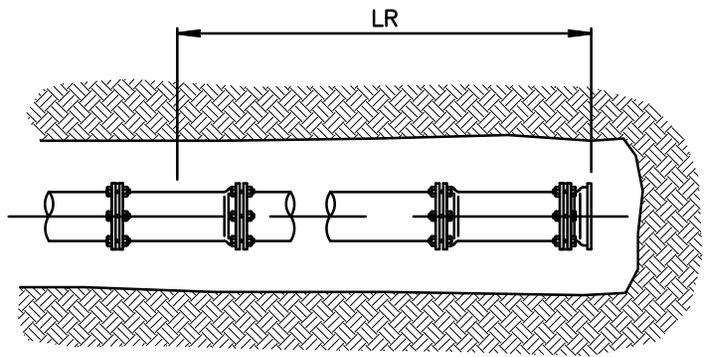


THIS DETAIL COVERS BUTTERFLY VALVE & HORIZONTALLY OPERATED GATE VALVE INSTALLATION, REGARDLESS OF TYPE OF PIPE OR JOINT USED.

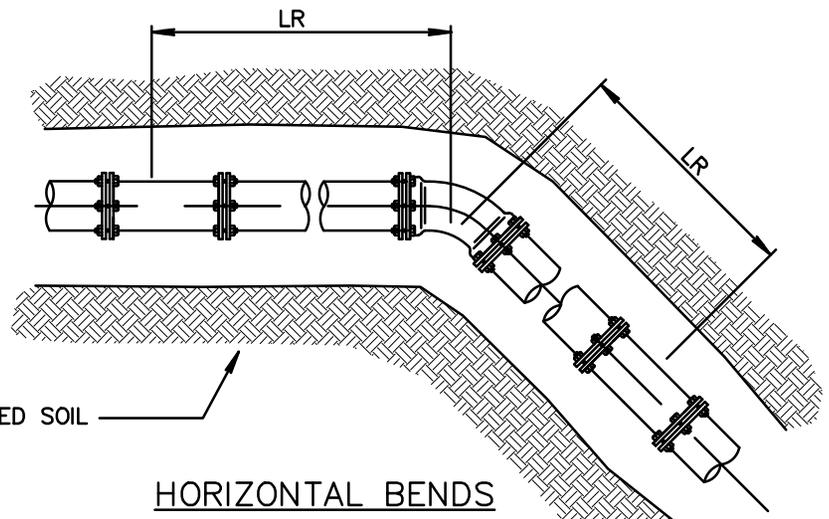
NOTES:

1. REFER TO APPROPRIATE AGENCY FOR VALVE BOX, COVER AND VALVE STABILIZER REQUIREMENTS.
2. ALL BOLTS AND JOINTS SHALL BE FREE AND CLEAR OF CONCRETE.
3. PROTECT ALL CONCRETE CONTACT AREAS WITH 8 MIL SHEET PLASTIC.
4. CLASS "B" CONCRETE AS PER MAG SECT. 725. FORM AS REQUIRED TO KEEP CLEAR OF JOINTS.



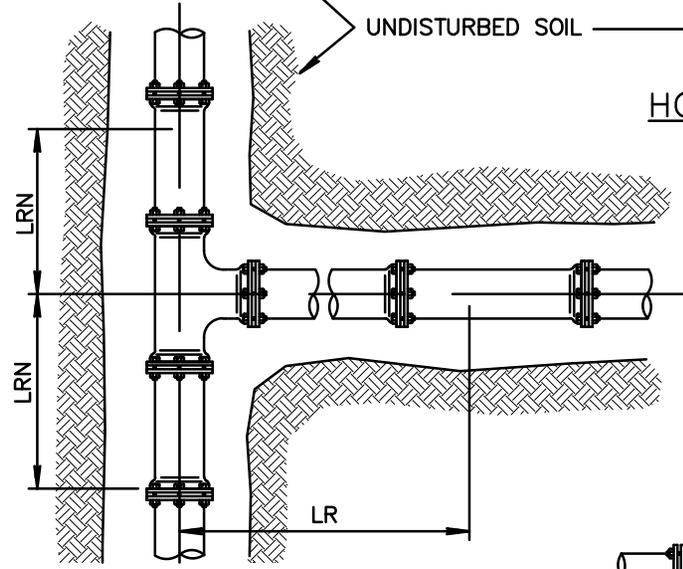


DEAD ENDS



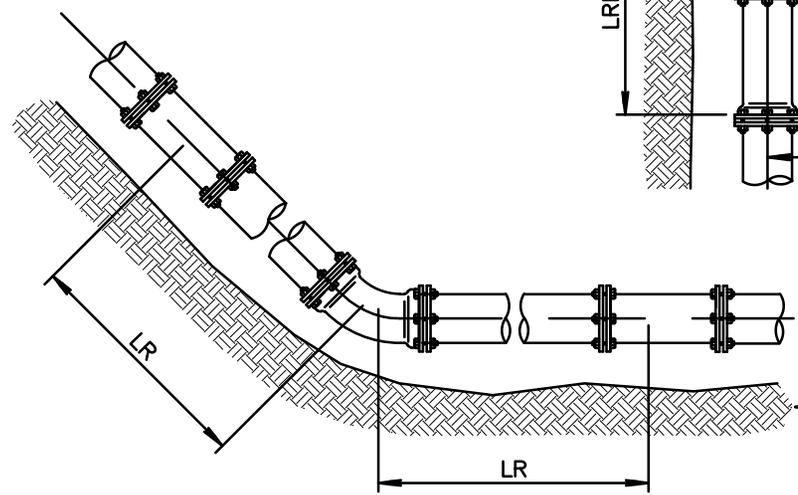
HORIZONTAL BENDS

LRN = SHORTEST LENGTH OF
PIPE RESTRAINED TO THE
RUN OF THE TEE FITTING
(BOTH SIDES OF TEE).

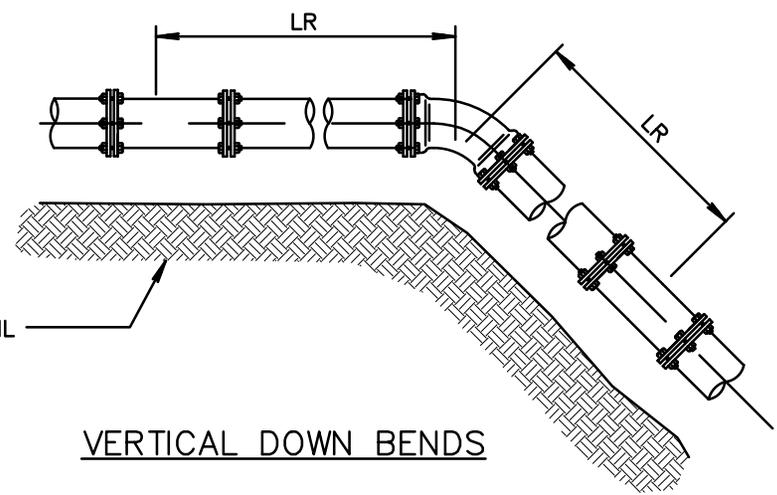


UNDISTURBED SOIL

TEES



VERTICAL UP BEND



VERTICAL DOWN BENDS

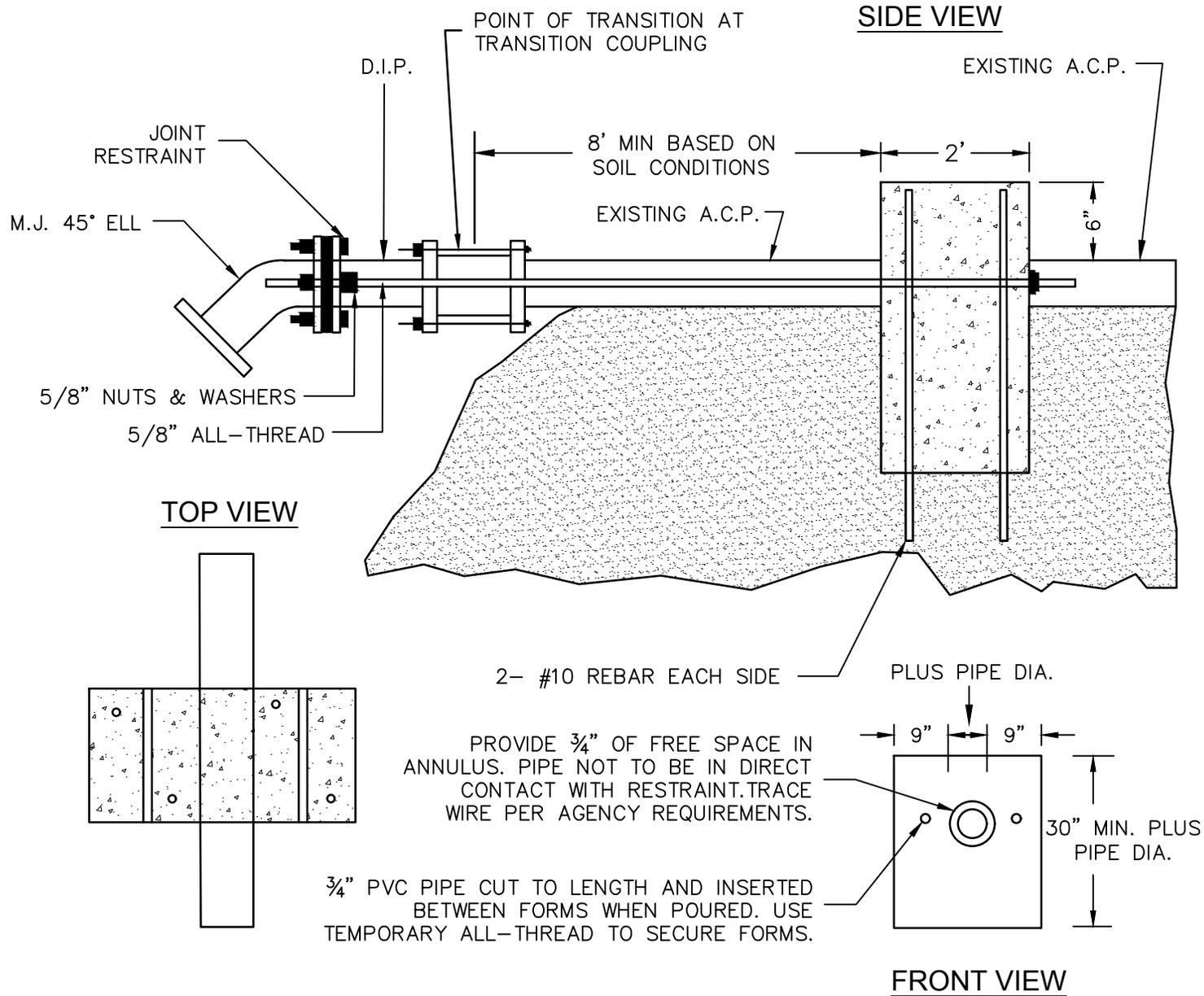
UNDISTURBED SOIL

| RESTRAINED LENGTHS, LR, FOR DUCTILE IRON PIPE | | | | | | | | | | | | |
|---|------------------|-----|---------|--------|---------|-------------------|------------|-------------------|------------|-----------------------|------------|-----------------------------|
| NOMINAL PIPE SIZE INCHES | HORIZONTAL BENDS | | | TEES | | VERTICAL OFFSETS | | | | | | VALVES & DEAD ENDS |
| | | | | | | 90° BEND FITTINGS | | 45° BEND FITTINGS | | 22-1/2° BEND FITTINGS | | |
| | 90° | 45° | 22-1/2° | LRN=0' | LRN=10' | DOWN BEND | UP BEND | DOWN BEND | UP BEND | DOWN BEND | UP BEND | |
| 4 | 18 | 7 | 4 | 30 | 8 | 31 | 18 | 13 | 7 | 6 | 3 | 31 |
| 6 | 25 | 10 | 5 | 43 | 20 | 44 | 25 | 18 | 10 | 9 | 5 | 44 |
| 8 | 32 | 13 | 6 | 56 | 34 | 58 | 32 | 24 | 13 | 11 | 6 | 58 |
| 10 | 38 | 16 | 8 | 68 | 45 | 69 | 38 | 29 | 16 | 14 | 8 | 69 |
| 12 | 45 | 19 | 9 | 80 | 57 | 81 | 45 | 34 | 19 | 16 | 9 | 81 |
| 14 | 51 | 21 | 10 | 91 | 68 | 92 | 51 | 38 | 21 | 18 | 10 | 92 |
| 16 | 57 | 24 | 11 | 103 | 79 | 104 | 57 | 43 | 24 | 21 | 11 | 104 |
| 18 | 62 | 26 | 12 | 113 | 90 | 115 | 62 | 48 | 26 | 23 | 12 | 115 |
| 20 | 68 | 28 | 14 | 125 | 100 | 126 | 68 | 52 | 28 | 25 | 14 | 126 |
| 24 | 79 | 33 | 16 | 145 | 121 | 147 | 79 | 61 | 33 | 29 | 16 | 147 |

| RESTRAINED LENGTHS, LR, FOR DUCTILE IRON PIPE WITH POLYETHYLENE WRAP OR PVC | | | | | | | | | | | | |
|---|------------------|-----|---------|--------|---------|-------------------|------------|-------------------|------------|-----------------------|------------|-----------------------------|
| NOMINAL PIPE SIZE INCHES | HORIZONTAL BENDS | | | TEES | | VERTICAL OFFSETS | | | | | | VALVES & DEAD ENDS |
| | | | | | | 90° BEND FITTINGS | | 45° BEND FITTINGS | | 22-1/2° BEND FITTINGS | | |
| | 90° | 45° | 22-1/2° | LRN=0' | LRN=10' | DOWN BEND | UP BEND | DOWN BEND | UP BEND | DOWN BEND | UP BEND | |
| 4 | 26 | 11 | 5 | 69 | 18 | 72 | 26 | 30 | 11 | 14 | 5 | 72 |
| 6 | 36 | 15 | 7 | 99 | 47 | 102 | 36 | 42 | 15 | 20 | 7 | 102 |
| 8 | 47 | 19 | 9 | 130 | 78 | 133 | 47 | 55 | 19 | 26 | 9 | 133 |
| 10 | 56 | 23 | 11 | 157 | 103 | 159 | 56 | 66 | 23 | 32 | 11 | 159 |
| 12 | 65 | 27 | 13 | 185 | 131 | 187 | 65 | 77 | 27 | 37 | 13 | 187 |
| 14 | 74 | 31 | 15 | 211 | 156 | 214 | 74 | 89 | 31 | 42 | 15 | 214 |
| 16 | 82 | 34 | 16 | 238 | 183 | 241 | 82 | 100 | 34 | 48 | 16 | 241 |
| 18 | 90 | 37 | 18 | 263 | 207 | 266 | 90 | 110 | 38 | 53 | 18 | 266 |
| 20 | 98 | 41 | 20 | 289 | 233 | 292 | 98 | 121 | 41 | 58 | 20 | 292 |
| 24 | 113 | 47 | 22 | 337 | 280 | 340 | 113 | 141 | 47 | 68 | 22 | 340 |

NOTES:

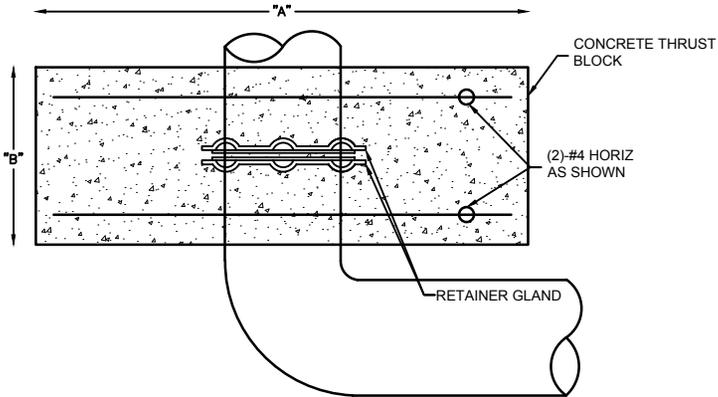
1. ALL JOINTS WITHIN THE SPECIFIED LENGTH LR MUST BE RESTRAINED.
ALL LENGTHS ARE GIVEN IN FEET.
2. THE MAXIMUM TEST PRESSURE SHALL NOT EXCEED 200 PSI
3. THE MINIMUM DEPTH OF BURY SHALL BE 4' TO TOP OF PIPE.



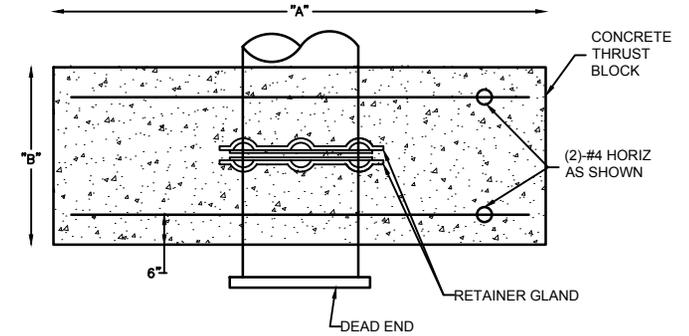
| PIPE SIZE | A | B | VERT REINF | C | VOL (yd ³) | WEIGHT (LBS) | THRUST (LBS) |
|-----------|-------|-------|------------|-------|------------------------|--------------|--------------|
| 4" Ø | 3'-0" | 1'-6" | (2)-#4 | 1'-4" | 0.23 | 932 | 3844 |
| 6" Ø | 3'-8" | 2'-0" | (2)-#4 | 2'-0" | 0.55 | 2228 | 8594 |
| 8" Ø | 4'-6" | 2'-0" | (2)-#4 | 3'-0" | 1.00 | 4050 | 15414 |
| 10" Ø | 6'-0" | 2'-4" | (4)-#4 | 3'-4" | 1.73 | 7007 | 23751 |
| 12" Ø | 6'-6" | 2'-4" | (4)-#4 | 4'-6" | 2.53 | 10247 | 34157 |
| 14" Ø | 7'-0" | 2'-6" | (4)-#4 | 5'-8" | 3.68 | 14904 | 46448 |
| 16" Ø | 8'-0" | 2'-9" | (6)-#4 | 6'-4" | 5.16 | 20898 | 60772 |
| 18" Ø | 9'-0" | 3'-0" | (6)-#4 | 7'-2" | 7.17 | 29039 | 77018 |

- NOTES:
1. CONCRETE SHALL BE MAG CLASS "B".
 2. CONCRETE SHALL BE POURED AGAINST UNDISTURBED EARTH EXCEPT WHERE FORMWORK IS A MUST. AFTER CONCRETE IS CURED THE FORMWORK SHALL BE REMOVED.
 3. PIPE MUST BE DUCTILE IRON.
 4. BASED ON 200 PSI TEST PRESSURE.
 5. HORIZONTAL AND VERTICAL BENDS OPTIONAL.
 6. RETAINER GLAND = FORD SERIES 1300 UNI-FLANGE RETAINER GLAND OR EQUAL (TYP).

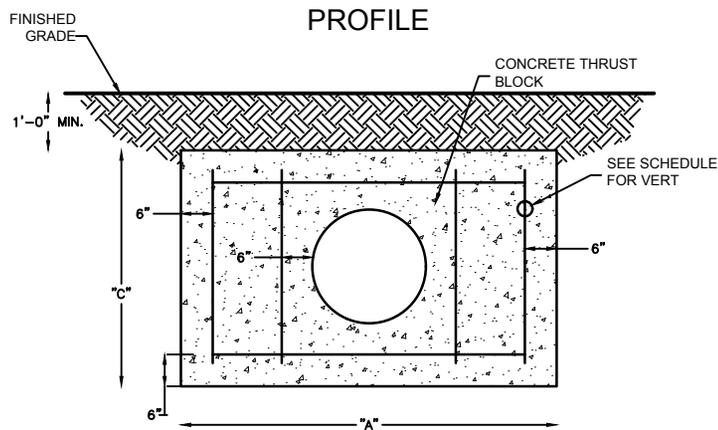
| PIPE SIZE | A | B | VERT REINF | C | VOL (yd ³) | WEIGHT (LBS) | THRUST (LBS) |
|-----------|-------|-------|------------|-------|------------------------|--------------|--------------|
| 4" Ø | 2'-0" | 1'-6" | (2)-#4 | 1'-4" | 0.15 | 608 | 2718 |
| 6" Ø | 3'-0" | 1'-8" | (2)-#4 | 2'-0" | 0.38 | 1539 | 6077 |
| 8" Ø | 3'-6" | 2'-0" | (2)-#4 | 2'-8" | 0.70 | 2835 | 10900 |
| 10" Ø | 5'-0" | 2'-2" | (4)-#4 | 3'-0" | 1.21 | 4901 | 16794 |
| 12" Ø | 5'-6" | 2'-4" | (4)-#4 | 4'-0" | 1.90 | 7695 | 24153 |
| 14" Ø | 6'-0" | 2'-4" | (4)-#4 | 5'-0" | 2.59 | 10490 | 32844 |
| 16" Ø | 6'-6" | 2'-8" | (6)-#4 | 5'-8" | 3.65 | 14783 | 42973 |
| 18" Ø | 7'-0" | 3'-0" | (6)-#4 | 6'-6" | 5.06 | 20493 | 54460 |



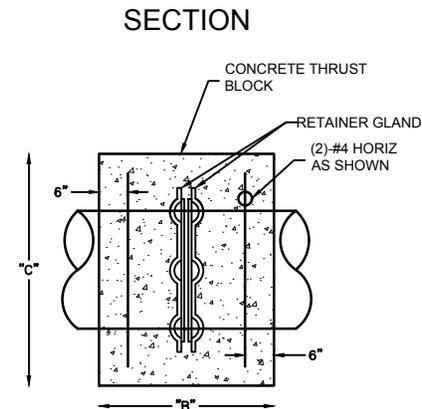
THRUST BLOCK AT ANGLE



THRUST BLOCK AT DEAD END



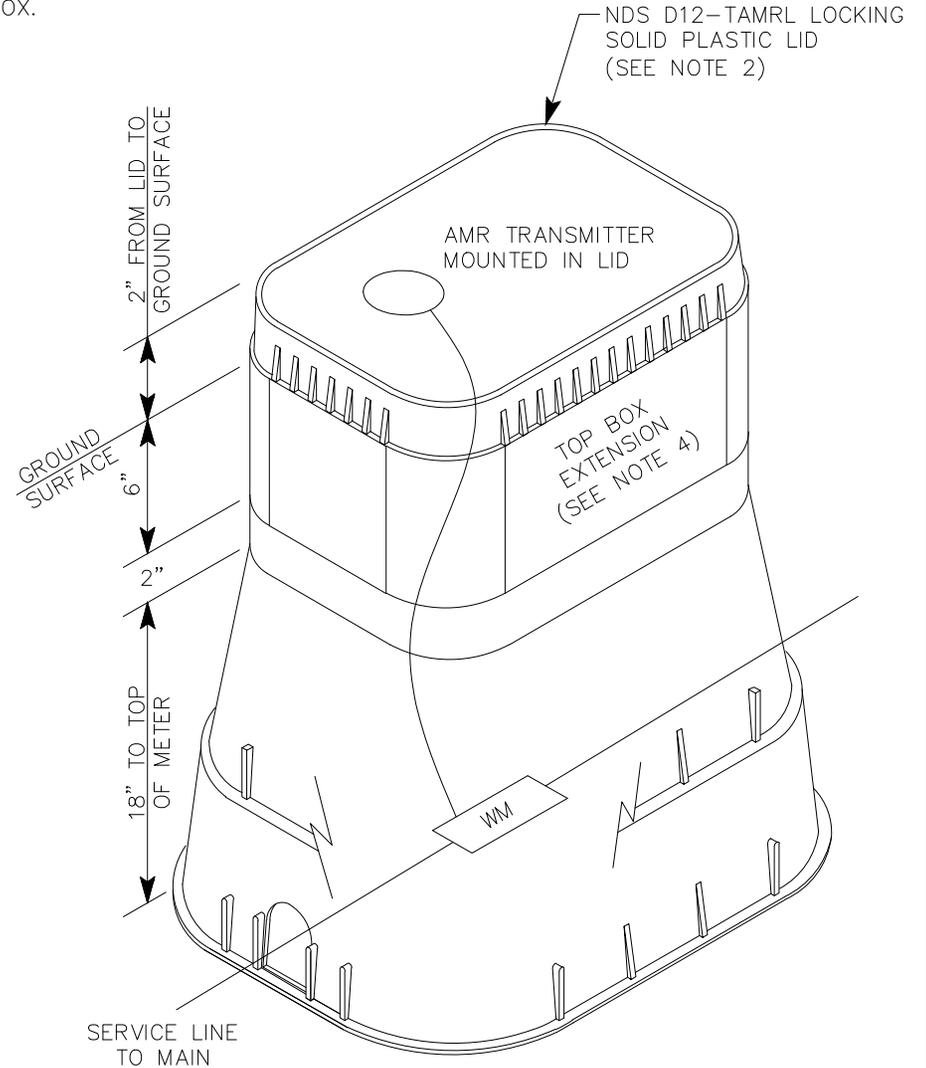
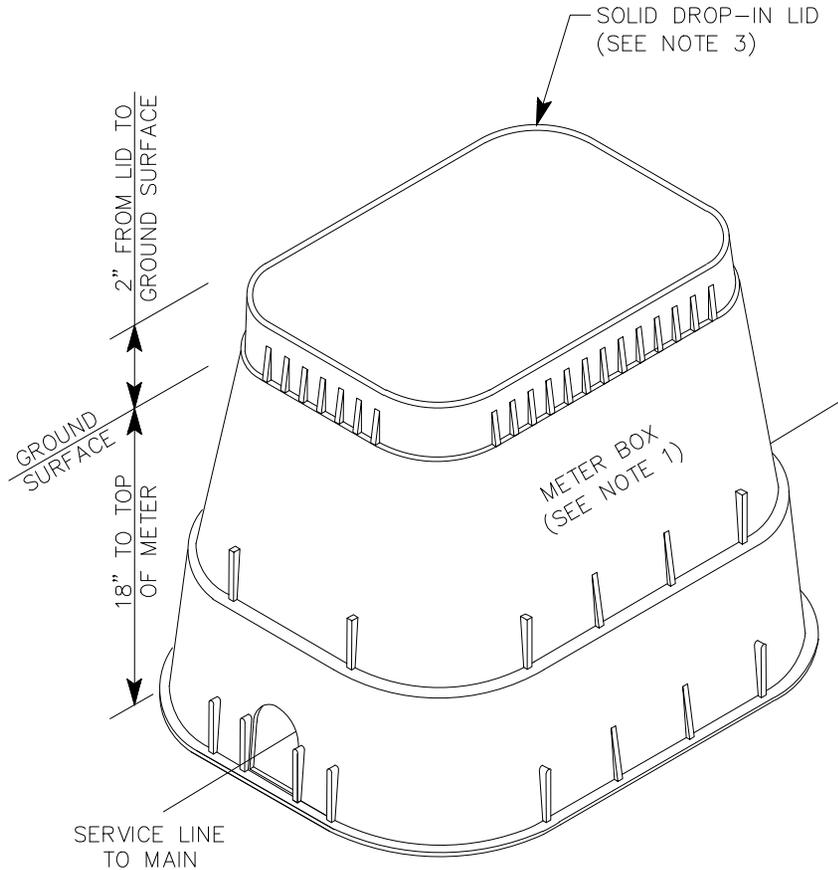
PROFILE



SECTION

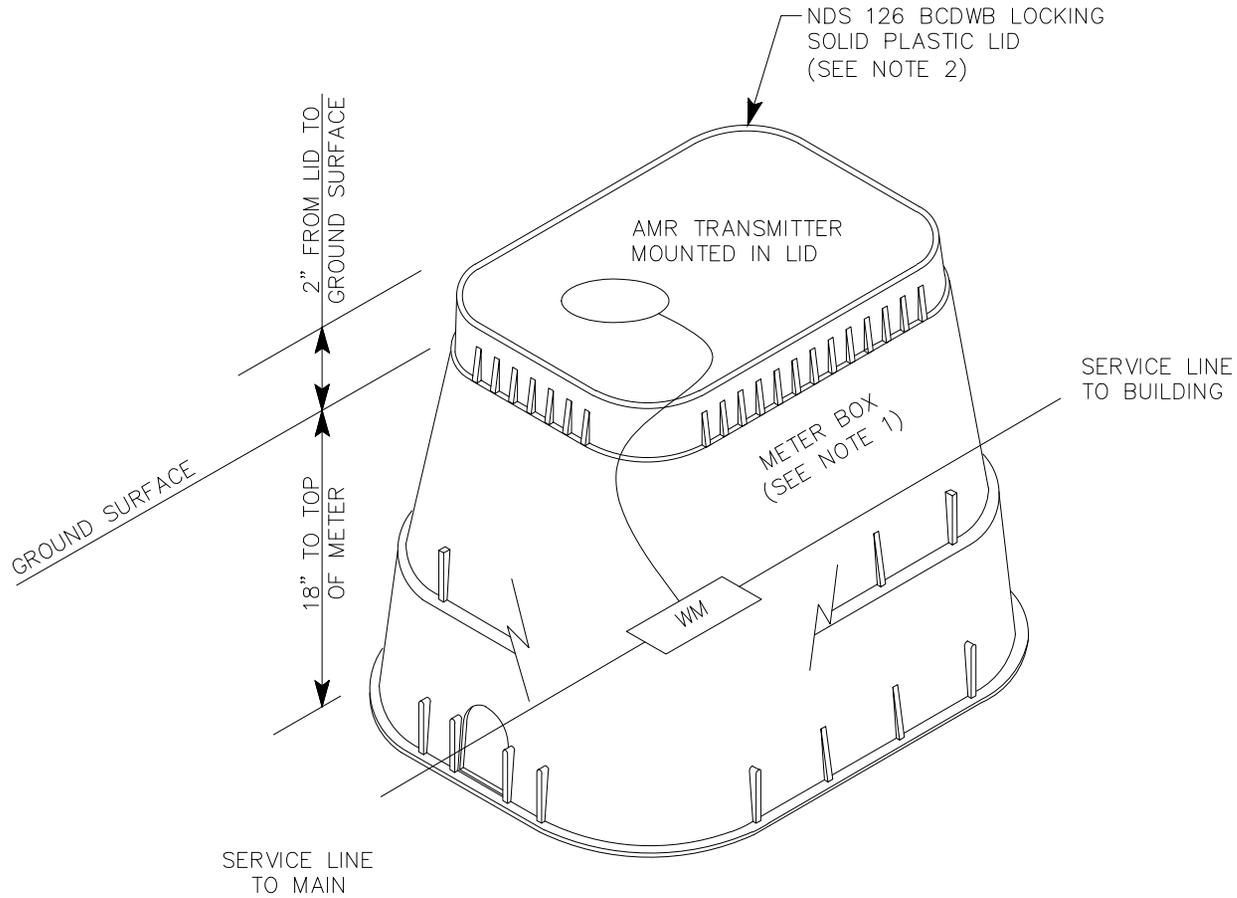
NOTES

1. NDS D1800 B/O (METER BOX ONLY) OR APPROVED EQUAL – 14"Wx19"Lx18"D.
2. FOR SINGLE BOX APPLICATION, USE NDS D12-TAMRL LOCKING SOLID PLASTIC LID WITH 2 INCH AMR/TRANSMITTER HOLE WITH 7 INCH TOP RECESS COVER.
3. FOR DUAL BOX (SIDE BY SIDE METERS), USE SOLID DROP-IN LID ON THE SECOND BOX.
4. NDS D600E-EXT-TOP BOX EXTENSION OR APPROVED EQUAL.



NOTES

1. NDS 17" WIDE x 30" LONG METER BOX OR APPROVED EQUAL.
2. NDS 126 BCDWB METER BOX, BOLT DOWN DROP-IN SOLID PLASTIC LID OR APPROVED EQUAL.



PV STANDARD DETAIL

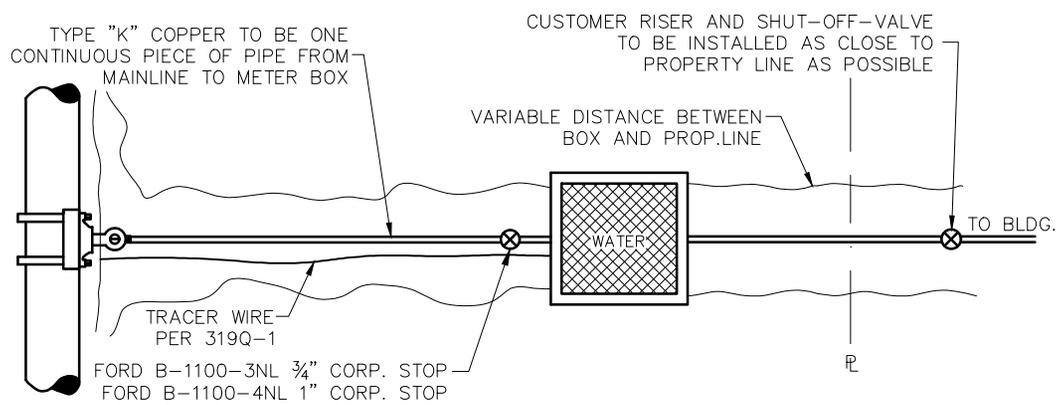
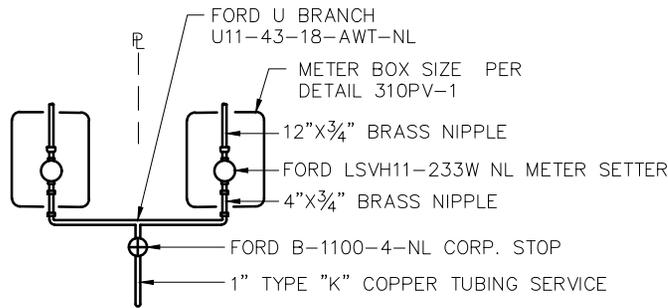
1½" THROUGH 2"
METER BOX APPLICATIONS

TOWN ENGINEER

REVISED:
07/16

DETAIL No.
310PV-2

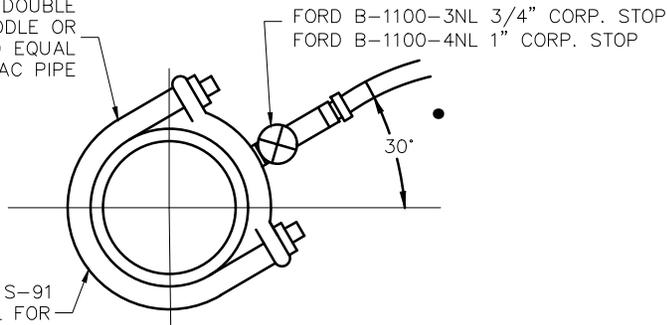
THIS DETAIL ONLY FOR
5/8" & 3/4" SERVICE FOR TWO LOTS



— PLAN VIEW —



USE FORD STYLE 202B DOUBLE STRAP BRASS SADDLE OR APPROVED EQUAL FOR DIP OR AC PIPE

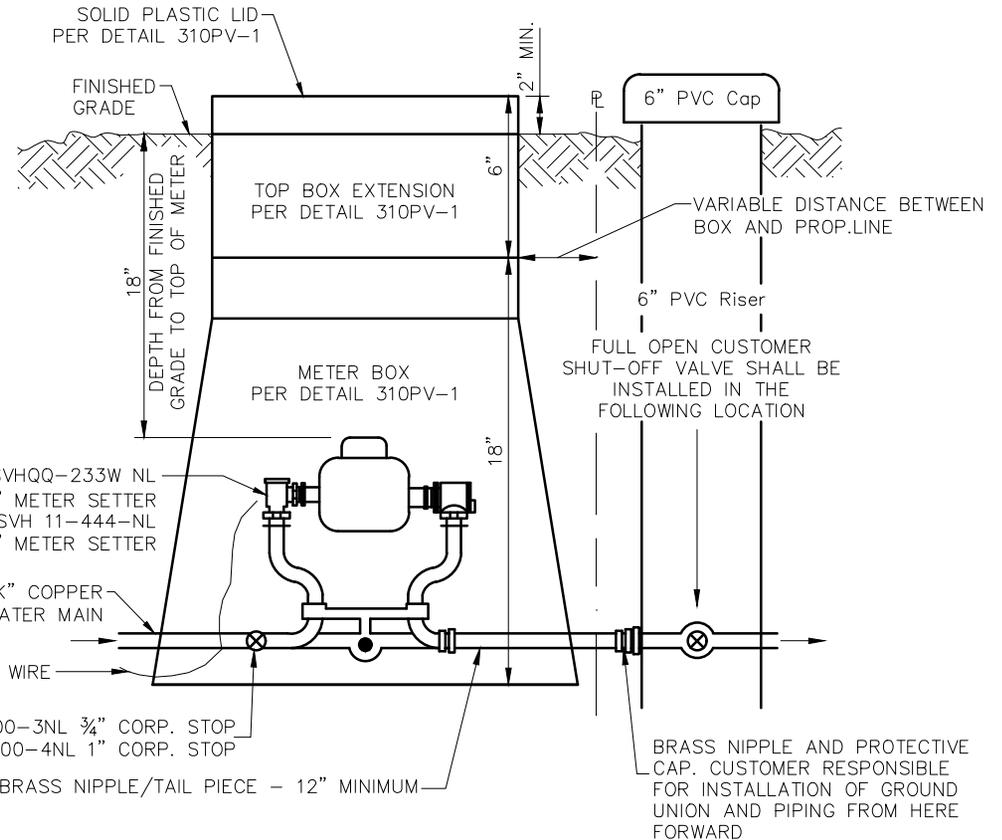


USE FORD STYLE S-90 OR S-91 OR APPROVED EQUAL FOR C-900 OR C-905 PVC

— SIDE VIEW —

NOTES

1. ALL BRASS PARTS LISTED ARE FORD METER BOX COMPANY OR APPROVED EQUAL.
2. SERVICE LINE SHALL BE TYPE "K" COPPER CONTAINING NO SPLICES.
3. METER BOX AND LID SHALL BE NDS OR APPROVED EQUAL.
4. METER BOXES SHALL BE PER DETAIL 310PV-1.
5. REFER TO DETAIL 200Q FOR WATER SERVICE BEDDING AND SHADING.
6. METER BOX SHALL NOT BE LOCATED IN DRIVEWAYS, SIDEWALKS OR STREETS.
7. NO LANDSCAPE FEATURES OR VEGETATION LOCATED WITHIN 5' OF METER BOXES.
8. SERVICE LINE IS TO HAVE A MIN. 3 FT COVER WITHIN ROAD PRISM.
9. FOR CUT OR FILL SLOPE METER LOCATIONS, CMU WALL OR APPROVED METHOD SHALL BE REQUIRED FOR PROTECTION OF METER SET.



— PROFILE VIEW —

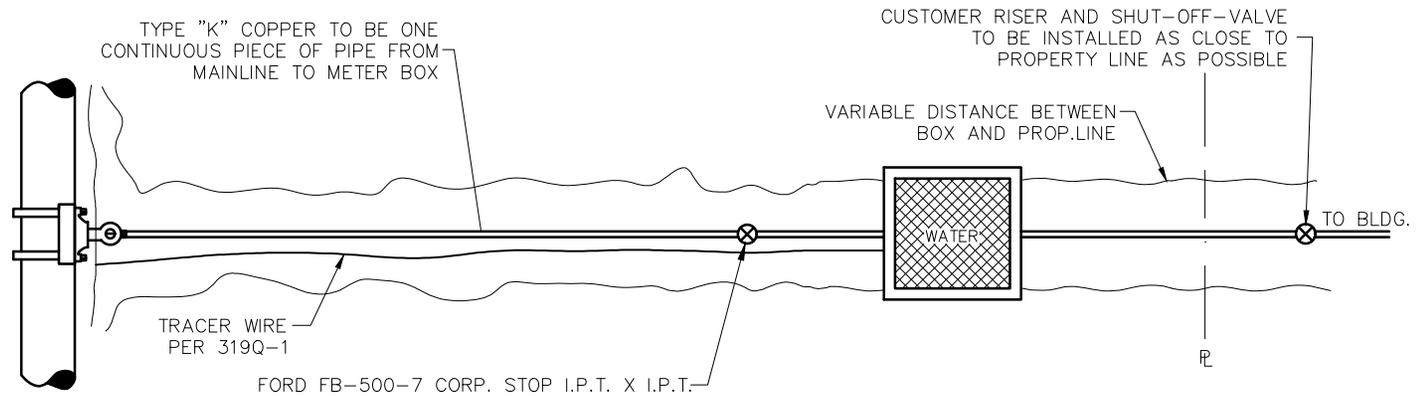
PV STANDARD DETAIL

5/8" THROUGH 1"
WATER SERVICE CONNECTION

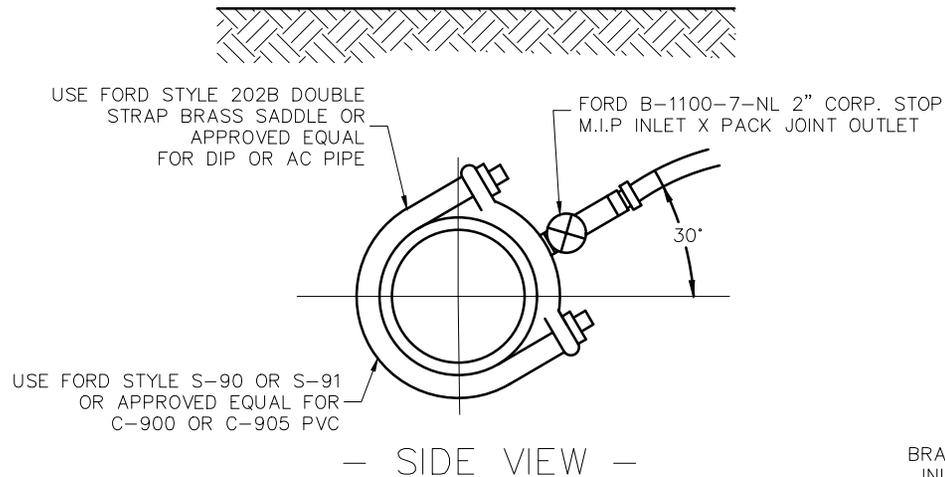
TOWN ENGINEER

REVISED:
07/16

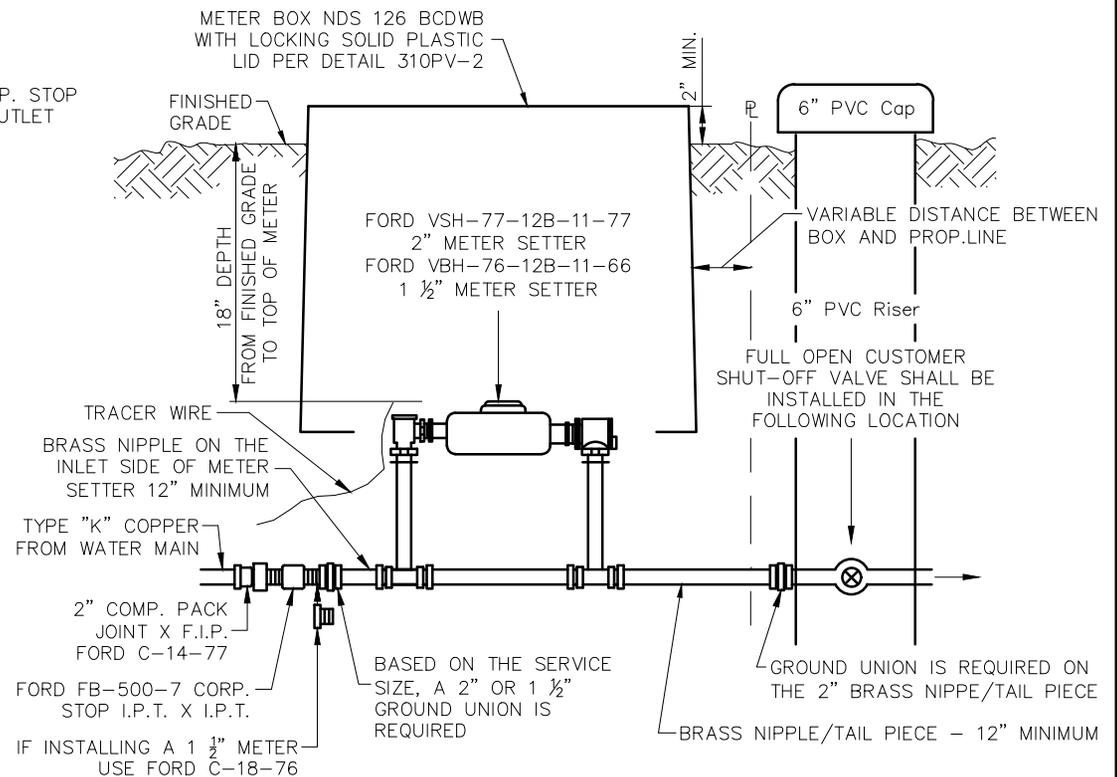
DETAIL No.
310PV-3



— PLAN VIEW —



— SIDE VIEW —



— PROFILE VIEW —

NOTES

1. ALL BRASS PARTS LISTED ARE FORD METER BOX COMPANY OR APPROVED EQUAL.
2. SERVICE LINE SHALL BE TYPE "k" COPPER CONTAINING NO SPLICES.
3. METER BOX AND LID SHALL BE NDS OR APPROVED EQUAL.
4. METER BOXES SHALL BE PER DETAIL 310PV-2.
5. REFER TO DETAIL 200Q FOR WATER SERVICE BEDDING AND SHADING.
6. METER BOX SHALL NOT BE LOCATED IN DRIVEWAYS, SIDEWALKS OR STREETS.
7. NO LANDSCAPE FEATURES OR VEGETATION LOCATED WITHIN 5' OF METER BOXES.
8. SERVICE LINE IS TO HAVE A MIN. 3 FT COVER WITHIN ROAD PRISM.
9. FOR CUT OR FILL SLOPE METER LOCATIONS, CMU WALL OR APPROVED METHOD SHALL BE REQUIRED FOR PROTECTION OF METER SET.
10. ALL 1 1/2" AND 2" SERVICES SHALL BE INSTALLED USING 2" SERVICE LINE FROM THE MAINLINE TO THE CORP STOP PRIOR TO THE METER.

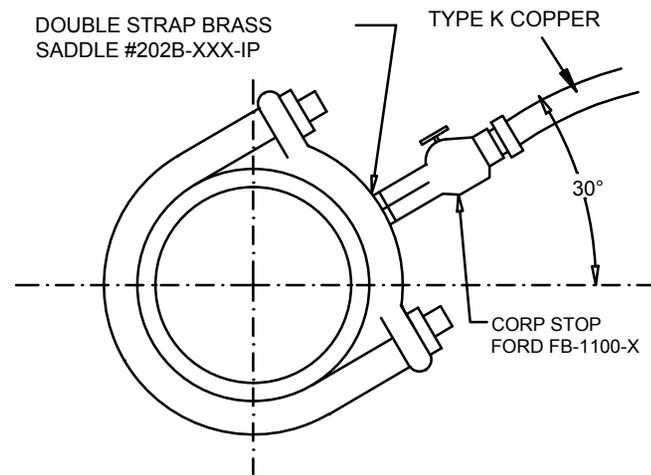
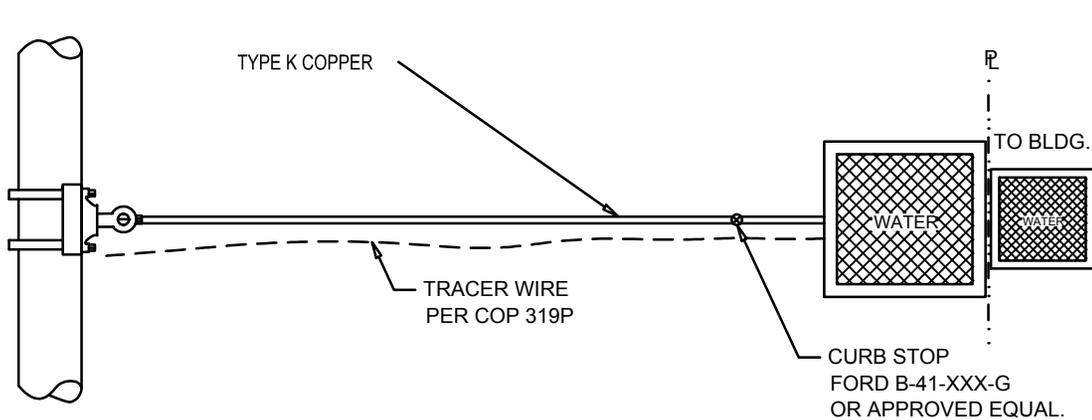
PV STANDARD DETAIL

**1 1/2" THROUGH 2"
WATER SERVICE CONNECTIONS**

TOWN ENGINEER

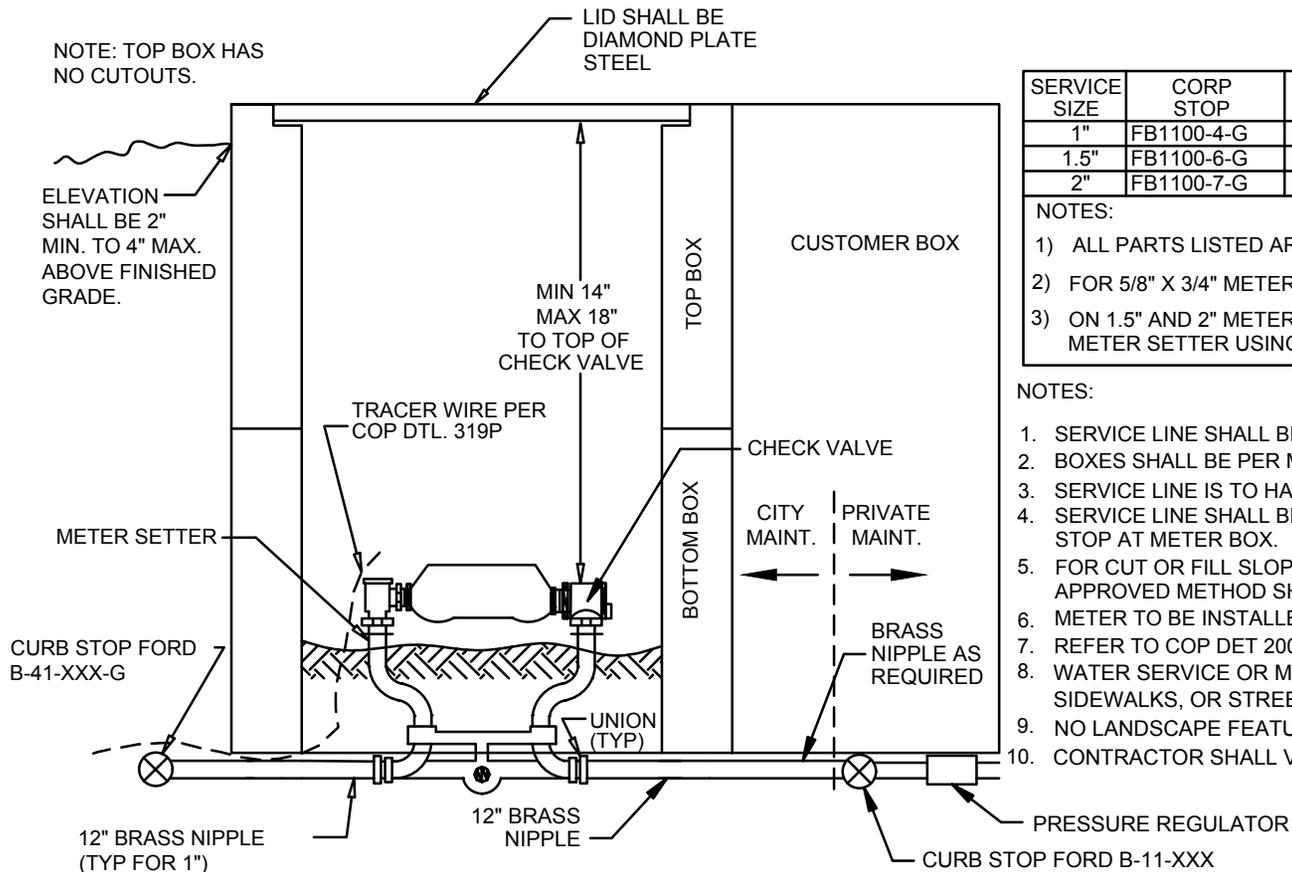
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07/16

DETAIL No.
310PV-4



NOTE: TOP BOX HAS NO CUTOUTS.

LID SHALL BE DIAMOND PLATE STEEL



| SERVICE SIZE | CORP STOP | CURB STOP | METER SETTER | CITY BOX | CURB STOP | CUSTOMER BOX |
|--------------|------------|-----------|-----------------|----------|-----------|--------------|
| 1" | FB1100-4-G | B41-444-G | VBH74-9W-11-44 | #2 | B11-444 | #1 |
| 1.5" | FB1100-6-G | B41-666-G | VBH76-12W-11-66 | #4 | B11-666 | #2 |
| 2" | FB1100-7-G | B41-777-G | VBH77-12W-11-77 | #4 | B11-777 | #2 |

NOTES:

- 1) ALL PARTS LISTED ARE FORD METER BOX CO. OR APPROVED EQUAL.
- 2) FOR 5/8" X 3/4" METERS, INSTALL FORD A24 METER ADAPTERS.
- 3) ON 1.5" AND 2" METERS INSTALL UNIONS ON EACH SIDE OF THE METER SETTER USING 6" BRASS NIPPLES IN-LIEU OF 12" BRASS NIPPLES.

NOTES:

1. SERVICE LINE SHALL BE TYPE K COPPER WITH 8 MIL POLYWRAP.
2. BOXES SHALL BE PER MAG STD. DETAIL 320.
3. SERVICE LINE IS TO HAVE MIN. 3 FT. COVER WITHIN THE ROADWAY PRISM.
4. SERVICE LINE SHALL BE UNSPLICED FROM CORP. STOP AT MAIN TO CURB STOP AT METER BOX.
5. FOR CUT OR FILL SLOPE METER LOCATIONS, CMU WALL OR OTHER APPROVED METHOD SHALL BE REQUIRED FOR PROTECTION OF METER SET.
6. METER TO BE INSTALLED BY C.O.P. ONLY.
7. REFER TO COP DET 200P FOR WATER SERVICE BEDDING AND SHADING.
8. WATER SERVICE OR METER BOX SHALL NOT BE LOCATED IN DRIVEWAYS, SIDEWALKS, OR STREETS.
9. NO LANDSCAPE FEATURES OR VEGETATION WITHIN 5' OF METER BOXES.
10. CONTRACTOR SHALL VERIFY FACTORY SETTING ON PRESSURE REGULATOR.

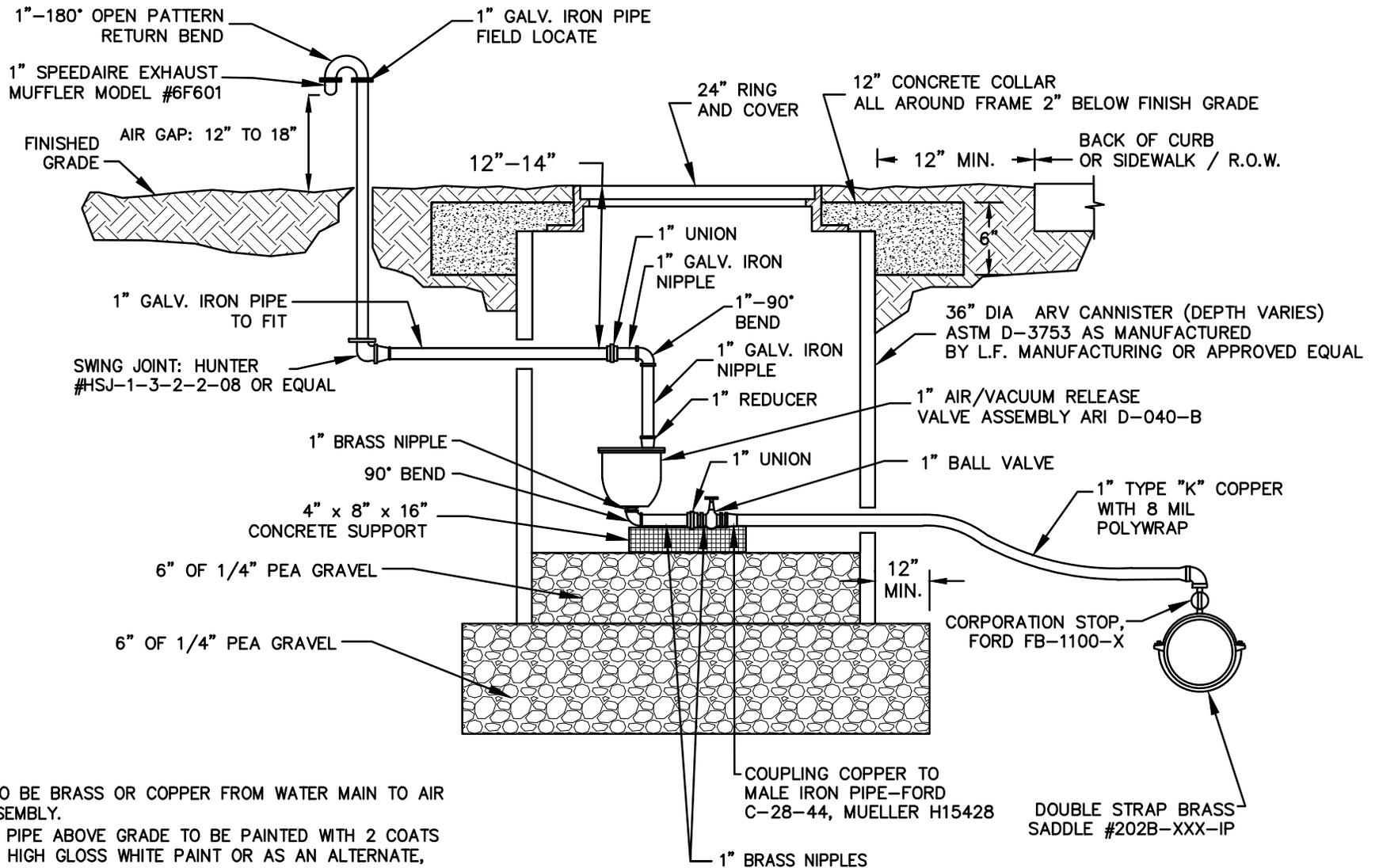
| METER SIZE | SETTER FLANGE TO FLANGE LENGTH |
|------------|--------------------------------|
| 5/8" | 7.75" |
| 1" | 10.75" |
| 1.5" | 12.75" |
| 2" | 17.25" |

COP STANDARD DETAIL

1" - 2" WATER SERVICE CONNECTIONS

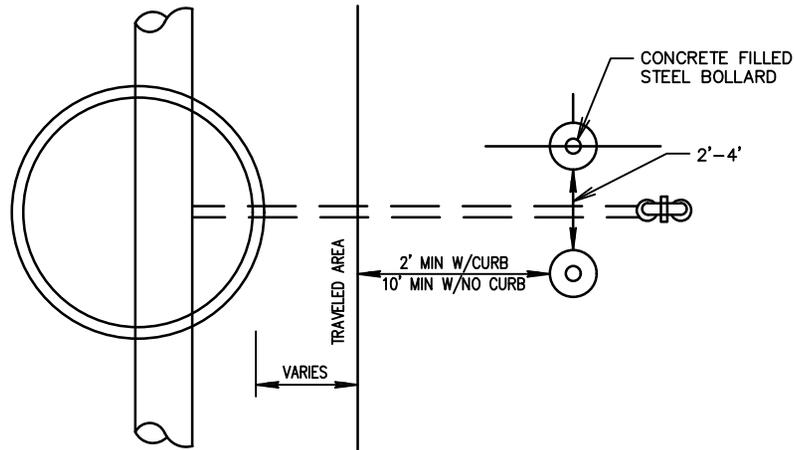
Charles Andrews
CITY ENGINEER

REVISED: 07/16
DETAIL No. 316P

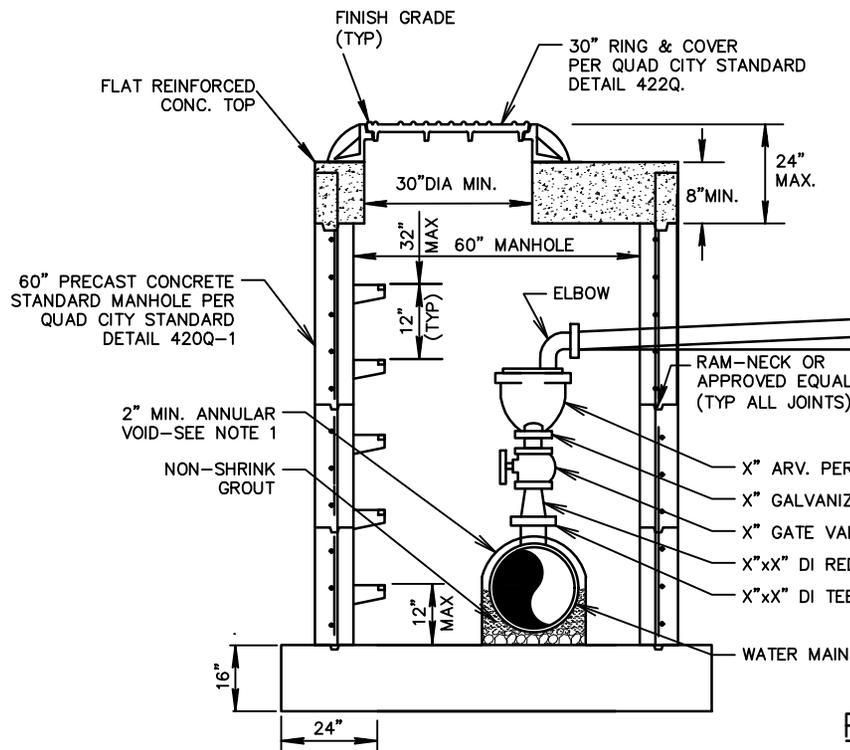


NOTES:

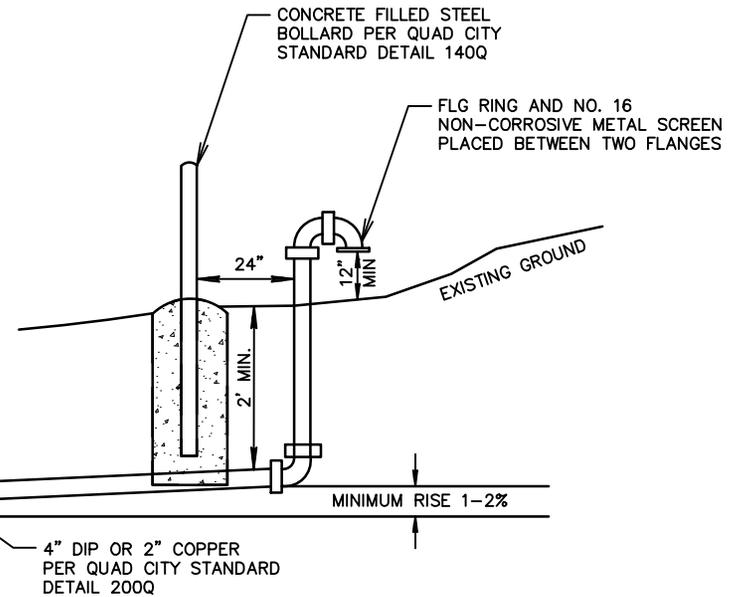
1. ALL FITTINGS TO BE BRASS OR COPPER FROM WATER MAIN TO AIR & VACUUM ASSEMBLY.
2. 1" GALVANIZED PIPE ABOVE GRADE TO BE PAINTED WITH 2 COATS OF RUSTOLEUM HIGH GLOSS WHITE PAINT OR AS AN ALTERNATE, BRASS PIPE WITHOUT PAINT.
3. AIR & VACUUM RELEASE VALVE ASSEMBLY MUST BE INSTALLED AT HIGHEST POINT OF LINE. IF HIGH POINT FALLS IN A LOCATION WHERE ASSEMBLY CANNOT BE INSTALLED, PROVIDE ADDITIONAL DEPTH OF LINE TO CREATE HIGHPOINT AT A LOCATION WHERE ASSEMBLY CAN BE INSTALLED.
4. LOCATE AIR & VACUUM METER BOX OUTSIDE OF TRAFFIC AREAS, BEHIND CURB. IN SIDEWALKS AND AREAS WITH VEHICULAR TRAFFIC, USE OLYMPIC FOUNDRY SM-30 CAST IRON BOX & COVER.



PLAN
NTS



PROFILE
NTS



NOTE 1: CONTRACTOR SHALL FILL ANNULAR VOID THROUGH VAULT WALL WITH BACKER ROD AND ELASTOMERIC SEALANT PER DETAIL.

24" MANHOLE RING AND COVER PER COP DETAIL 423P-1 WITH "PRESCOTT WATER" LETTERING AND (3) 6" MANHOLE RISER RINGS. ADJUST RING TO GRADE PER COP DETAIL 422P

FINISHED GRADE

DIAMOND PLATE STEEL METER BOX COVER PER MAG DETAIL 320

ELEVATION SHALL BE 2" MIN TO 4" MAX ABOVE FINISHED GRADE

CONCRETE WATER METER BOX NO. 2 PER MAG DETAIL 320

MIN. 14"
MAX. 18"

2" ABOVE GRADE WITH PVC CAP (TYP)

CAP

2" ADAPTER FORD C84-77G

6" SDR 35 PVC BOTTOM TO REST ON BLOCKING

6" GRAVEL BED

2" BRONZE CURB STOP FORD B11-777 (FIP X FIP) OR APPROVED EQUAL

2" TYPE K COPPER PIPE

TAPPED PLUG OR CAP

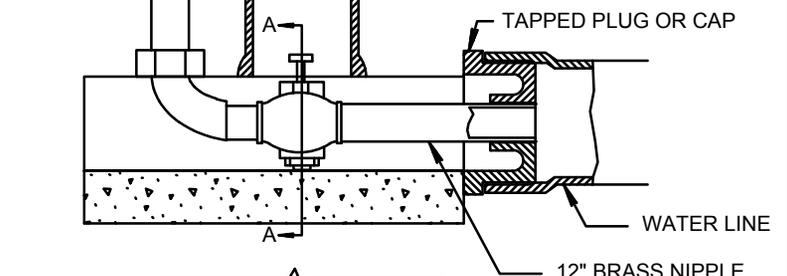
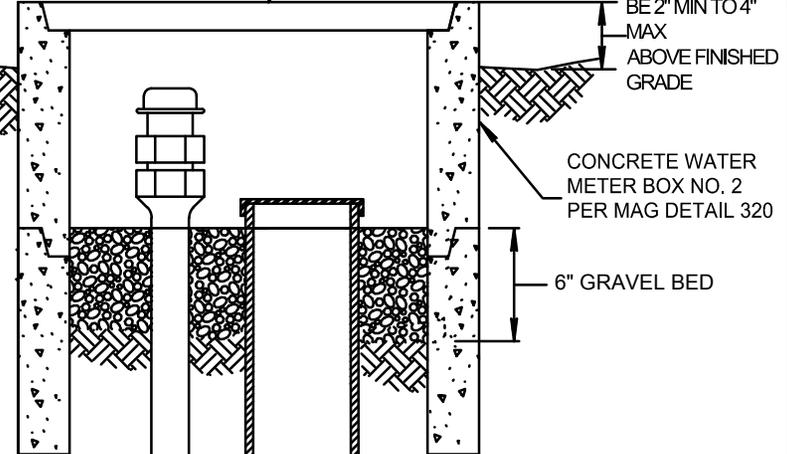
ELL COUPLING FORD L14-77G

4" BRASS NIPPLE

WATER LINE

12" BRASS NIPPLE

SOLID CONCRETE BLOCK



SECTION A-A

TRAFFIC AREAS

NON-TRAFFIC AREAS

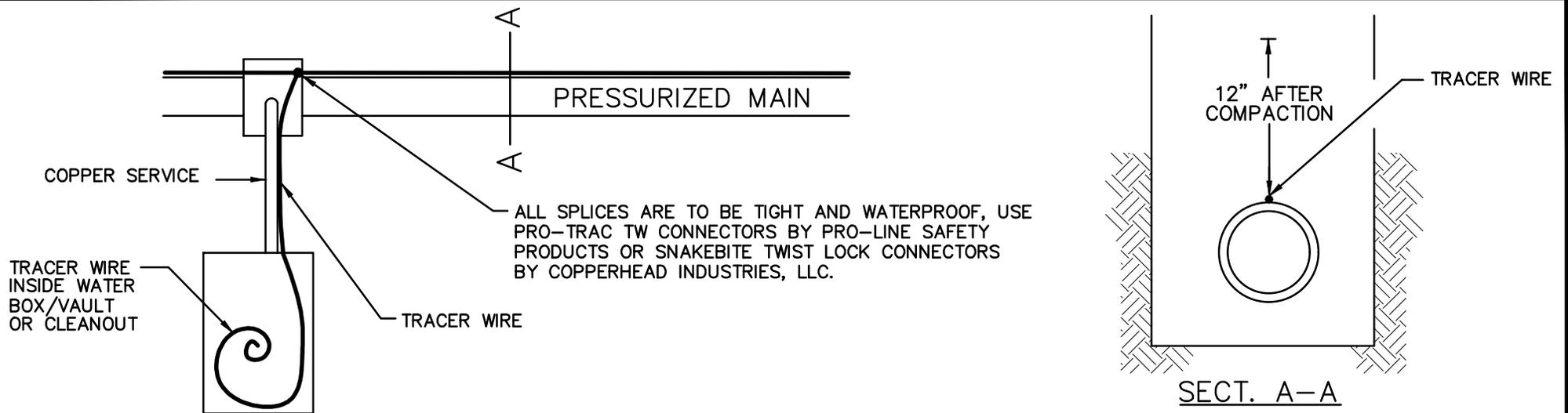
COP STANDARD DETAIL

BLOW OFF

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
318P

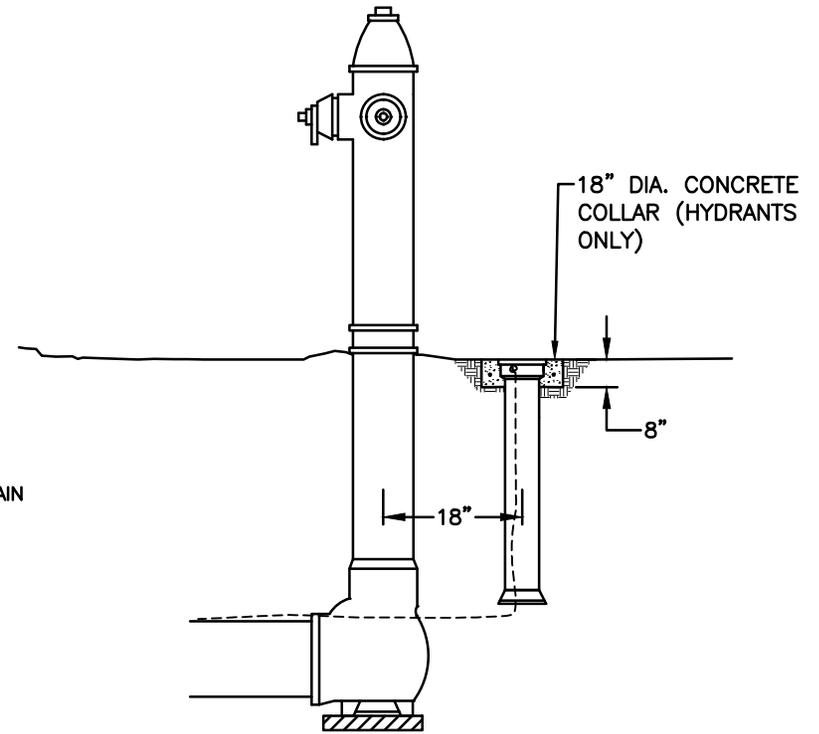
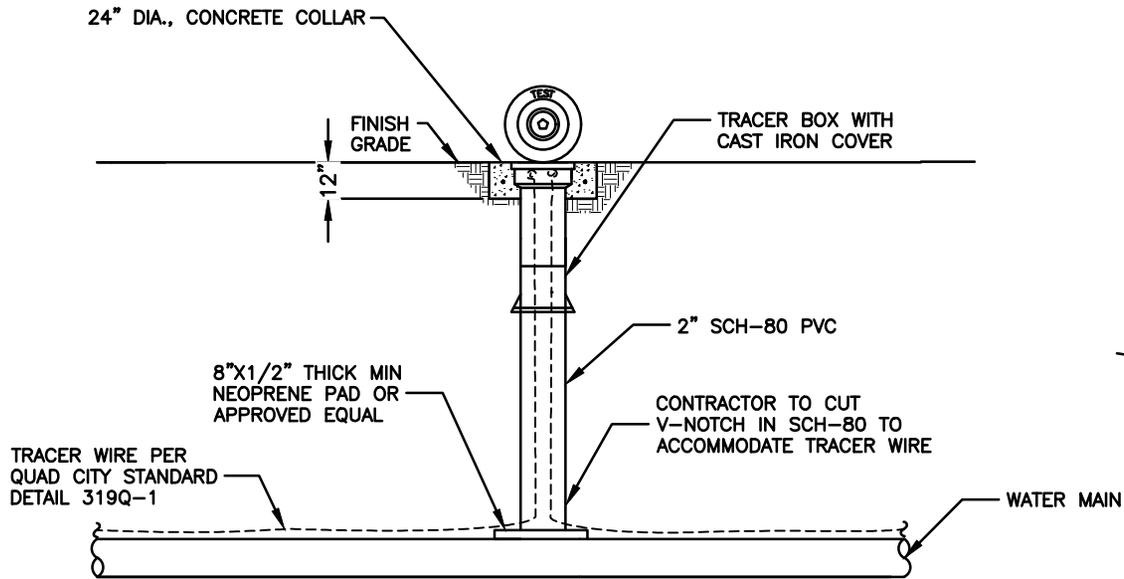


NOTES:

1. TRACER WIRE SHALL BE REQUIRED FOR ALL PRESSURE PIPE INSTALLATION AS INDICATED ON OTHER STANDARD DETAILS. TRACER WIRE SHALL BE #14 AWG HS-CCS, 30 MIL HDPE INSULATION.
2. FINAL CONTINUITY TESTING FOR ACCEPTANCE OF TRACER WIRE SHALL BE MADE AT PROJECT COMPLETION.
3. WIRE IS TO BE ATTACHED TO ALL APPURTENANCES.
4. TRACER WIRE IS TO BE PLACED AT TOP CENTER OF PIPE AND SECURED BY TAPE AT A MINIMUM OF 6' INTERVALS.
5. THIS DETAIL DOES NOT PERTAIN TO STRAIGHT RUN GRAVITY SANITARY SEWER MAINS IN THE CITY OF PRESCOTT.
6. TRACER WIRE STATION SHALL BE USED AT ALL FIRE HYDRANTS, SEWER WET WELLS, FORCE MAIN DISCHARGE MANHOLES, FORCE MAIN CLEANOUTS, FORCE MAIN VALVE LOCATIONS AND LOW PRESSURE SEWER MAIN APPURTENANCES IN ACCORDANCE WITH QUAD CITY STANDARD DETAIL 319Q-2.
7. PASS A CONTINUITY TEST, CONDUCTED BY THE CONTRACTOR AND WITNESSED BY THE ENGINEER OF RECORD (EOR) AND THE AGENCY REPRESENTATIVE. THE EOR SHALL PROVIDE A CERTIFICATION OF PASSING TO THE CONTRACTOR AND AGENCY.
8. TO PASS A CONTINUITY TEST, THE FOLLOWING CONDITIONS MUST BE MET:
 1. CONTINUITY TEST SHALL BE PERFORMED BY USING A METALLIC LOCATOR WITH AUDIBLE TONE AND NUMERIC VALUES FOR CERTIFICATION OF FACILITY LOCATIONS AND SHALL BE IDENTIFIABLE BETWEEN ACCESS POINTS.
 2. THE WIRE SHALL BE ACCESSIBLE AT ALL ACCESS POINTS AND FROM ACCESS POINT TO ACCESS POINT.
 3. DEPTH READINGS MUST BE ACCURATE AND CONSISTENT TO WITHIN 15 (DEPTH TO DIAMETER RATIO).
 4. ACCESS POINTS THAT ARE WIDELY SPACED CAN BE TRACED IN WORST CASE FROM EACH END TO A COMMON METTING POINT BETWEEN THEM.
9. CONTRACTOR MUST PROVIDE EOR AND TOWN WITH THE FOLLOWING:
 1. SAMPLE OF WIRE.
 2. SAMPLE OF CONNECTOR.
 3. INVOICE COPY SHOWING PURCHASE OF WIRE AND CONNECTOR.

DETECTABLE TAPE SHALL BE:

1. PRO-LINE SAFETY PRODUCTS OR HYTECH DETECTABLE TAPE (UNDERGROUND WARNING TAPE). NO EXCEPTIONS.
2. 3 INCHES IN WIDTH
3. 5 MIL OVERALL THICKNESS WITH A .35 MIL SOLID ALUMINUM FOIL CORE.
4. CONSTRUCTED WITH A .8 MIL CLEAR FILM, REVERSE PRINTED WITH A REPEATING WARNING MESSAGE AND LAMINATE TO ALUMINUM FOIL WITH A 3.75 MIL CLEAR FILM BACKING.
5. LETTERED WITH A MINIMUM OF 1" INCH.
6. COLOR CODED FOR SPECIFIC UTILITY. WATER - BLUE, SANITARY SEWER - GREEN, RECLAIMED WATER - PURPLE IN ACCORDANCE WITH APWA UNIFORM COLOR CODE.
7. CONTRACTOR MUST PROVIDE EOR AND AGENCY WITH THE FOLLOWING:
 1. SAMPLE OF TAPE.
 2. INVOICE COPY SHOWING PURCHASE OF WIRE AND CONNECTOR.

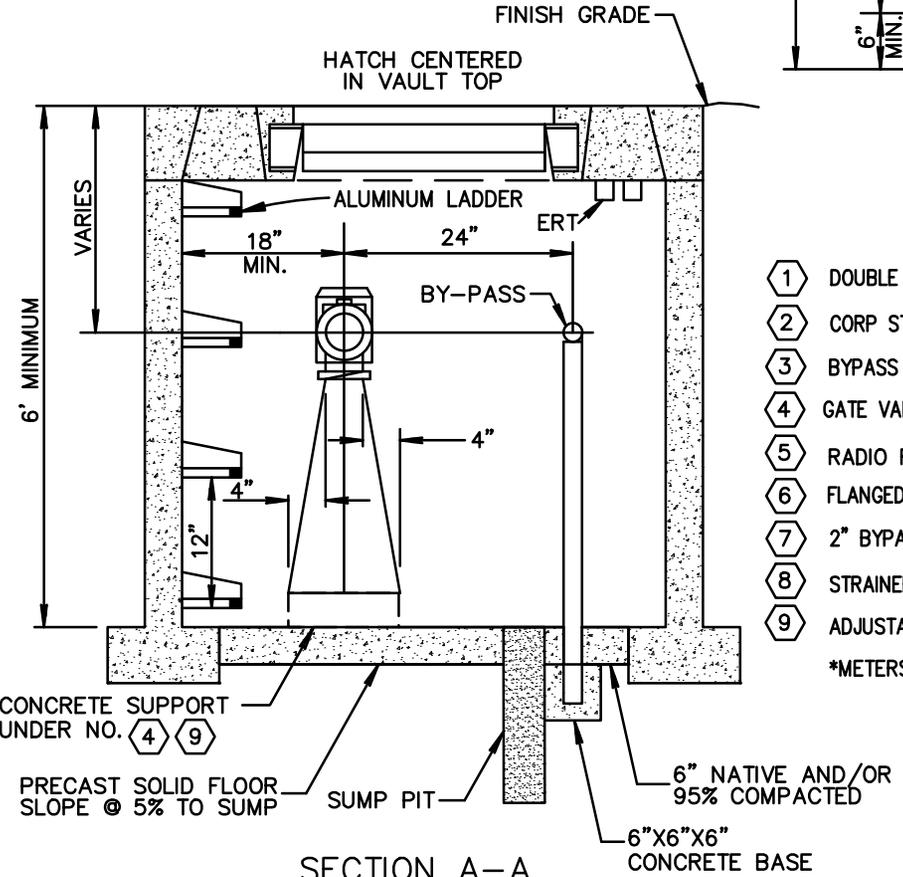
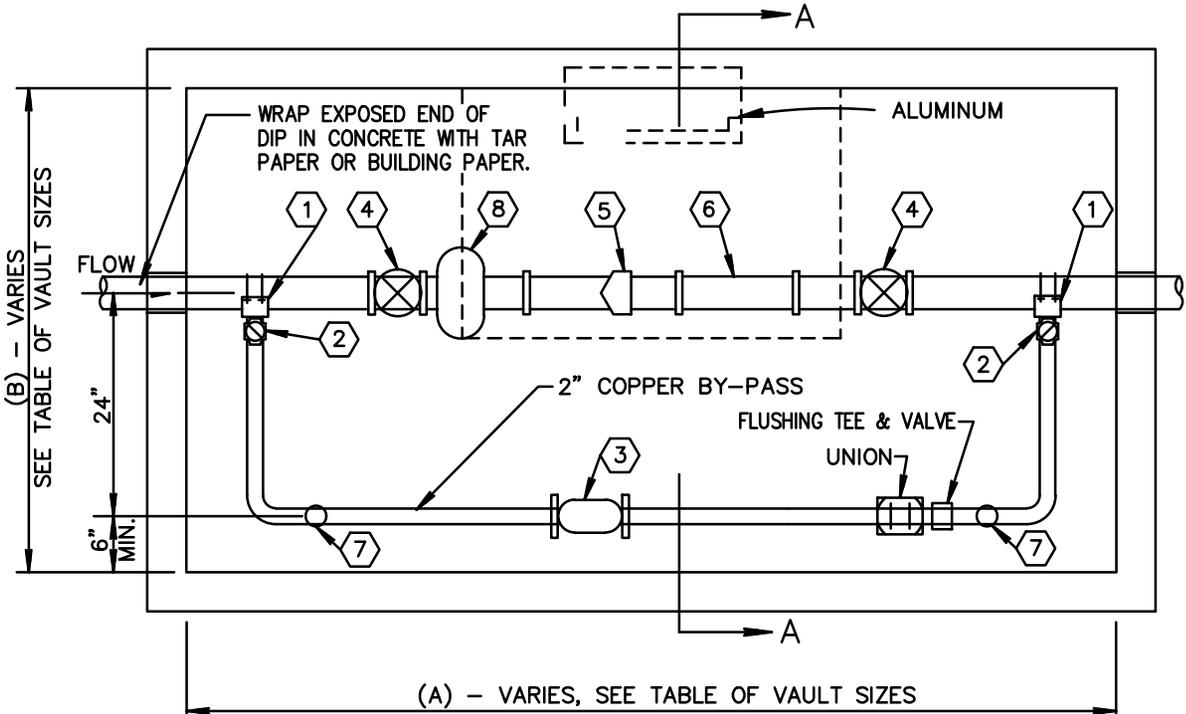


FIRE HYDRANT TRACER STATION

NOTES:

1. TRACER BOX AND CAST IRON COVER SHALL BE COPPERHEAD INDUSTRIES, LLC SNAKEPIT OR APPROVED EQUAL, MODEL RB14*TP OR CD14*TP AS APPROPRIATE, LID COLOR SHALL COINCIDE WITH TYPE OF UTILITY.
2. MAXIMUM DISTANCE BETWEEN TRACER WIRE POINT OF CONTACT SHALL BE 500'.
3. ATTACH TRACER WIRE TO PIPE AT 6' INTERVALS.

| VAULT DIMENSION DETAILS (MINIMUM) | | | | |
|--------------------------------------|-------|--------|-----|-----|
| METER SIZE | 3" | 4" | 6" | 8" |
| (A) | 8'-4" | 10'-6" | 12' | 15' |
| (B) | 4'-4" | 5' | 5' | 6' |
| HATCH | | | | |
| (A) | 6' | 6' | 6' | 6' |
| (B) | 3' | 3' | 4' | 4' |

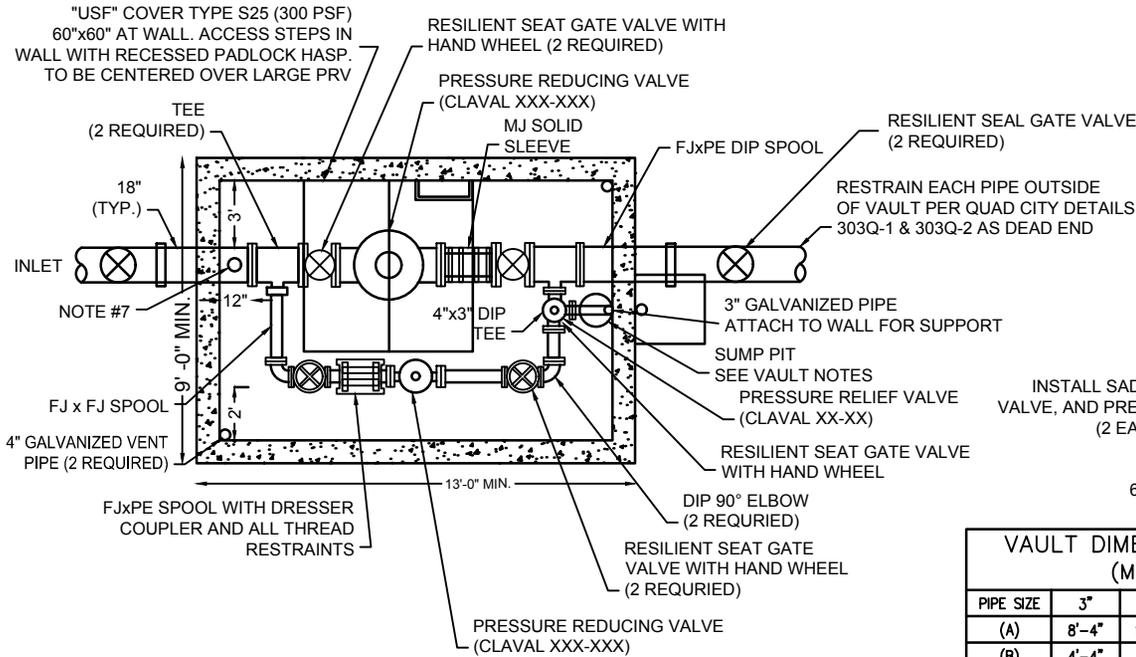


- ① DOUBLE STRAP ALL BRONZE SADDLE
- ② CORP STOP 2" BALL TYPE
- ③ BYPASS METER*
- ④ GATE VALVE R/S FLANGED WITH HAND WHEEL
- ⑤ RADIO READ* METER PER AGENCY
- ⑥ FLANGED SPOOL (MIN. 3 PIPE DIAMETERS IN LENGTH)
- ⑦ 2" BYPASS SUPPORT
- ⑧ STRAINER
- ⑨ ADJUSTABLE PIPE SUPPORT

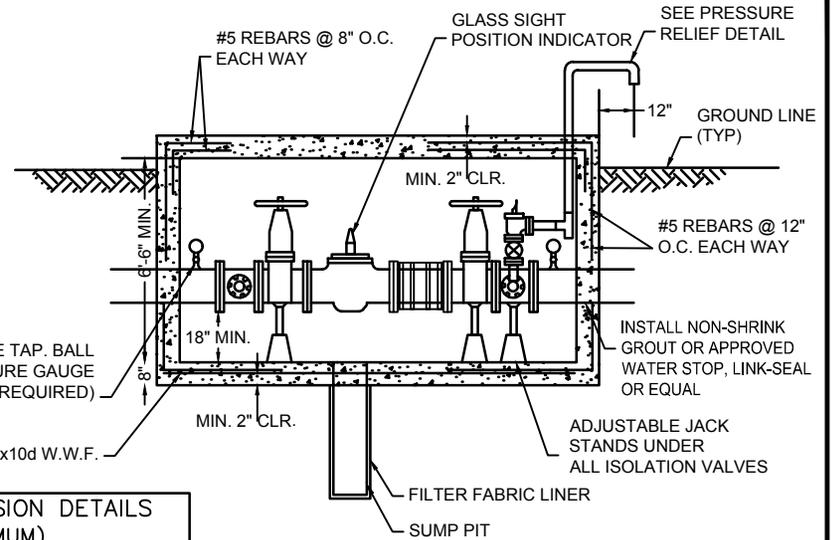
*METERS TO BE SUPPLIED BY AGENCY

- NOTES:
1. BOTH METERS SHALL BE RADIO READ WITH ERT MOUNTED WITHIN 6" OF FINISH GRADE.
 2. METER VAULTS TO BE PRE-CAST CONCRETE.
 3. JOINT RESTRAINT TO AND THROUGH REQUIRED.
 4. SUMP PIT TO BE 8 CUBIC FEET OF CLEAN #57 ROCK AND LINED WITH FABRIC FILTER LINER MIRAFI TYPE 14ONL OR APPROVED EQUAL.
 5. ACCESS HATCH - USF FABRICATION INC. ALUMINUM SPRING OR TORSION BAR ASSISTED DOUBLE LIDS WITH 90 TO 180 DEGREE OPEN RANGE WITH RECESSED PADLOCK/HASP ASSEMBLY AND 30 PSF RATING.
 6. PIPELINE PRODUCTS VL-100 EXTENDABLE LADDER. TELESCOPING LADDER MUST BE INSTALLED ON THE VERTICAL WALL JUST BELOW THE SAFETY NET. POSITION THE LADDER TO CENTER OF ACCESS HATCH OR EQUAL.
 7. SAFETY NET - U.S. WEBBING SLING STD. EZ BARRIER OR APPROVED EQUAL.

PLAN VIEW



PROFILE VIEW



VAULT DIMENSION DETAILS (MINIMUM)

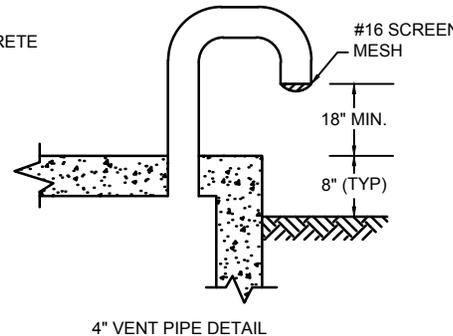
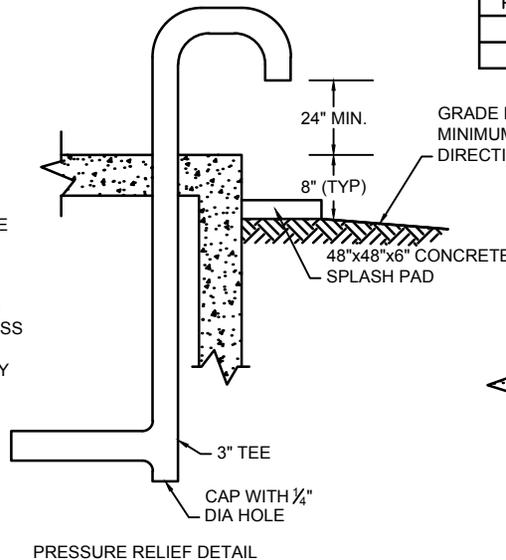
| PIPE SIZE | 3" | 4" | 6" | 8" |
|-----------|-------|--------|-----|-----|
| (A) | 8'-4" | 10'-6" | 12' | 15' |
| (B) | 4'-4" | 5' | 6' | 6' |
| HATCH | | | | |
| (A) | 6' | 6' | 6' | 6' |
| (B) | 4' | 4' | 5' | 5' |

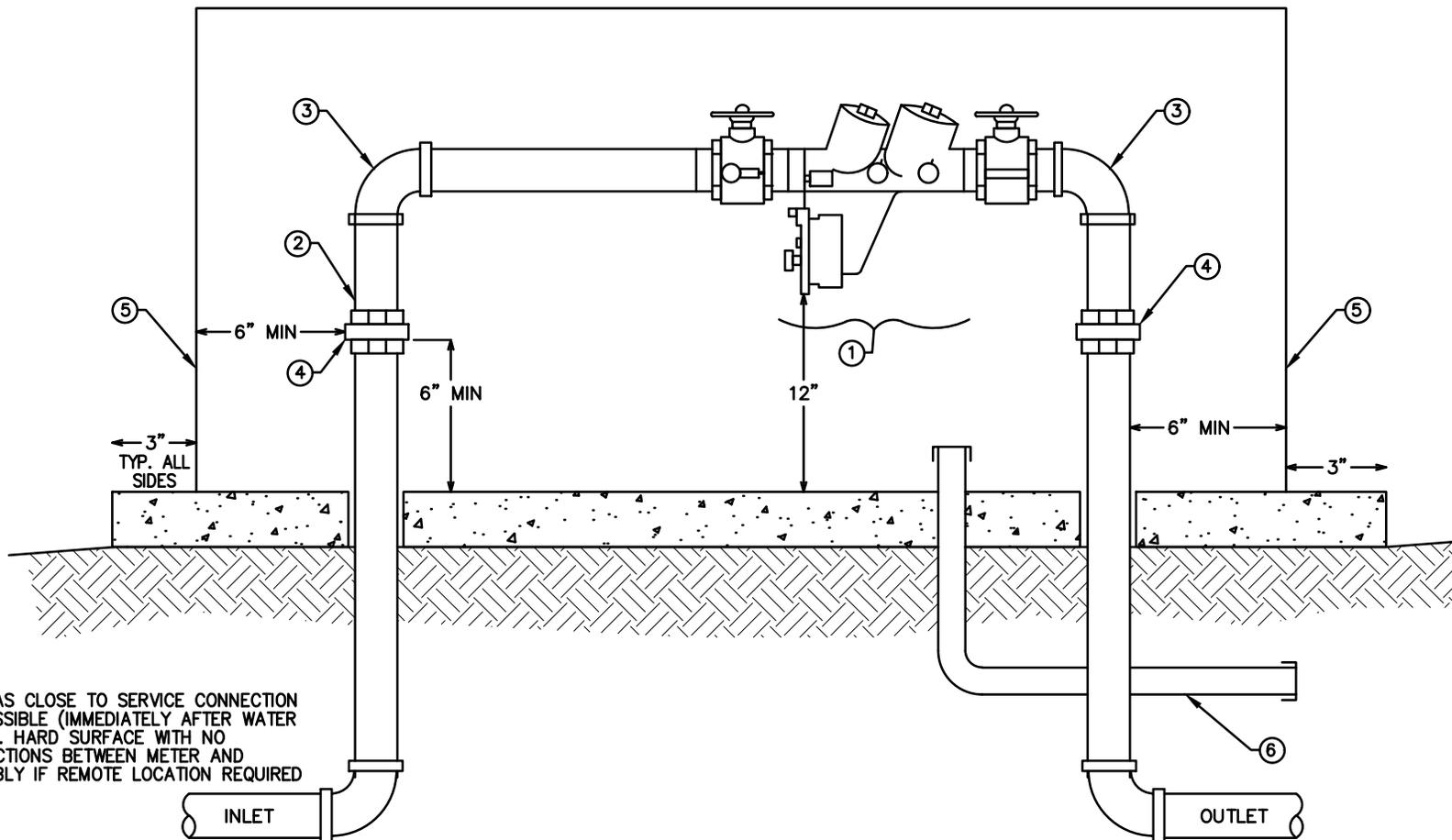
VAULT NOTES:

- ALL PRECAST OR POUR IN PLACE CONCRETE TO BE CLASS "A" PER M.A.G. STD. SPEC. 725. ALTERNATE DESIGN TO BE 8" C.M.U. WALLS, GROUT ALL CELLS SOLID, USE 2 #4 REBAR IN BOND BEAM AT TOP OF WALLS AND AT 30" ABOVE FOOTING. #4 REBAR AT 16" O.C. VERTICAL.
- SUMP PIT TO BE 8 CUBIC FEET OF CLEAN #57 ROCK AND LINED WITH FABRIC FILTER LINER MIRAFI TYPE 14ONL OR APPROVED EQUAL.
- ALL FITTINGS AND VALVES TO BE FLANGE JOINT DUCTILE IRON PIPE.
- SLOPE FLOOR AT 5% TO SUMP.
- ACCESS HATCH - USF FABRICATION INC. ALUMINUM SPRING OR TORSION BAR ASSISTED DOUBLE LIDS WITH 90 TO 180 DEGREE OPEN RANGE WITH RECESSED PADLOCK/HASP ASSEMBLY AND 30 PSF RATING.
- PIPELINE PRODUCTS VL-100 EXTENDABLE LADDER. TELESCOPING LADDER MUST BE INSTALLED ON THE VERTICAL WALL JUST BELOW THE SAFETY NET. POSITION THE LADDER TO CENTER OF ACCESS HATCH OR EQUAL.
- 2" SADDLE WITH 2" BALL VALVE & 2" PLUG. THIS IS USED FOR FLOW MODELING & INSERTION PROBES.
- SAFETY NET - U.S. WEBBING SLING STD. EZ BARRIER OR APPROVED EQUAL.

VALVE NOTES:

- PRESSURE REDUCING CLAVAL SHALL INCLUDE THE FOLLOWING FEATURES:
 - X46A FLOW CLEAN STRAINER
 - CK2 COOK (ISOLATION VALVE)
 - CV SPEED CONTROL (OPENING & CLOSING)
 - ALL PRV PILOT SYSTEMS MUST BE STAINLESS STEEL HIGH PRESSURE TUBING.
- FINAL PRESSURE SETTINGS TO BE DETERMINED BY AGENCY





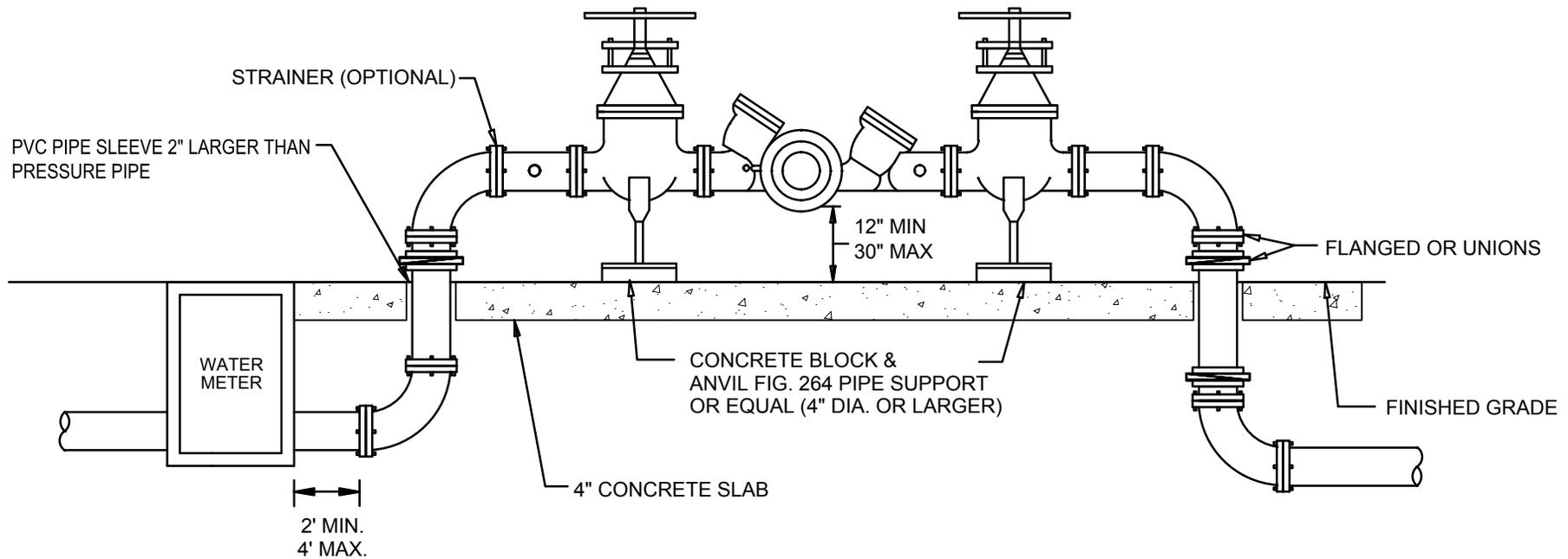
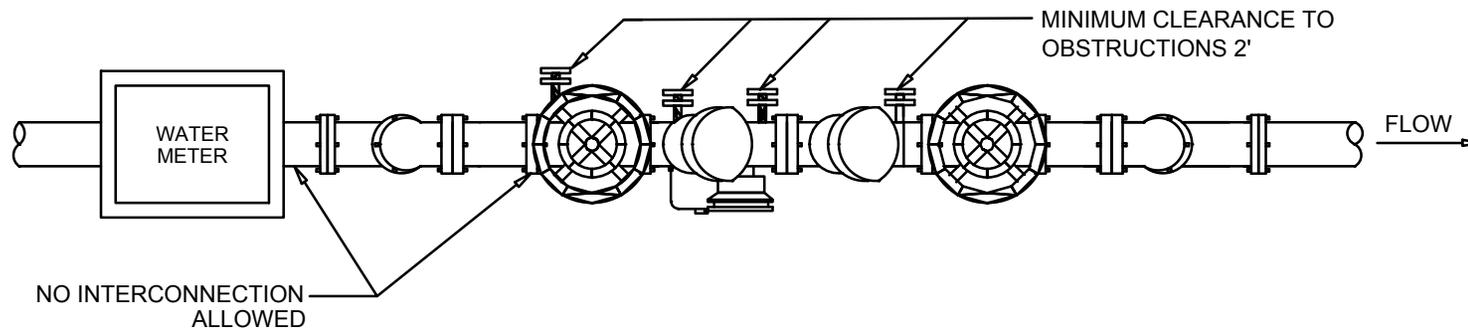
INLET AS CLOSE TO SERVICE CONNECTION AS POSSIBLE (IMMEDIATELY AFTER WATER METER). HARD SURFACE WITH NO CONNECTIONS BETWEEN METER AND ASSEMBLY IF REMOTE LOCATION REQUIRED

NOTES:

1. ALL PIPE/FITTINGS TO BE TYPE "K" COPPER.
2. BACKFLOW PREVENTION ASSEMBLY MUST BE LEVEL AND INSTALLED A MINIMUM OF 16" FROM ASSEMBLY BODY TO FINAL GRADE.
3. ALL TEST COCKS (4 REQUIRED) SHALL BE FITTED WITH BRASS PLUGS INSTALLED WITH TEFLON TAPE.
4. COMPRESSION TYPE FITTINGS ARE NOT ALLOWED.
5. INSTALL THE BACKFLOW PREVENTION ASSEMBLY IMMEDIATELY DOWNSTREAM OF THE AGENCY WATER METER.
6. CALL FOR UNDERGROUND INSPECTION BEFORE BACKFILLING TRENCH.
7. A COPPER/BRASS UNION MUST BE INSTALLED IN THE MIDDLE OF BOTH RISERS.
8. ASSEMBLY SHALL BE APPROVED BY USC FOUNDATION FOR CROSS-CONNECTION CONTROL AND HYDRAULIC RESEARCH.

LIST OF MATERIALS:

- ① REDUCED PRESSURE BACKFLOW PREVENTION ASSEMBLY, BALL VALVES INCLUDED.
- ② DOUBLE CHECK VALVE BACKFLOW PREVENTION ASSEMBLY USE SHALL BE REVIEWED AND APPROVED BY AGENCY.
- ③ PIPE, TYPE "K" HARD COPPER, 3/4" THRU 3".
- ④ 90° ELL, COPPER, 3/4" THRU 3"
- ⑤ PIPE UNION, BRASS OR COPPER.
- ⑥ INSTALL 4" CONCRETE PAD, ENCLOSURE, AND HARDWARE. ENCLOSURE SHALL BE AN ASSE 1060 CLASS 1 APPROVED.
- ⑦ ELECTRICAL CONDUIT FOR HEAT TRACE.



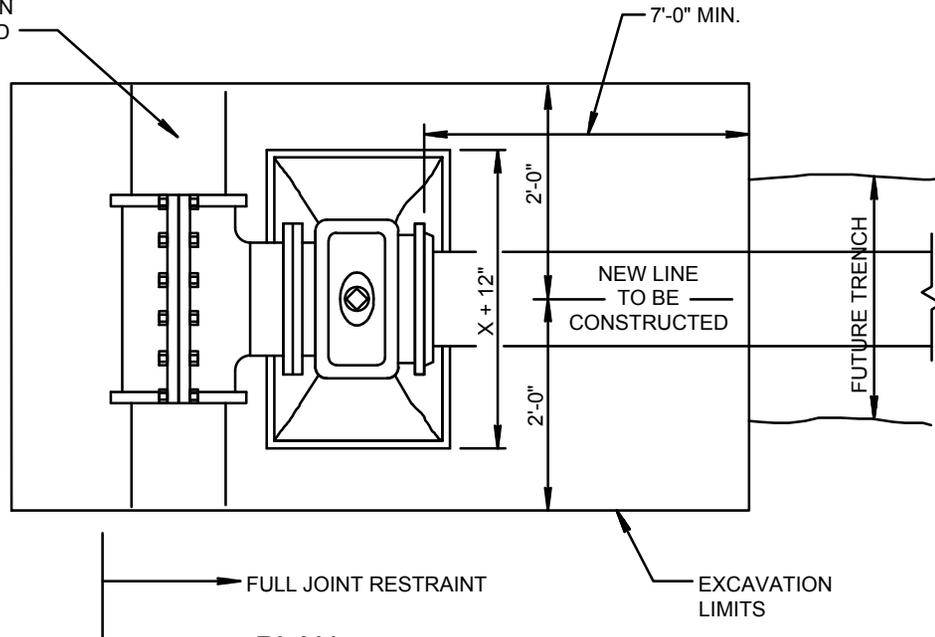
NOTES:

1. NO OBSTRUCTIONS ON ONE SIDE.
2. BACKFLOW PROTECTION REQUIRED PER CITY/TOWN CODE.
3. ENCLOSURE REQUIRED (ASSE 1060 CLASS 1).
4. ALL CONNECTIONS SHALL BE FLANGED OR MECHANICALLY RESTRAINED JOINTS.
5. THERE SHALL NOT BE ANY CONNECTIONS ON THE SERVICE LINE BETWEEN THE RP AND THE WATER METER. ENCLOSURES INSTALLED MUST MEET CLEARANCE REQUIREMENTS IN ADDITION TO PROVIDING SIDE AND TOP ACCESS. ENCLOSURES MUST NOT RETAIN WATER.
6. THE ASSEMBLY SHALL BE ACCESSIBLE AT ALL TIMES.

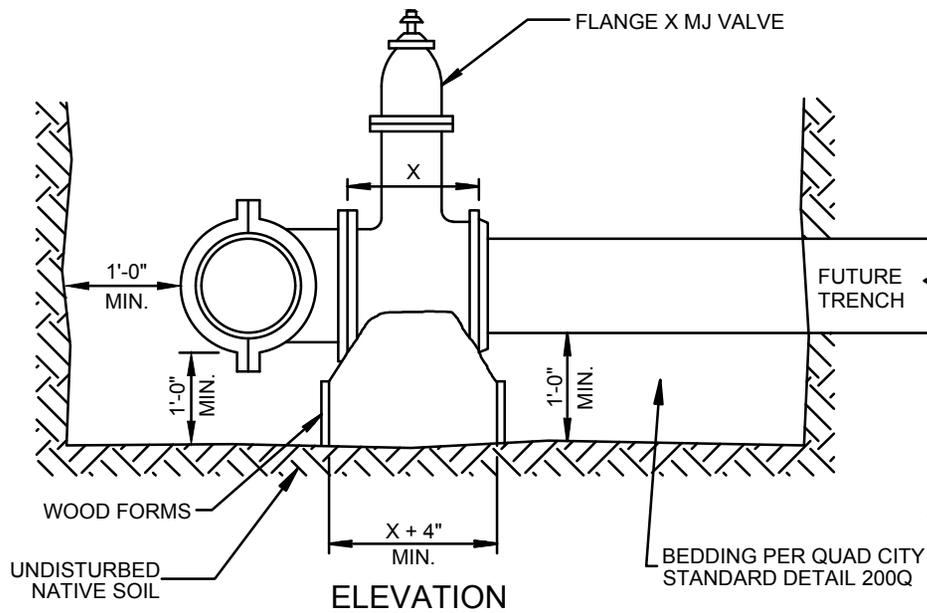
7. DISTANCE FROM THE BOTTOM OF PRESSURE RELIEF VALVE TO THE FINISHED GRADE SHALL BE A MINIMUM OF TWICE THE DIAMETER OF THE ASSEMBLY PIPING. MINIMUM HEIGHT 12".
8. INSTALLATION MUST MEET INTERNATIONAL PLUMBING CODES IN ADDITION TO STANDARD WATER DETAILS. INSTALLATION MUST BE LEFT EXPOSED UNTIL INSPECTED AND APPROVED BY THE CITY/TOWN.
9. RP SHALL BE LOCATED ABOVE GROUND AND ENCLOSED IN AN ASSE 1060 CLASS 1 APPROVED ENCLOSURE PER INTERNATIONAL BUILDING CODES.
10. RP SHALL BE LOCATED WITHIN 4' OF METER, UNLESS OTHERWISE APPROVED.

11. DOUBLE CHECK VALVE BACKFLOW PREVENTION ASSEMBLY USE SHALL BE REVIEWED AND APPROVED BY AGENCY.
12. ASSEMBLY SHALL BE APPROVED BY USC FOUNDATION FOR CROSS-CONNECTION CONTROL AND HYDRAULIC RESEARCH.

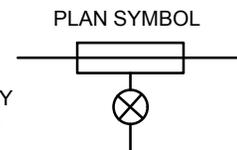
EXISTING MAIN
TO BE TAPPED



PLAN

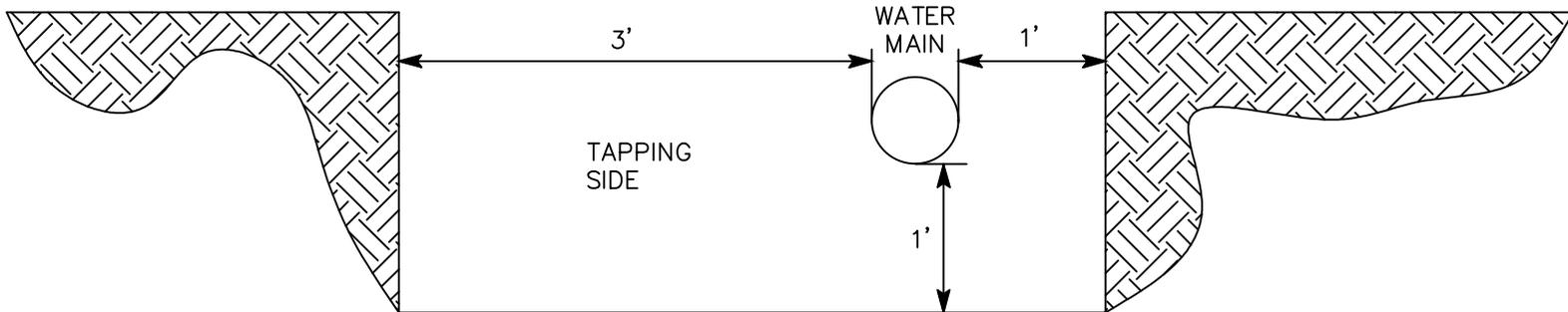


ELEVATION



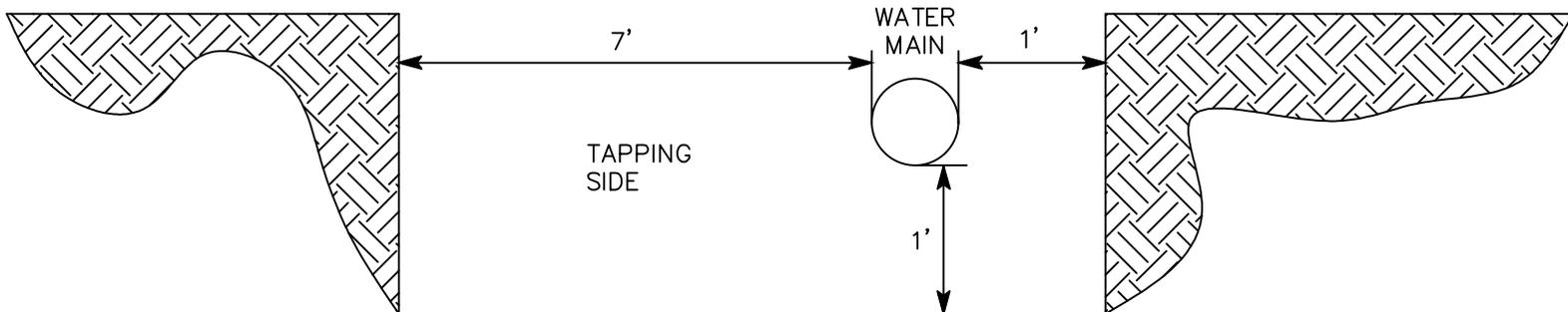
NOTES:

1. VALVE BLOCKING PER QUAD CITY STANDARD DETAIL 301Q.
2. TAPS SHALL BE MADE BY CITY/TOWN CREWS AT PREVAILING RATES OR BY APPROVED CONTRACTORS WHEN ALLOWED BY CITY/TOWN.
3. INSTALL TEMPORARY BLOCKING UNDER VALVE BEFORE TAP IS MADE. ALL FLANGE BOLTS SHALL BE CLEAR OF FOOTING.
4. TAPPING SLEEVES SHALL BE FORD FTSS, ROMAC SST III OR APPROVED EQUAL.
5. INSTALLATION SHALL BE LEAK TESTED TO A MINIMUM OF 200 PSI FOR 30 MINUTES PRIOR TO TAP.
6. TAPPING SLEEVE SHALL BE PLACED AT A MINIMUM OF 3' FROM ANY BELL, COUPLING, VALVE, FITTING, OR OTHER OBSTRUCTION.
7. PROTECT ALL CONCRETE CONTACT AREAS WITH 8 MIL SHEET PLASTIC.
8. JOINT RESTRAINT PER QUAD CITY STANDARD DETAIL 303Q USING A DEAD END ON NEW CONSTRUCTION.
9. INSTALL BOX, COVER, AND VALVE BOX STABILIZER PER AGENCY REQUIREMENTS.
10. ALL EXCAVATIONS SHALL BE OSHA COMPLIANT.
11. ALL PIPE, VALVES, FITTINGS, AND APPURTENANCES SHALL BE MANUFACTURED IN THE U.S.A.
12. MIN. EXCAVATION LENGTH FOR THE TAPPING OPERATION SHALL BE 5' FROM THE FACE OF VALVE



NOTE
 CONTRACTOR TO EXPOSE AND MAINTAIN TRENCH/EXCAVATION
 PER OSHA REGULATIONS.

3/4" THROUGH 2" SERVICE TAP



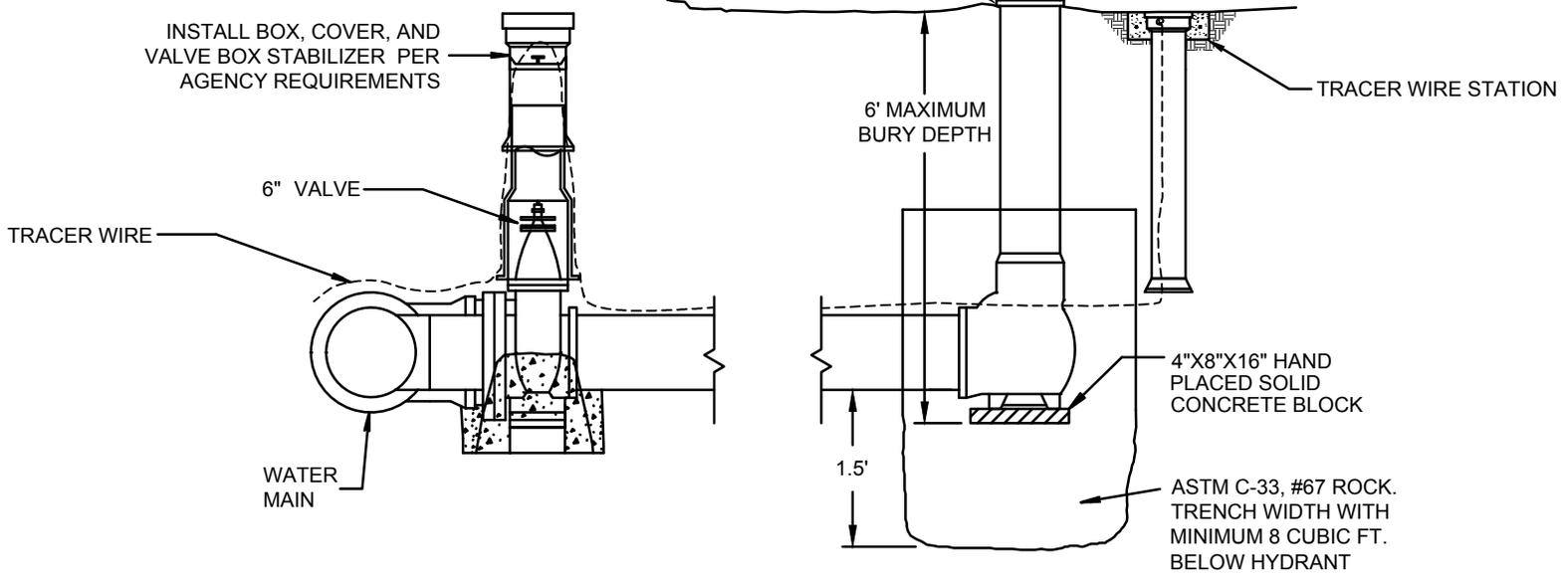
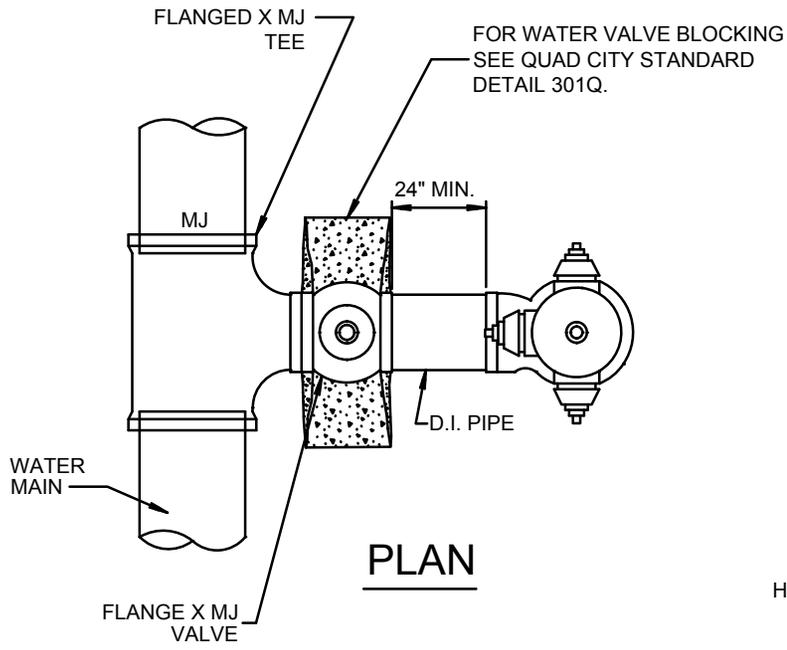
NOTE
 CONTRACTOR TO EXPOSE AND MAINTAIN TRENCH/EXCAVATION
 PER OSHA REGULATIONS.

3" THROUGH 12" SERVICE TAP

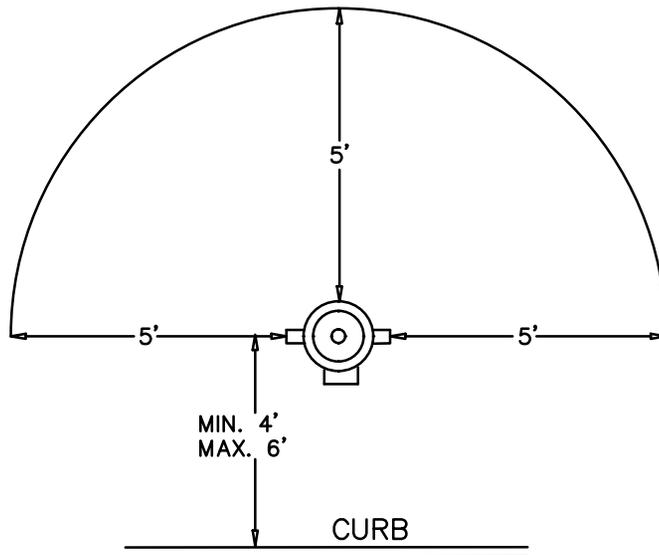
NOTE
 ALL TRENCHES FOR SERVICE TAPS
 SHALL BE MINIMUM 3' WIDE.

NOTES:

1. ALL HYDRANTS WHICH ARE PRIVATELY MAINTAINED AND OWNED ARE TO BE PAINTED RED.
2. ALL HYDRANTS TO BE WATEROUS, MUELLER OR CLOW.
3. HYDRANT LEADS SHALL HAVE NO HORIZONTAL BENDS.
4. TRACER WIRE SHALL CONFORM TO QUAD CITY STANDARD DETAIL 319Q-1
5. FULLY RESTRAIN HYDRANT LEAD AND HYDRANT. THRUST BLOCKS ARE NOT ALLOWED.
6. INSTALL TRACER WIRE STATION PER QUAD CITY STANDARD DETAIL 319Q-2.
7. SEE QUAD CITY STANDARD DETAIL 363Q FOR VERTICAL ADJUSTMENT.
8. ALL DUCTILE IRON PIPE AND FITTINGS SHALL BE MANUFACTURED IN THE U.S.A.
9. 4.5" PUMPER CONNECTION TO FACE CURB.
10. MAXIMUM OF 1 HYDRANT EXTENSION - ORIGINAL MANUFACTURER.



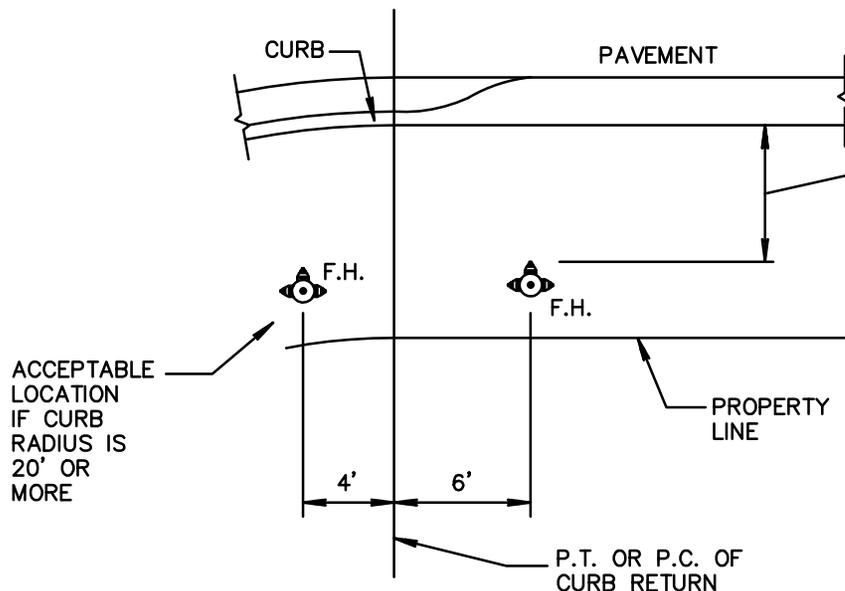
PROFILE



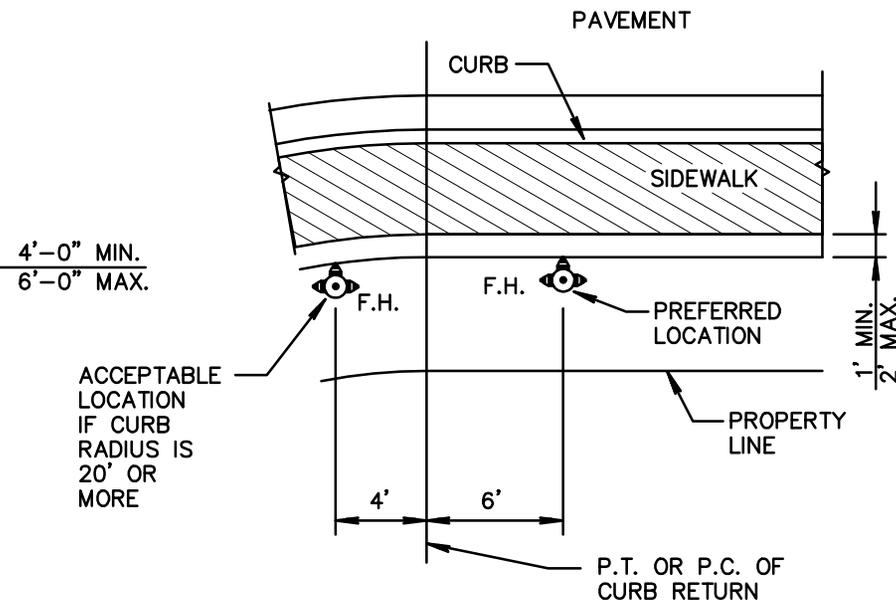
HYDRANT OBSTRUCTION CLEARANCE

NOTES:

1. OBSTRUCTIONS SUCH AS UTILITY POLES, STREET SIGNS, IRRIGATION BOXES, FENCES, LANDSCAPE VEGETATION, ETC. MUST NOT BE PLACED BETWEEN CURB AND HYDRANT
2. SOME LOCATIONS APPLY AT EITHER END OF CURB RETURNS.
3. ALL HYDRANTS WHICH ARE PRIVATELY MAINTAINED AND OWNED ARE TO BE PAINTED RED.
4. IN PARKING LOT ISLANDS, HYDRANT TO BE MIN. 3' IN ALL DIRECTIONS FROM BACK OF CURB.
5. HYDRANTS TO BE CLEAR OF LANDSCAPE & VEGETATION WITHIN A 5' RADIUS.



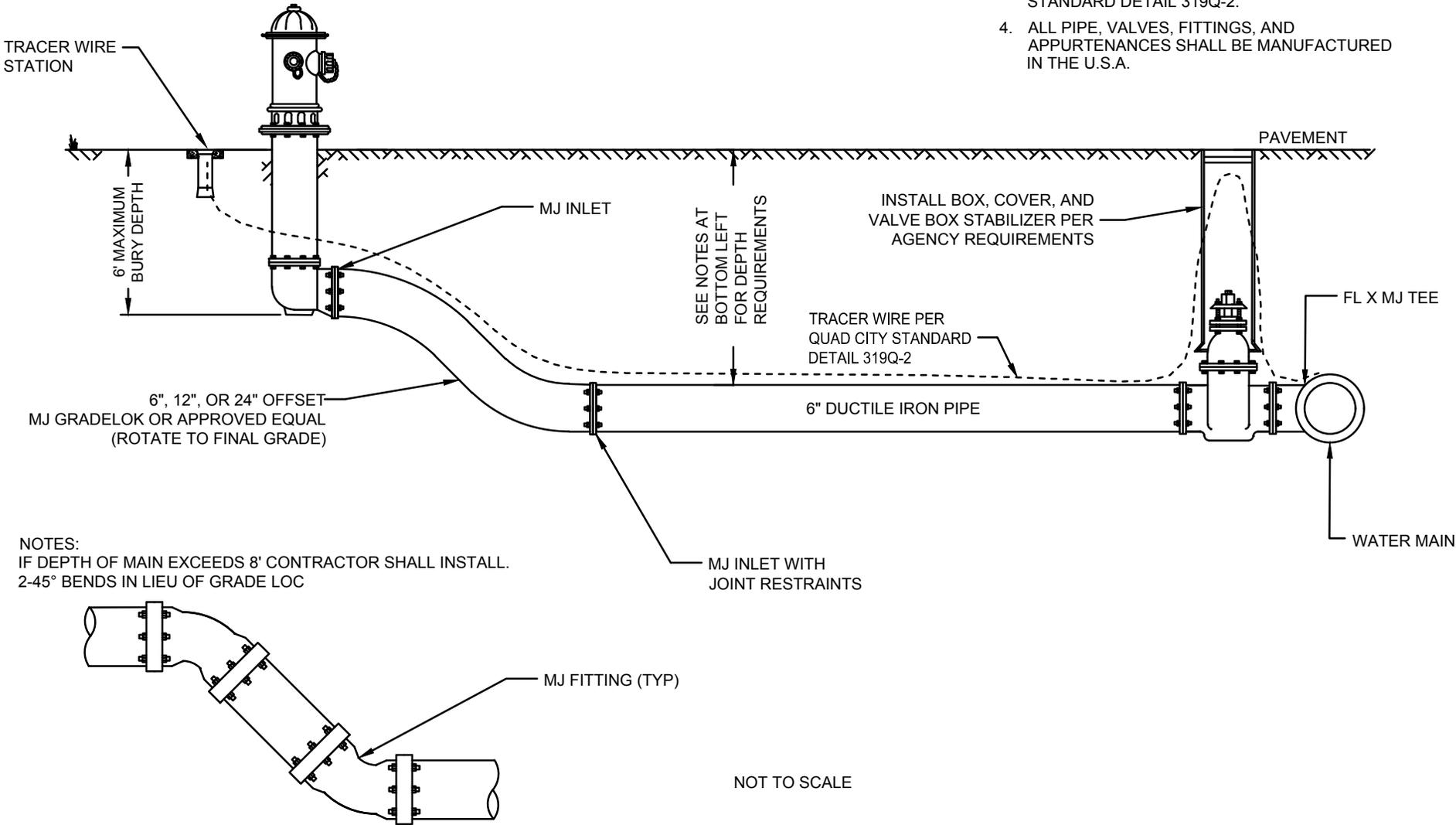
PARKWAY AREA OR NO SIDEWALK



AREA WITH SIDEWALK

NOTES:

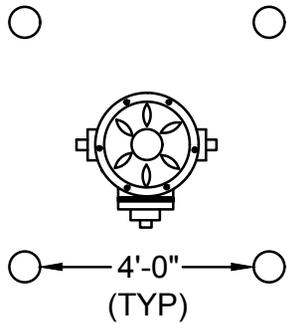
1. THIS DETAIL IS FOR VERTICAL DEFLECTION ONLY. REFER TO QUAD CITY STANDARD DETAIL 360Q FOR HYDRANT DETAILS.
2. MECHANICALLY RESTRAIN ALL JOINTS FROM HYDRANT TO HYDRANT TEE.
3. INSTALL TRACER WIRE STATION PER QUAD CITY STANDARD DETAIL 319Q-2.
4. ALL PIPE, VALVES, FITTINGS, AND APPURTENANCES SHALL BE MANUFACTURED IN THE U.S.A.



NOTES:
IF DEPTH OF MAIN EXCEEDS 8' CONTRACTOR SHALL INSTALL 2-45° BENDS IN LIEU OF GRADE LOC

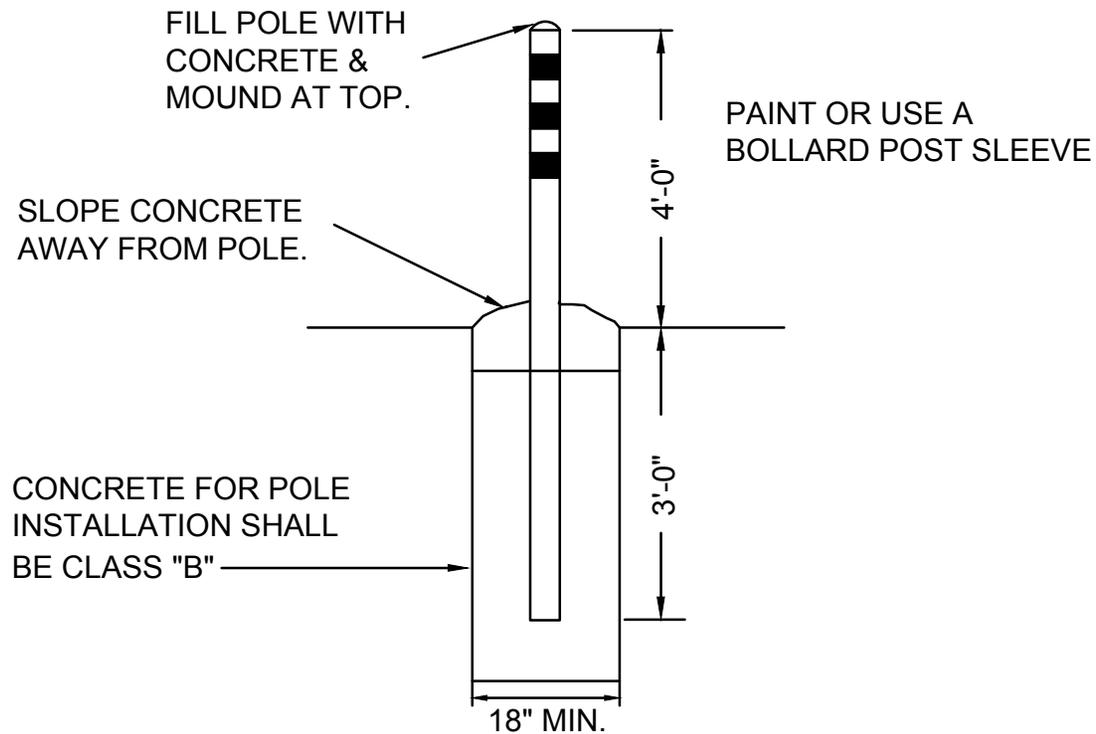
NOT TO SCALE

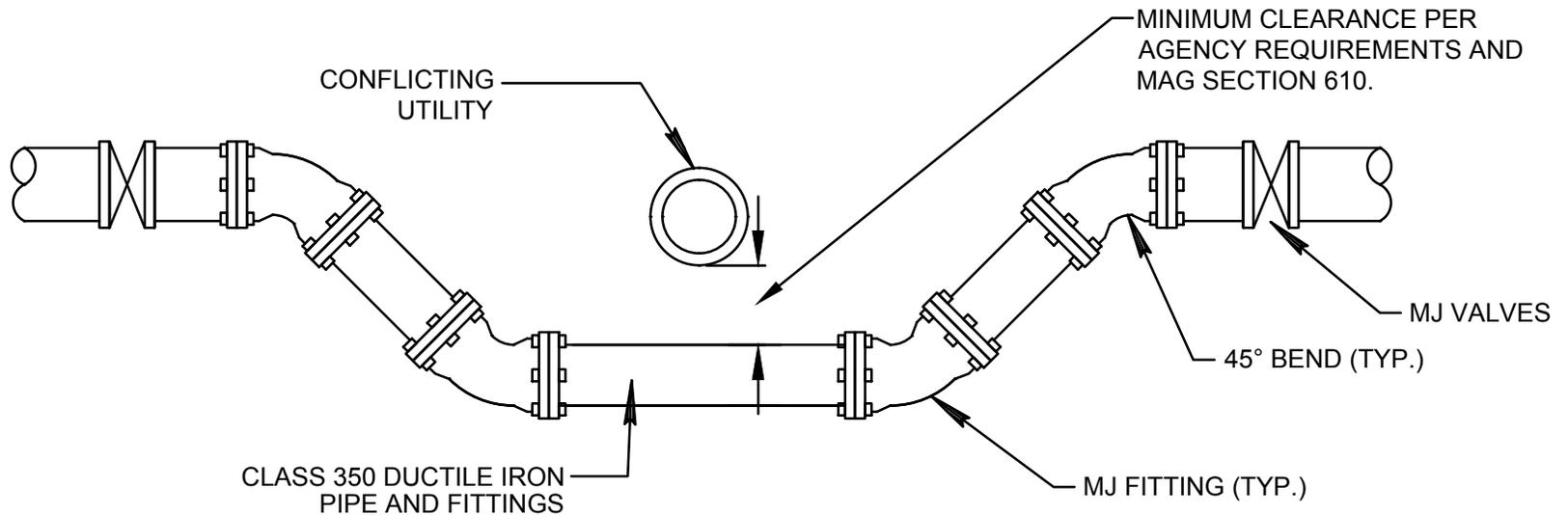
PROTECTION POLE PLACEMENT



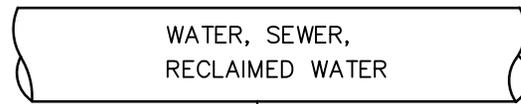
NOTES:

1. DO NOT PLACE POLES IN FRONT OF NOZZLE.
2. POLES MAY BE CHANGED IN NUMBER AND ARRANGEMENT DEPENDING ON INDIVIDUAL NEED.
3. REQUIRED AT SPECIFIED LOCATIONS ONLY.
4. POLE SHALL BE SCHEDULE 40 4" STEEL.
5. PAINT POLE YELLOW AND PLACE 3 - 3" RED REFLECTIVE BANDS.





1. ALL PIPE, VALVES, FITTINGS, AND APPURTENANCES SHALL BE MANUFACTURED IN THE U.S.A.
2. ENTIRE VERTICAL REALIGNMENT SHALL BE MECHANICALLY RESTRAINED CL. 350 D.I.P. PER QUAD CITY DETAILS 303Q-1 AND 303Q-2. RESTRAINED LENGTHS FROM VALVES SHALL BE THE SAME AS DEAD ENDS ON NEW CONSTRUCTION.
3. BOTTOM OF VERTICAL ALIGNMENT SHALL BE ONE PIECE. IF JOINTS ARE REQUIRED, WATER MAIN SEGMENT SHALL BE CENTERED BELOW PIPE OR IN A CASING PIPE.
4. NO SERVICE CONNECTIONS SHALL BE LOCATED WITHIN VERTICAL REALIGNMENT
5. AIR RELEASE VALVE TO BE INSTALLED ON THE VERTICAL REALIGNMENT AT THE HIGH POINT OF THE LOW SIDE.
6. VALVE(S) AND BLOCKING PER QUAD CITY STANDARD DETAIL 301Q (MJ VALVE, TYP.)



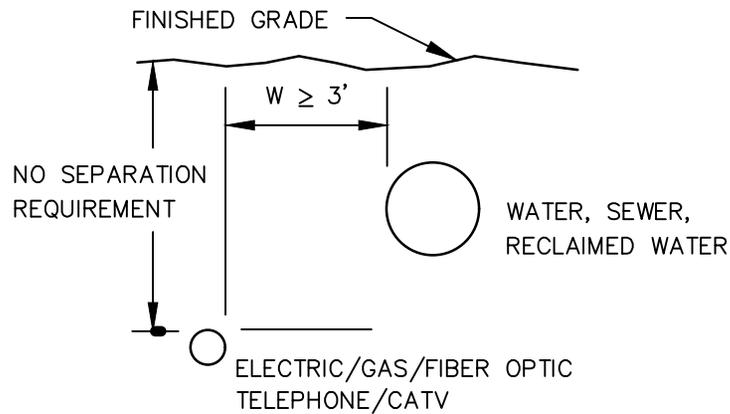
1' MIN.



ELECTRIC/GAS/FIBER OPTIC
TELEPHONE/CATV

CROSSING

W = HORIZONTAL SEPARATION



NOTES:

1. PRIMARY ELECTRIC, GAS, TELEPHONE, CABLE TV OR FIBER OPTIC LINES SHALL NOT CROSS ABOVE A WATER LINE WITHOUT WRITTEN TOWN APPROVAL.
2. LOCATION OF EXISTING WATER, SEWER AND RECLAIMED WATER LINES MUST BE POTHOLED TO VERIFY ACTUAL LOCATION PRIOR TO DESIGN/INSTALLATION.

24" MANHOLE RING AND COVER PER AGENCY
 DETAIL WITH AGENCY LETTERING AND
 (3) 6" MANHOLE RISER RINGS. ADJUST RING TO
 GRADE PER QUAD CITY STANDARD DETAIL 422Q

FINISHED GRADE

DIAMOND PLATE STEEL
 METER BOX COVER
 PER MAG DETAIL 320

ELEVATION SHALL
 BE 2" MIN TO 4"
 MAX
 ABOVE FINISHED
 GRADE

CONCRETE WATER
 METER BOX NO. 2
 PER MAG DETAIL 320

MIN. 14"
 MAX. 18"

4" ABOVE GRADE WITH
 PVC CAP (TYP)
 CAP
 2" ADAPTER
 FORD C84-77G
 6" SDR 35 PVC
 BOTTOM TO REST
 ON BLOCKING

6" GRAVEL BED

2" BRONZE CURB STOP
 FORD B11-777
 (FIP X FIP) OR
 APPROVED EQUAL

2" TYPE K
 COPPER PIPE

TAPPED PLUG OR CAP

ELL COUPLING
 FORD L14-77G

4" BRASS NIPPLE

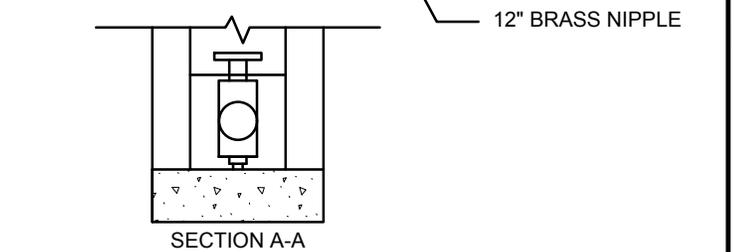
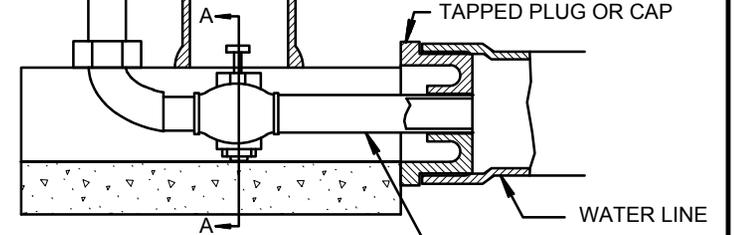
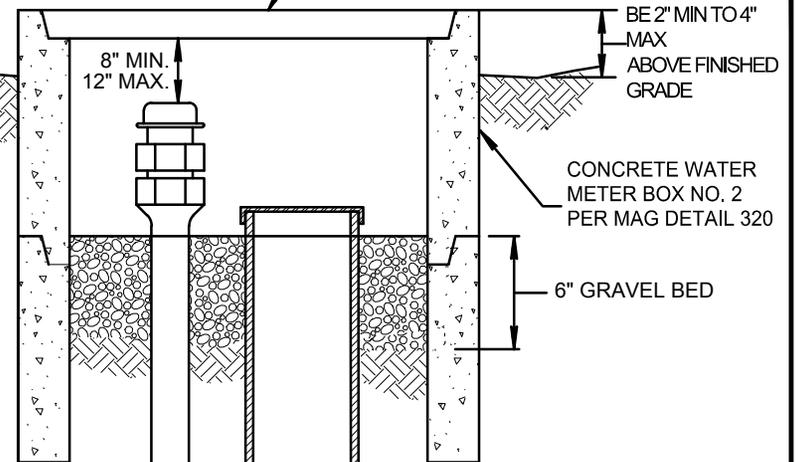
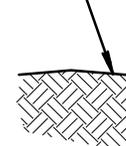
WATER LINE

12" BRASS NIPPLE

SOLID CONCRETE
 BLOCK

NOTE: ADD DEBRIS CAP PER MAG 392

TRAFFIC AREAS



NON-TRAFFIC AREAS

IN DIRT SIDE

IN PAVEMENT SIDE

"WATER", "SEWER", "EFFL."
ON COVER (TYP.)

CLASS "AA" CONCRETE
8" THICK, BROOM FINISH
30" DIAMETER.

4" RADIUS
(TYP)

XXXXXX

0" TO 1/4"

1/4" RADIUS (TYP)

ASPHALTIC
CONCRETE
PAVEMENT

12"
6"

EXTEND TRACER WIRE TO
18" ABOVE GROUND AND
LOOP INSIDE VALVE BOX
ABOVE DEBRIS CAP.

TOP OF THE BOTTOM RISER
SHALL BE A MINIMUM OF 12"
BELOW FINISHED GRADE

2" SQUARE
OPERATOR NUT

TRACER WIRE PER QUAD CITY
STANDARD 319Q-1 AND SPECIFICATIONS

GUIDE RING 1/2"
SMALLER O.D. THAN
RISER I.D. (TYP)

RESTRAIN TWO 3/8" SET
SCREWS ON EITHER SIDE TO
LOCK THE OPERATING NUT

PROVIDE VALVE STEM
EXTENSION WHERE DEPTH
TO OPERATOR NUT
EXCEEDS 4'

GRADELOK SADDLE TYPE VALVE BOX
STABILIZER OR APPROVED EQUAL

NOTES:

1. VALVE BOX SHALL BE ADJUSTED TO THE FINISHED GRADE AFTER PLACING OF THE FINISH PAVEMENT SURFACE.
2. USE PARKSON TYLER #6855, APCO OR EQUAL DEEP SKIRTED LID (4" OR MORE) TYPE, SLIDING ADJUSTABLE CAST IRON VALVE BOX C.I. MIN. T.S. 30,000 P.S.I.
3. ALL VALVES CONNECTED TO EXISTING MAINS ARE TO BE OPERATED BY AGENCY PERSONNEL ONLY.
4. U.S. MANUFACTURED IRON ONLY.
5. ALL STEEL FOR EXTENSION TO HAVE SHOP PRIME COAT ZINC CHROMATE, AND ONE HEAVY APPLICATION NO-OX-10 "A" IN ACCORDANCE WITH MANUFACTURE'S DIRECTION.
6. DEBRIS CAP PER MAG DETAIL 392.
7. VALVE BLOCKING REQUIRED PER QUAD CITY STANDARD DETAIL 301Q.

DEBRIS CAP COLOR TABLE
VALVE TYPE COLOR

| VALVE TYPE | COLOR |
|------------------|--------|
| IN-LINE-RW | BLACK |
| HYDRANT | BLUE |
| BUTTERFLY | YELLOW |
| ZONE | RED |
| FIRE LINE | WHITE |
| EFFLUENT | PURPLE |
| SEWER FORCE MAIN | GREEN |

REMOVAL AND DISPOSAL OF ASBESTOS CEMENT PIPE

1. Asbestos Cement Pipe (ACP) is a mixture of Portland Cement and asbestos fibers.
2. The Environmental Protection Agency (EPA) determined that asbestos, in an airborne condition, is a hazardous material and established laws/guidelines for the handling and disposal of the material. The Asbestos National Emission Standard for Hazardous Air Pollutants (NESHAP) establishes requirements for the removal and disposal of regulated asbestos containing materials. This PVSD establishes procedures and assigns responsibilities for the proper handling of ACP in conformance with the Asbestos NESHAP requirements currently in effect along with any other Federal, State, Local laws, rules and regulations including but not limited to MAG, ANSI, AWWA, EPA and OSHA standards
3. As used herein, the term "Town" shall refer to the Town of Prescott Valley or designated representative. The term "Excavator" shall refer to that entity (individual or contractor) which actually excavates and exposes the pipe. The term "Generator" means any owner or operator whose act or process produces asbestos containing waste material.
4. It is the intent of the Town to comply with the requirements of the Asbestos NESHAPS found at 40 CFR Part 61, Subpart M. This PVSD establishes procedures to be used by all Excavators in the removal and disposal of ACP in compliance with NESHAPS. Nothing in this PVSD shall be construed to void any provision of a contract or other law, ordinance, regulation or policy whose requirements are more stringent.
5. It is the intent of the Town that all ACP shall be removed in such careful and prudent manner that it remains intact and non-friable. The Excavator is responsible to employ those means, methods, techniques, and sequences to ensure this result.
6. The Excavator of non-hazardous ACP and the Generator of hazardous ACP is responsible for all identification measures, costs, notifications, documentation, proper handling, transportation, disposal of the material, etc. This would include costs to retain the services of a qualified, licensed asbestos abatement consultant, if required. Therefore, it is the policy of the Town that if the actions of the Excavator cause the material to become friable (hazardous), said Excavator becomes the Generator.
7. The Excavator shall not direct-bury any of the ACP without approval from the Town. If sections of ACP are to be left in the ground and abandoned in place, the Town / Consultant shall inspect the visible sections that remain to insure they are intact and non-friable. The ends of the ACP shall be encapsulated and any friable ACP shall be removed. ACP shall not be crushed and left in place. If ACP is crushed or otherwise caused to become friable, it shall be removed.
8. The Town / Consultant shall not direct the means, methods or sequence of work of the Excavator nor shall he/she be responsible for the Excavator's safety programs or procedures. Compliance with all aspects of worker safety and health regulations including but not limited to the OSHA Asbestos Standard is the responsibility of the Excavator. The Town assumes no responsibility for compliance programs, which are the responsibility of the Excavator. However, should the Town / Consultant determine the means, methods, sequences or safety measures are contrary to this PVSD or any applicable law, ordinance or regulation, the Town may issue a Stop Work Order which shall remain in effect until such time that the Excavator has made the necessary corrections
9. ACP must be disposed of at a landfill licensed and approved for acceptance of said products.
10. The Excavator shall provide documentation and certification to the Town detailing proper transport and disposal of ACP.

PV STANDARD DETAIL

ASBESTOS CEMENT PIPE

TOWN ENGINEER

REVISED:
07/16

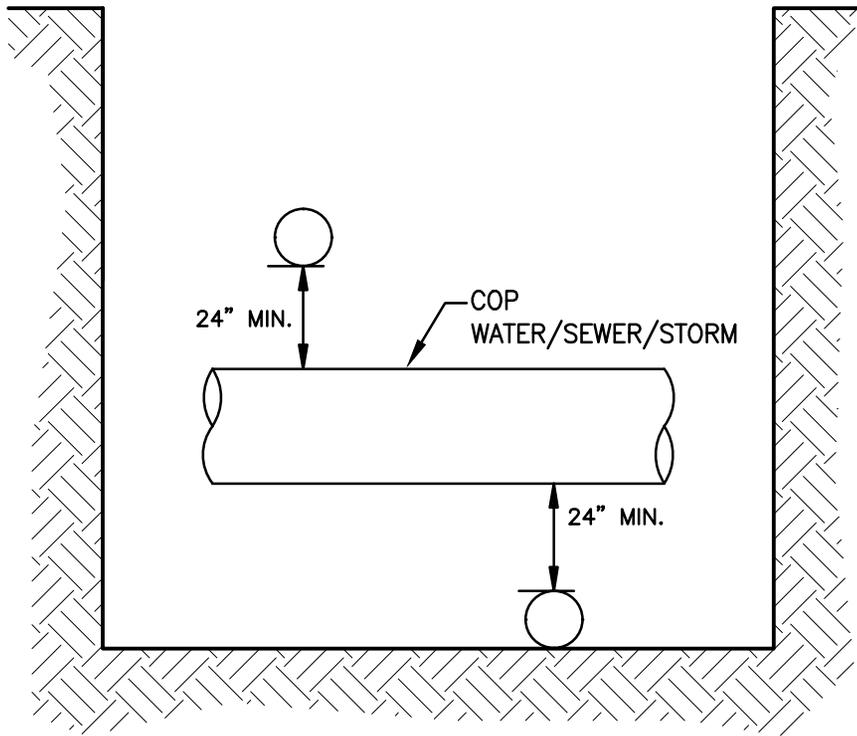
DETAIL No.
395PV

Utility Abandonment in Place

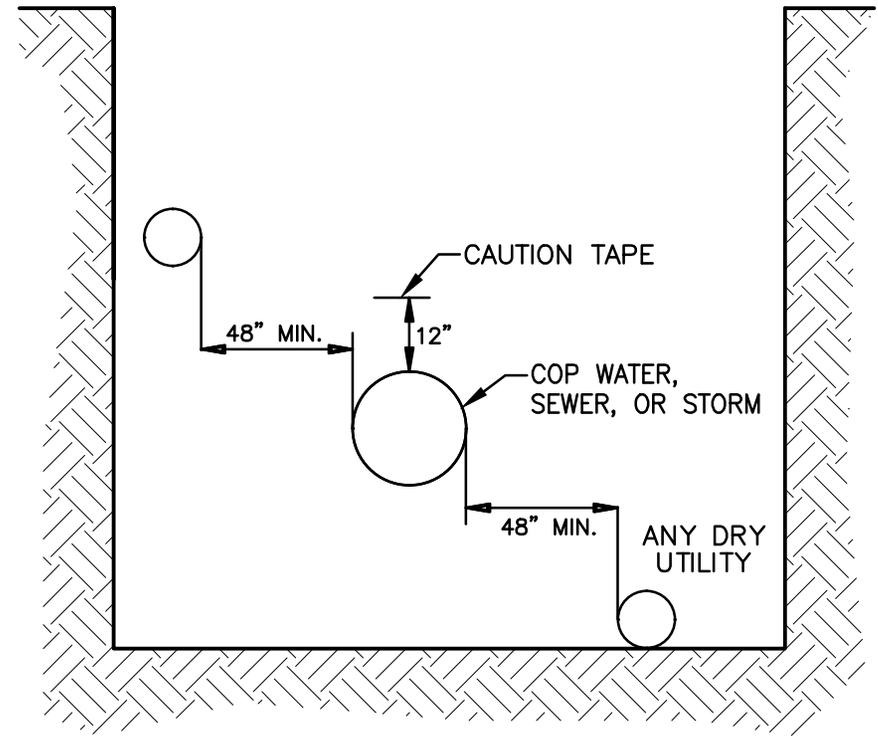
1. Abandonment in place of public utility lines shall be at the discretion of the Town, reviewed on a case by case basis, and shall be in accordance with these instructions.
2. Abandonment in Place of public utility infrastructure (water, sewer, reclaimed water) is based upon Town approval and shall be in accordance with the following:
 - 1) All requests to abandon utility pipe must be submitted in writing to the Utilities Department by formal letter or email and accompanied by 3 sets of drawings certified by an Engineer registered in the State of Arizona.
 - 2) Upon approval, the applicant shall provide to the Town an as-built Mylar drawing (24" x 36") with approval stamp and as-built certification by an Engineer registered in the State of Arizona.
 - 3) Plan view with profile is required.
 - 4) Abandoned pipes are not allowed if new pipe is being installed in the same easement or ROW.
 - 5) Survey grade GPS coordinates of the pipe are required. The coordinates shall designate start of pipe, any start and finish of bends, and end of pipe.
 - 6) The ends of the pipe must be plugged with waterproof grout or sealed with a manufactured cap made for such purposes.
 - 7) Abandonment not allowed for the following infrastructure:
 - Manholes
 - Valves *
 - Cleanouts
 - Pressure Reducing Valves *
 - Air Release Valves *
 - Meters *
 - Meter boxes *
 - Fire Hydrants *
 - Blow-Offs *

*Salvage and arrange for the above materials to be picked up by the Town' maintenance and operations contract operator unless otherwise directed.

- 8) Abandon unused sanitary sewer laterals in place from back of the Right-of-Way (ROW) to the main. The end of the pipe must be plugged with waterproof grout or sealed with a manufactured cap made for such purposes. Survey grade GPS coordinates of the end of the pipe is required.
- 9) Abandon unused water service lines in place from the face of the ROW to the main. Isolate corp-stop and cut a segment of the copper tubing within six (6) inches of the corp-stop. Also, remove meter setter and box at curb. To abandon connections larger than two (2) inches, a mechanical fitting, cap, plug or valve must be installed as part of the abandonment.



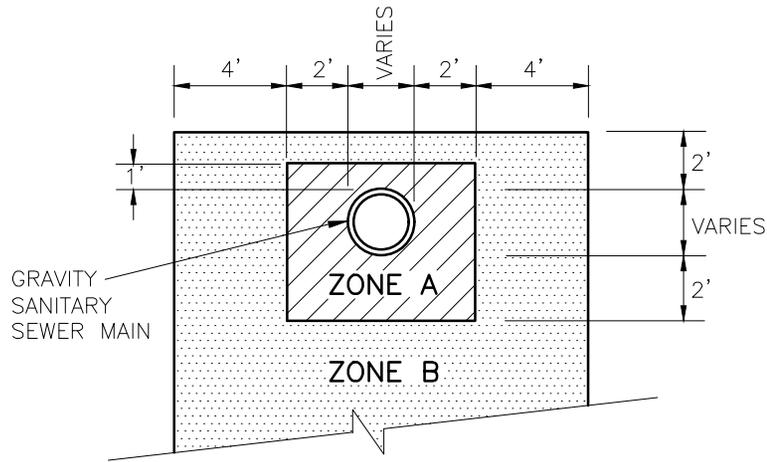
VERTICAL CLEARANCE



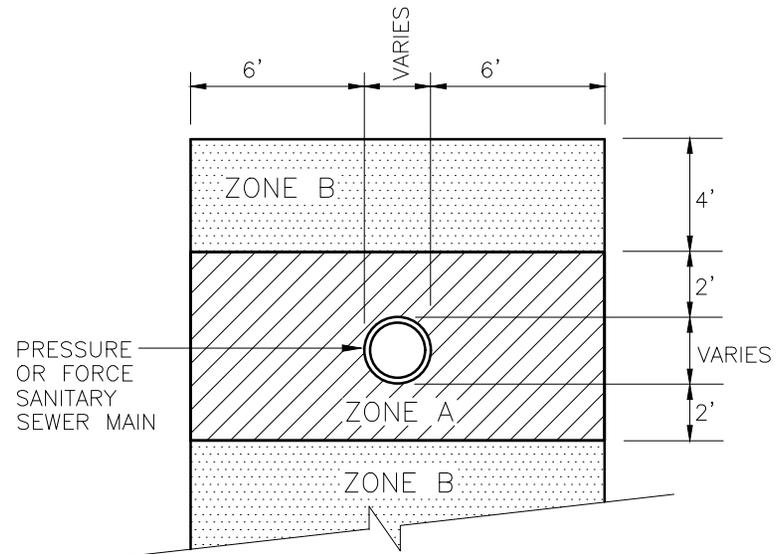
HORIZONTAL CLEARANCE

WATER LINE EXCLUSION AND EXTRA PROTECTION ZONES*

GRAVITY SANITARY SEWER



PRESSURIZED SANITARY SEWER



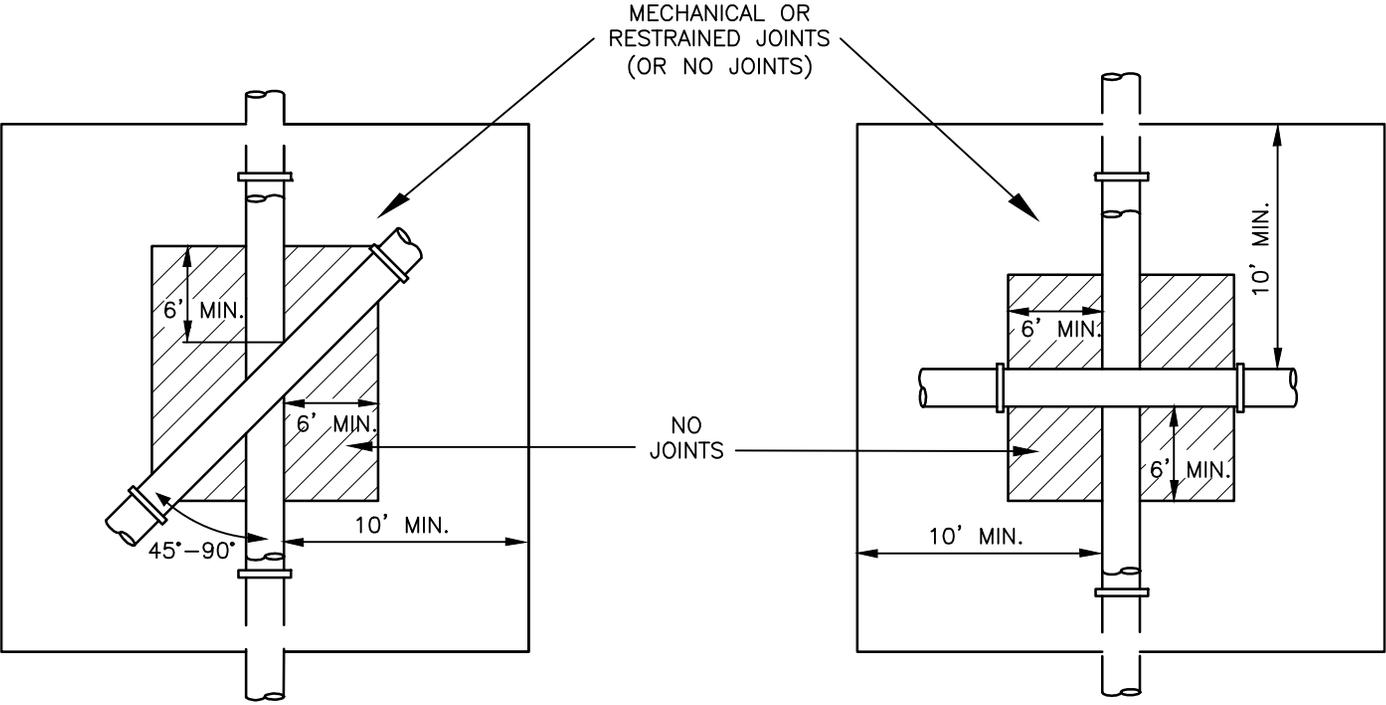
NOTES:

ZONE A: NO WATER LINES ALLOWED/MINIMUM SEPARATION.

ZONE B: EXTRA PROTECTION REQUIRED FOR WATER LINES.

* REFER TO MAG SPECIFICATION SECTION 610, WATER LINE CONSTRUCTION.

WATER LINE EXTRA PROTECTION
 DUCTILE IRON PIPE WITH RESTRAINED OR MECHANICAL JOINTS*

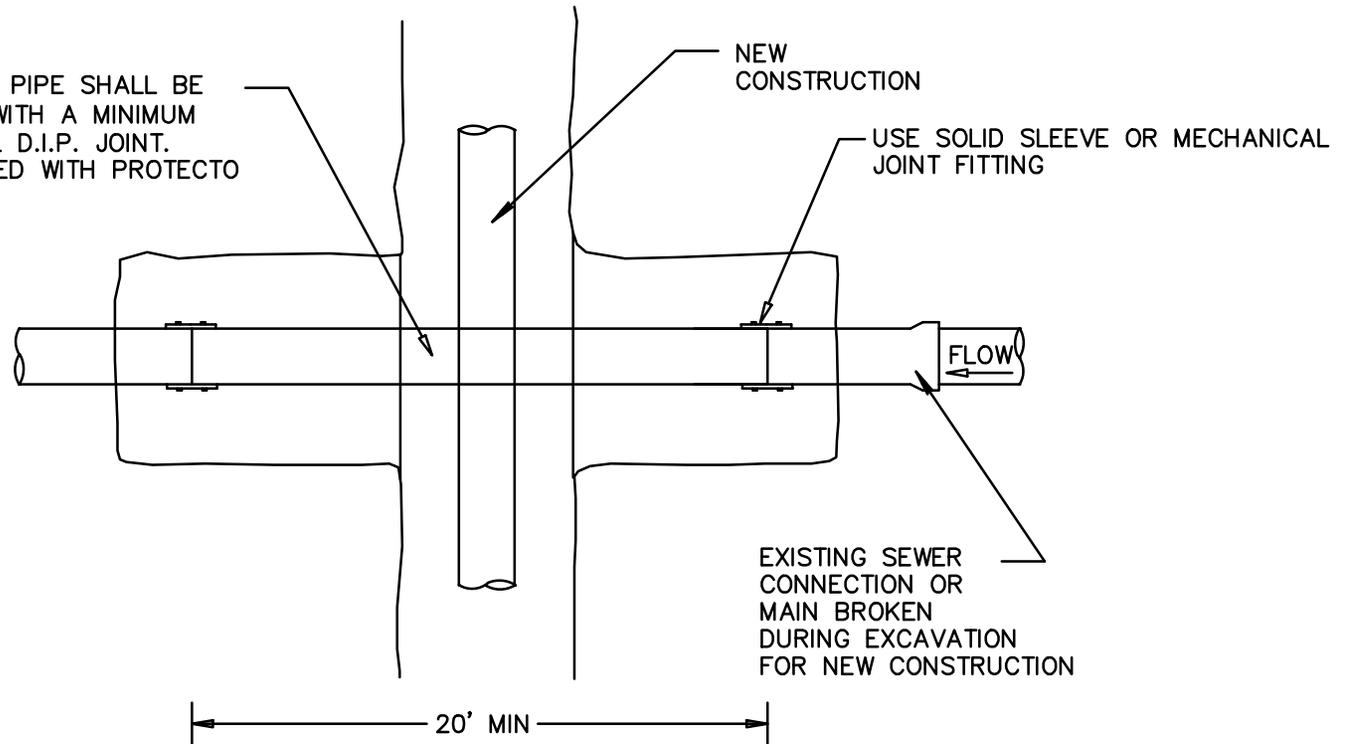


EXTRA PROTECTION DUCTILE IRON PIPE
 (GRAVITY OR PRESSURIZED) SEWER LINE

NOTES:

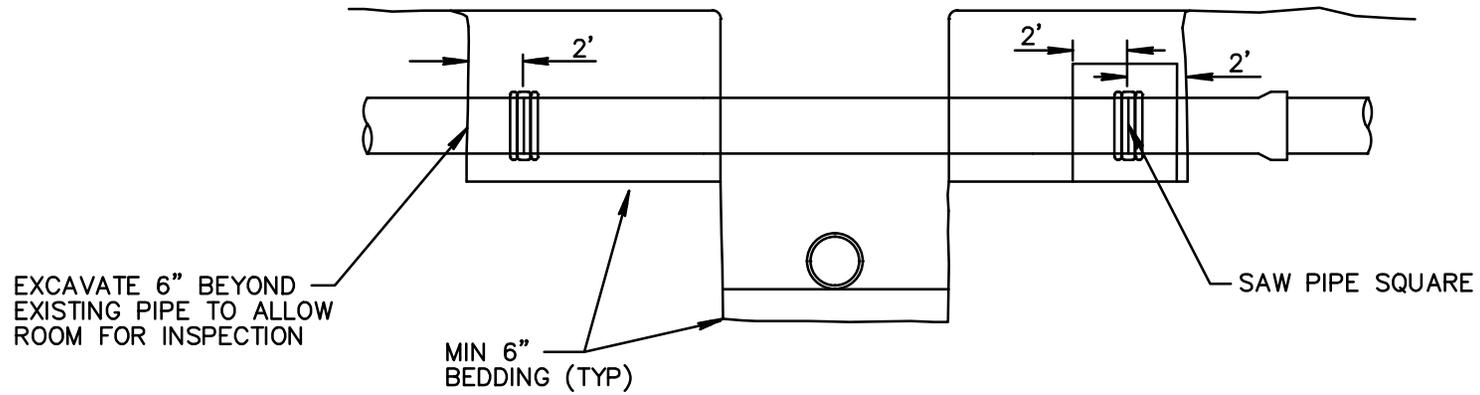
* REFER TO MAG STANDARD SPECIFICATION SECTION 610.

BROKEN PIPE SHALL BE REPLACED WITH A MINIMUM OF ONE FULL D.I.P. JOINT. EPOXY COATED WITH PROTECTO COAT 401



COMPACTION SHALL BE DONE IN ACCORDANCE WITH QUAD CITY STANDARD DETAIL 200Q

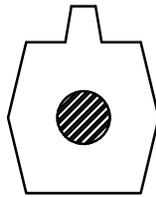
MAINTAIN EXISTING FLOW LINE



EXCAVATE 6" BEYOND EXISTING PIPE TO ALLOW ROOM FOR INSPECTION

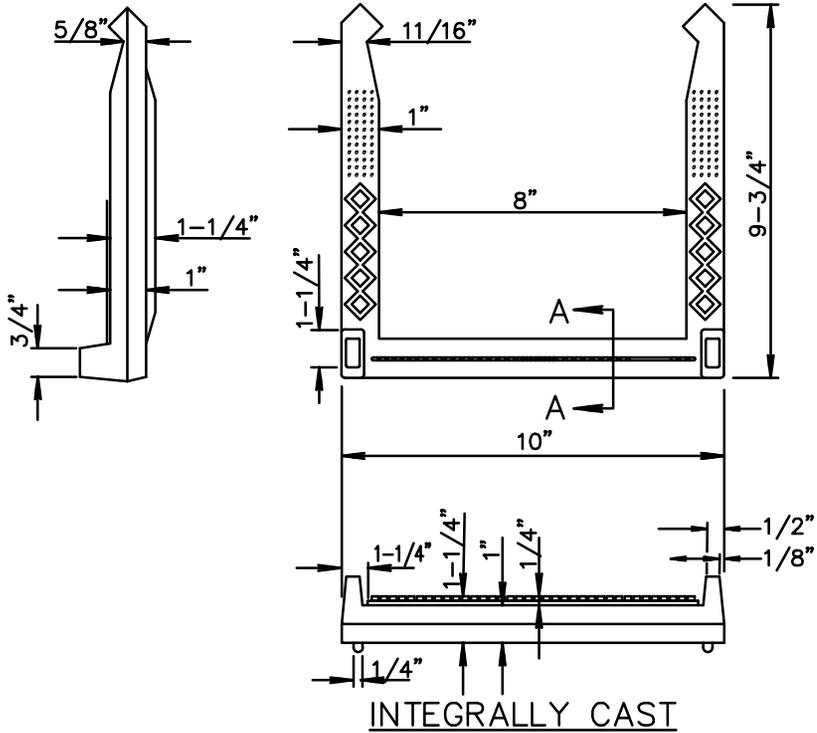
MIN 6" BEDDING (TYP)

SAW PIPE SQUARE



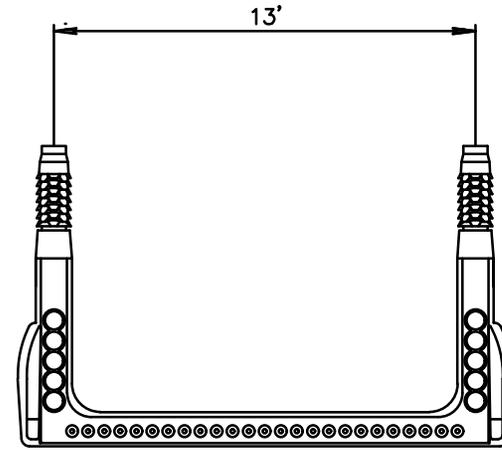
POLYPROPYLENE
PLASTIC-NO 3
DEFORMED
REBAR

SECTION A-A

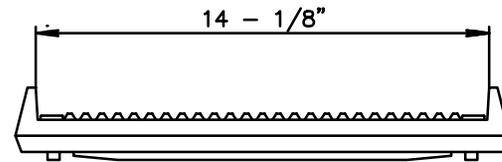


NOTES

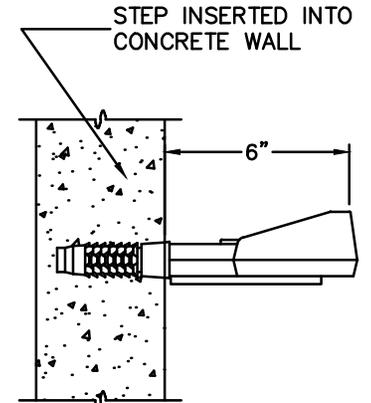
1. STEPS SHALL BE PLACED INTO WET CONCRETE WALL DURING MANUFACTURE.
2. POLYPROPYLENE MUST MEET REQUIREMENTS OF A.S.T.M. 2146, TYPE II, GRADE 16906.



TOP VIEW



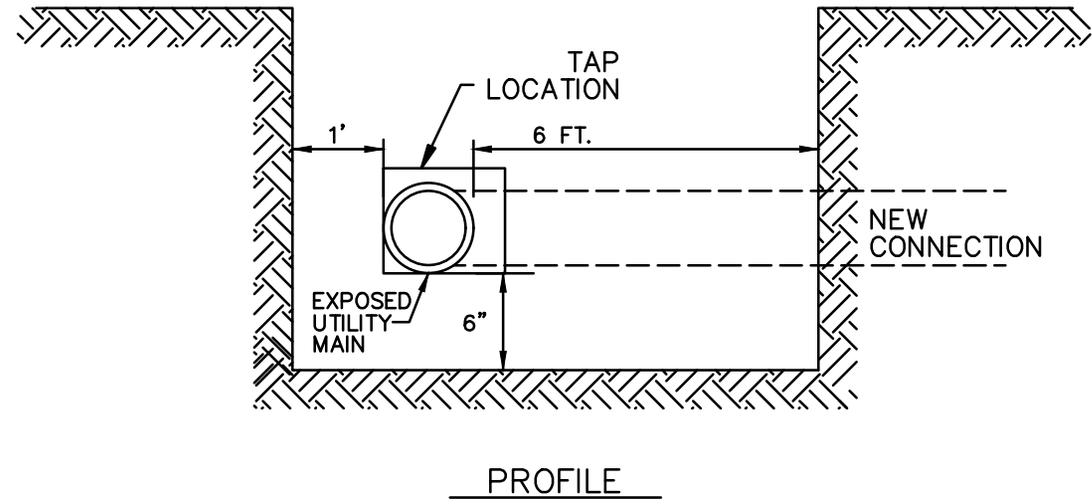
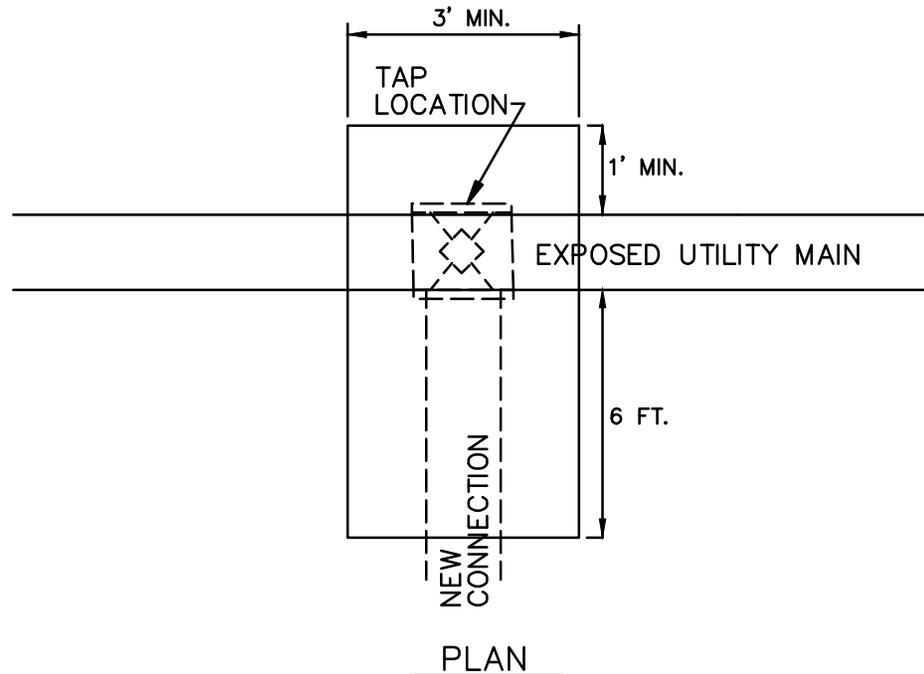
FRONT VIEW



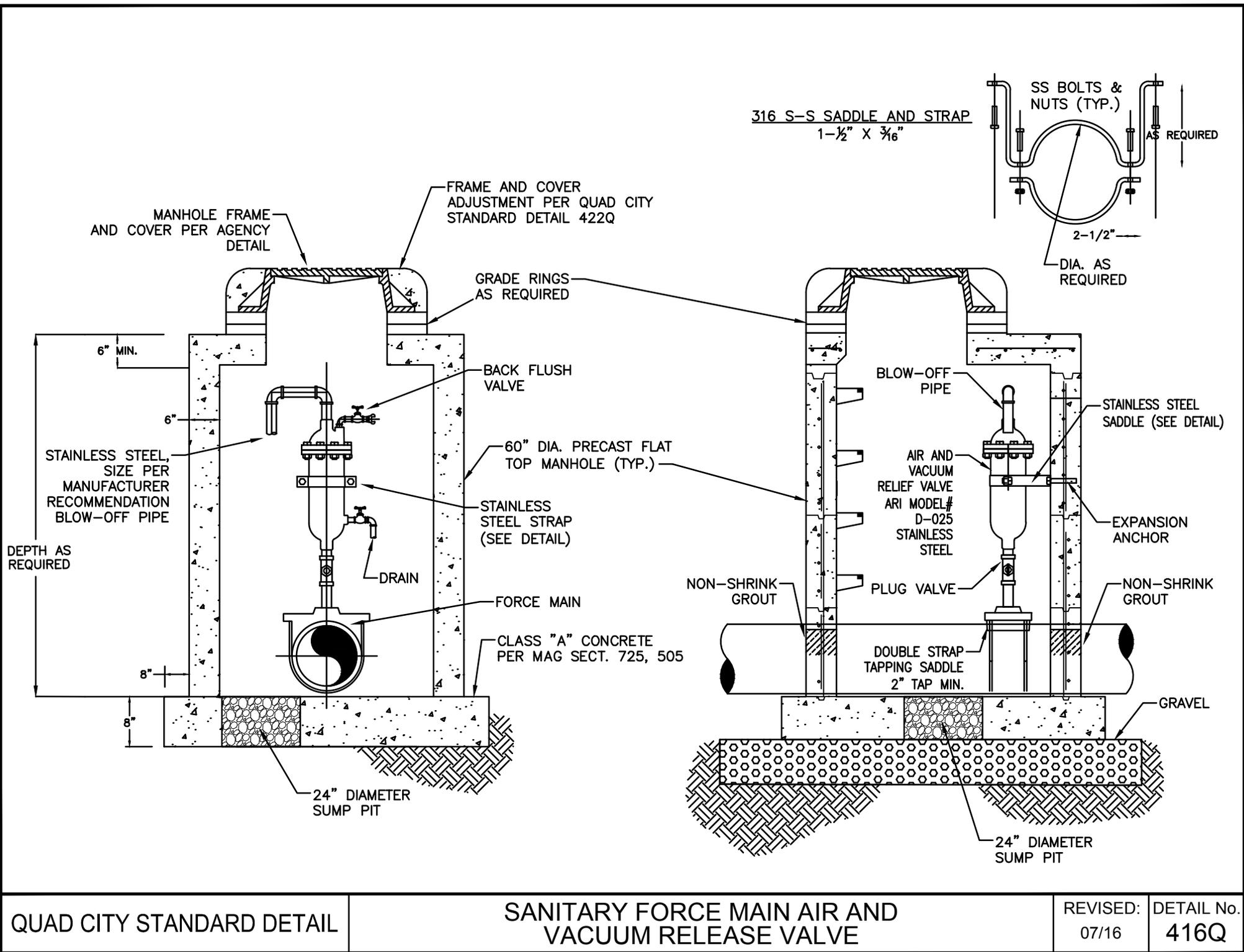
SIDE VIEW

NOTES

1. ALL STEPS SHALL MEET THE REQUIREMENTS OUTLINED IN ASTM D-4101, TYPE II.
2. THE STEEL USED IN MANUFACTURING IS A #4 DEFORMED REBAR GRADE 60, ASTM A-615.



- NOTES:
1. CONTRACTOR MUST EXPOSE MAIN TO THESE MINIMUM DIMENSIONS.
 2. CONTRACTOR TO PROVIDE SAFE EXCAVATION CONFORMING TO O.S.H.A. REGULATIONS.
 3. TAPS PERFORMED BY CITY PERSONNEL ONLY.



QUAD CITY STANDARD DETAIL

SANITARY FORCE MAIN AIR AND VACUUM RELEASE VALVE

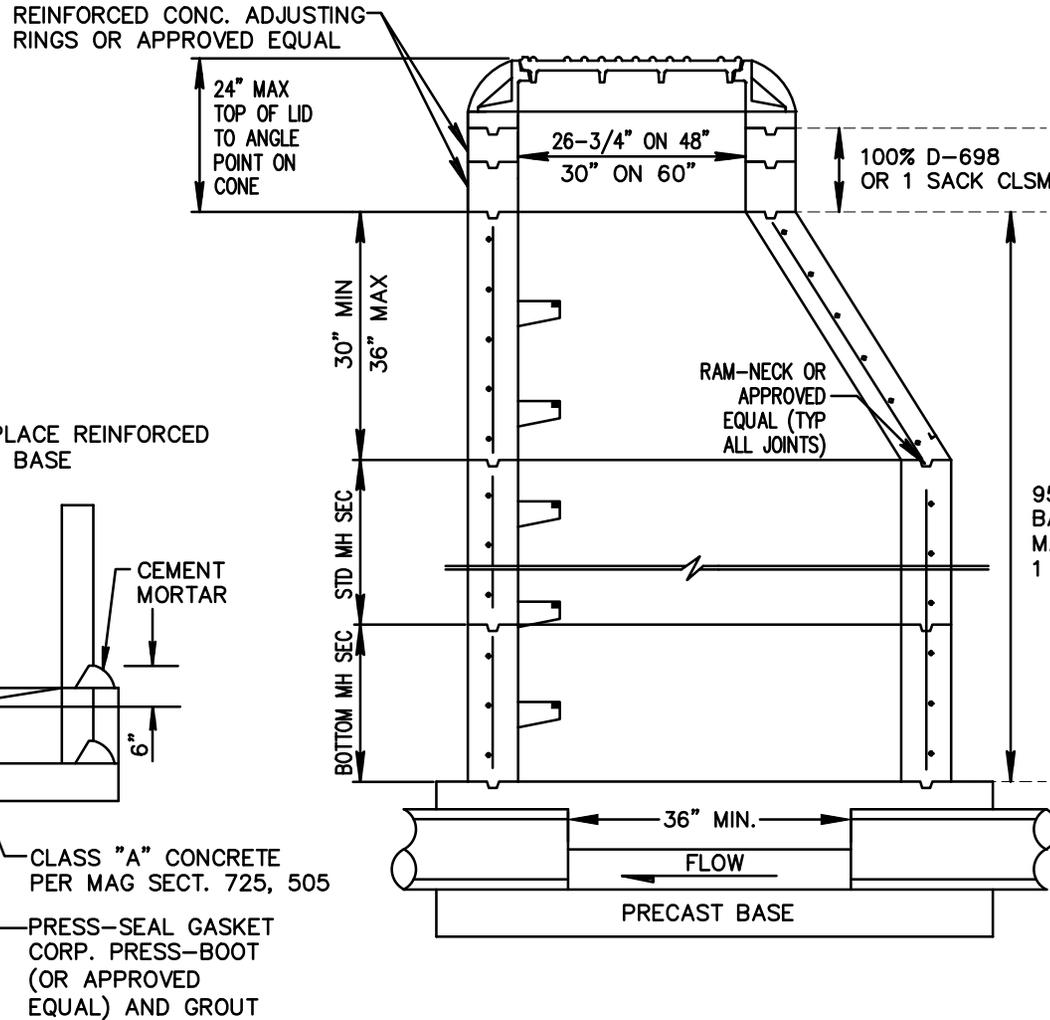
REVISED: 07/16
DETAIL No. 416Q

MANHOLE INSIDE DIAMETER REQUIREMENTS

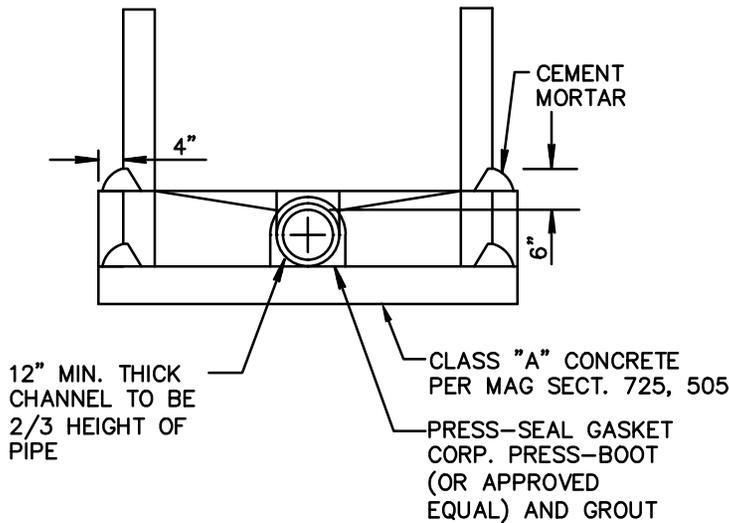
| PIPE DIAMETER | DEPTH OF MAIN | MANHOLE DIA | RING & COVER |
|-----------------|---------------|-------------|--------------|
| 8" - 12" | UP TO 12' | 4' | 24" DIA |
| 8" - 12" | 12' - 20' | 5' | 30" DIA |
| 15" - 18" | UP TO 20' | 5' | 30" DIA |
| *21" AND LARGER | UP TO 20' | 6' | 30" DIA |

*24" AND LARGER REQUIRE COP APPROVAL

- NOTES:
1. PRE-CAST, REINFORCED MANHOLE SECTIONS SHALL BE MANUFACTURED IN ACCORDANCE WITH A.S.T.M. C-478
 2. MANHOLE FRAME AND COVER PER AGENCY REQUIREMENTS AND ADJUSTMENT PER 422Q.
 3. MANHOLE STEPS PER QUAD CITY STANDARD DETAIL 412Q. STEPS TO BE LOCATED ON OULET SIDE OF MANHOLE.
 4. ALL CONCRETE TO BE CURED AND PROTECTED.
 5. GROUT ALL JOINTS TO SMOOTH SURFACE.
 6. PRESCOTT MANHOLES: REQUIRING LINING SHALL BE LINED WITH SEWER SHIELD 150, SEWERGARD 210S, OR RAVEN 405.
 7. PRESCOTT VALLEY: MANHOLES SHALL BE CONSTRUCTED WITH CONmicSHIELD PER APPLICABLE PVSD AND TOPV DESIGN AND CONSTRUCTION STANDARDS.
 8. CONTRACTOR SHALL VERIFY INVERT AND RIM ELEVATIONS TO ENSURE 24" MAX. DISTANCE FROM CONE TO RIM WILL BE ATTAINED PRIOR TO SETTING CONE.

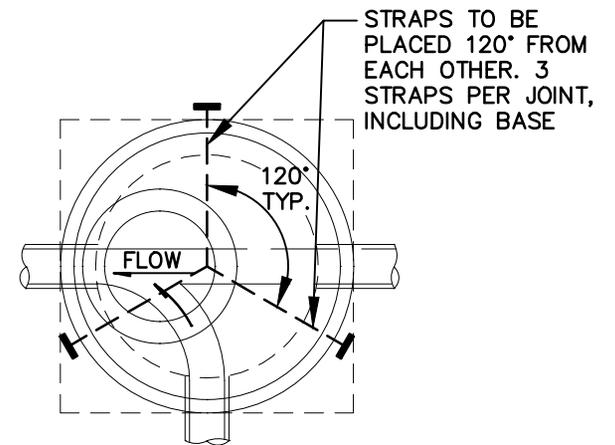
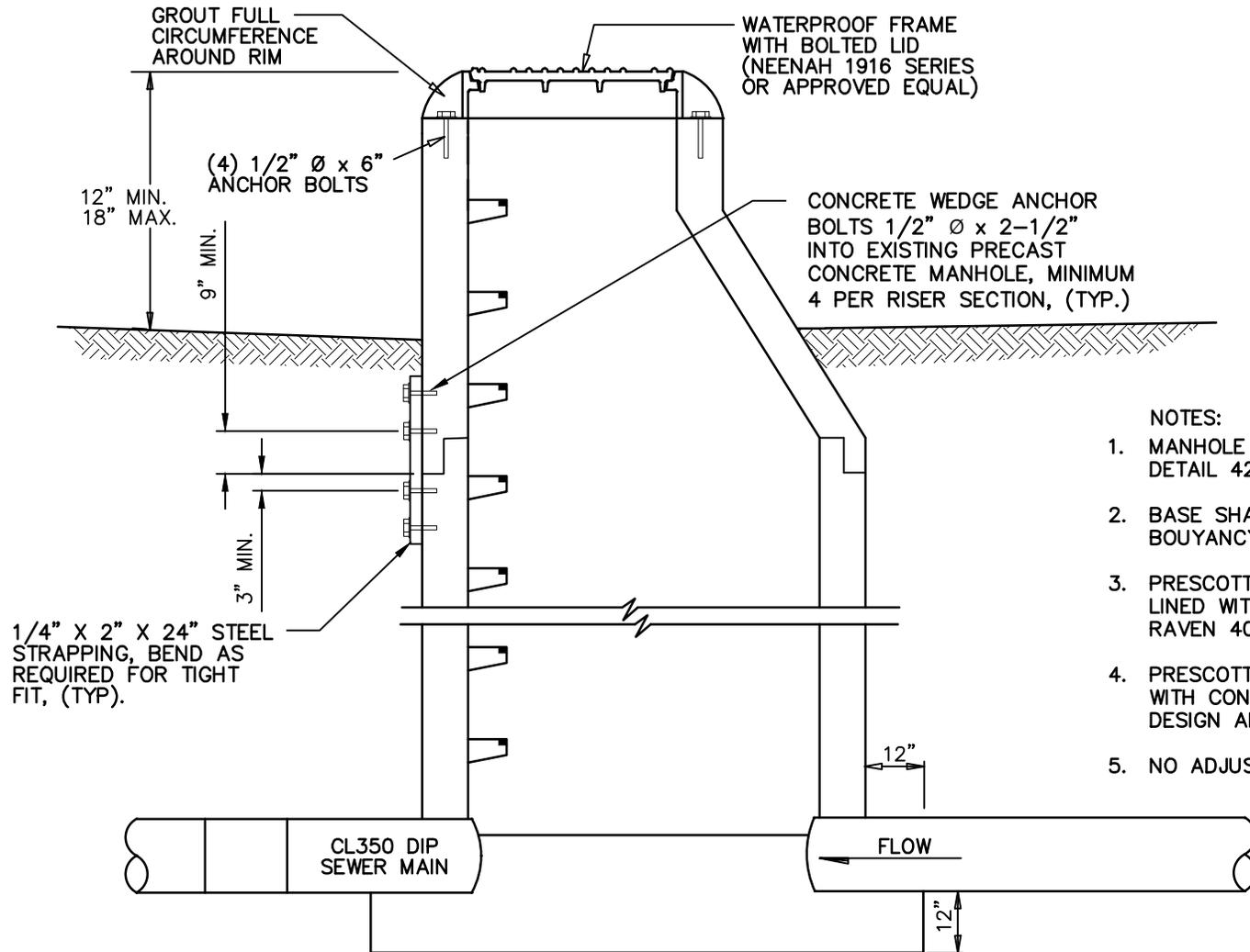


ALTERNATE: CAST IN PLACE REINFORCED CONCRETE BASE



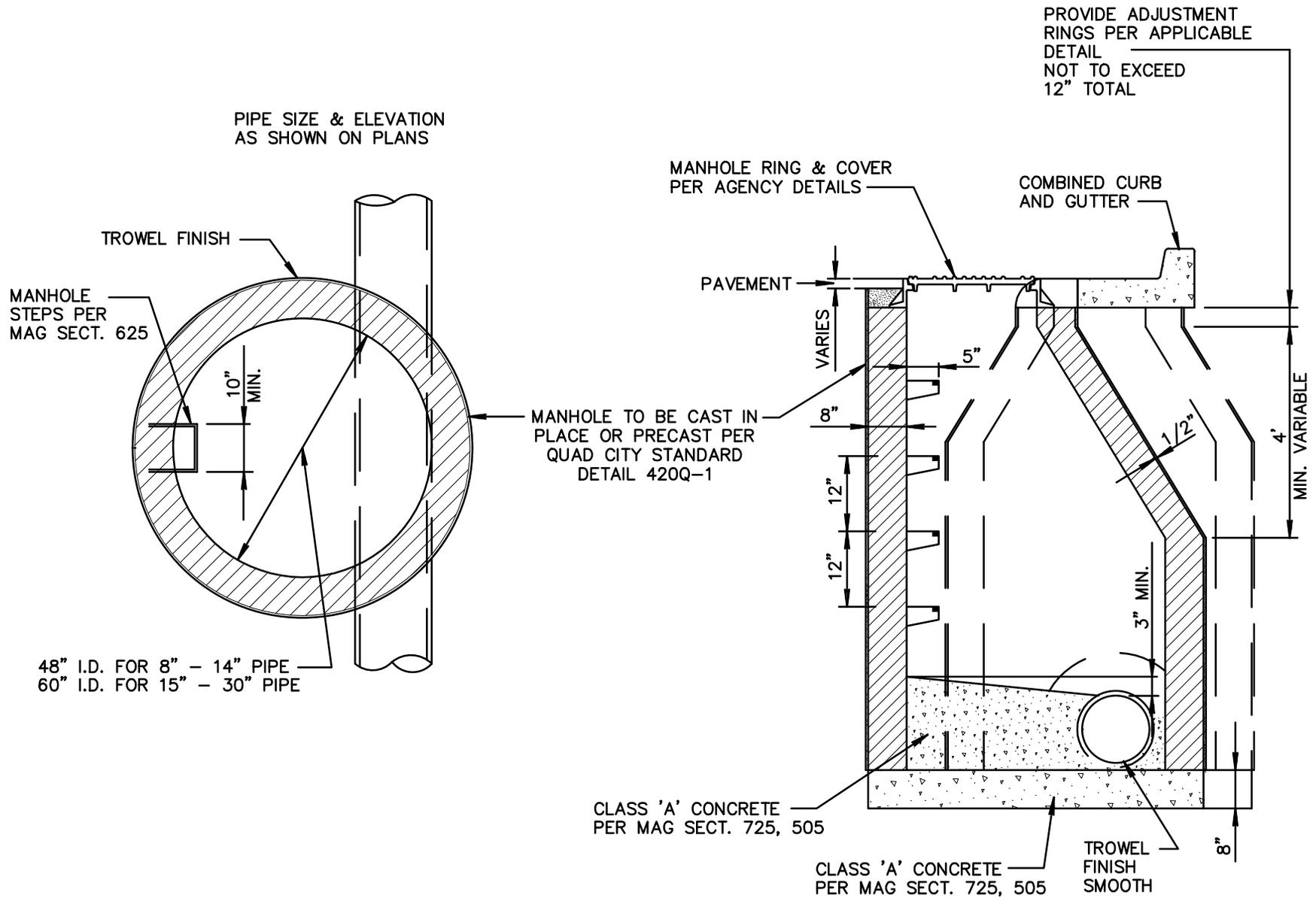
12" MIN. THICK CHANNEL TO BE 2/3 HEIGHT OF PIPE

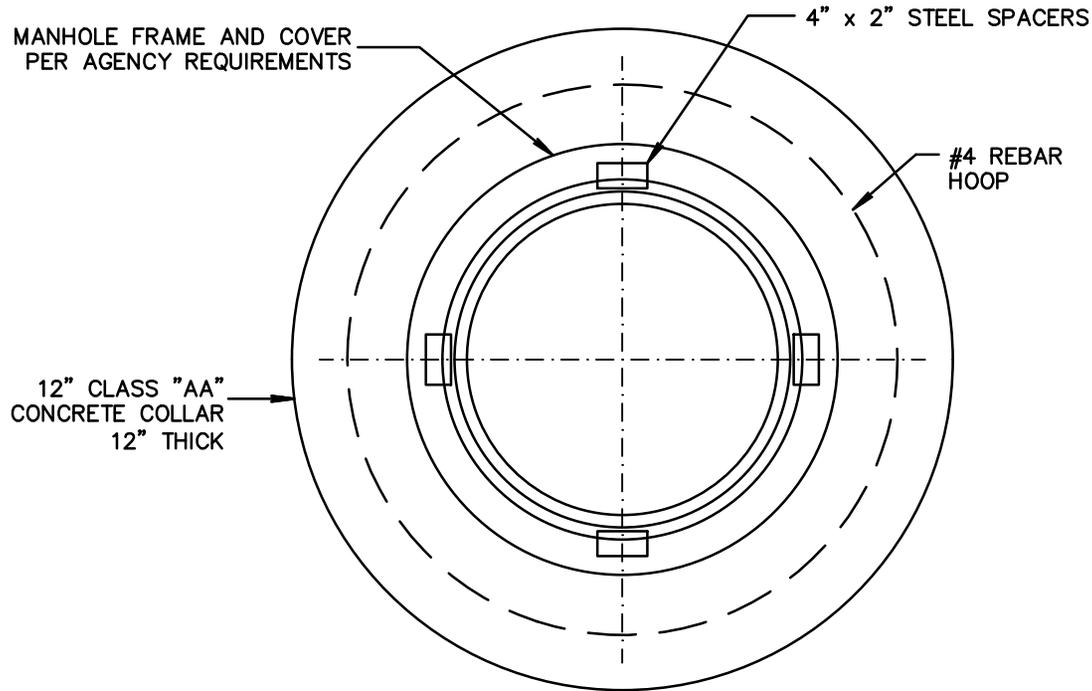
CLASS "A" CONCRETE PER MAG SECT. 725, 505
 PRESS-SEAL GASKET CORP. PRESS-BOOT (OR APPROVED EQUAL) AND GROUT



NOTES:

1. MANHOLE IN ACCORDANCE WITH QUAD CITY STANDARD DETAIL 420Q-1.
2. BASE SHALL BE SIZED FOR EXTERNAL FORCE AND BOUYANCY CALCULATIONS.
3. PRESCOTT MANHOLES: REQUIRING LINING SHALL BE LINED WITH SEWER SHIELD 150, SEWERGARD 210S, OR RAVEN 405.
4. PRESCOTT VALLEY: MANHOLES SHALL BE CONSTRUCTED WITH CONmicSHIELD PER APPLICABLE PVSD AND TOPV DESIGN AND CONSTRUCTION STANDARDS.
5. NO ADJUSTING RINGS SHALL BE USED.

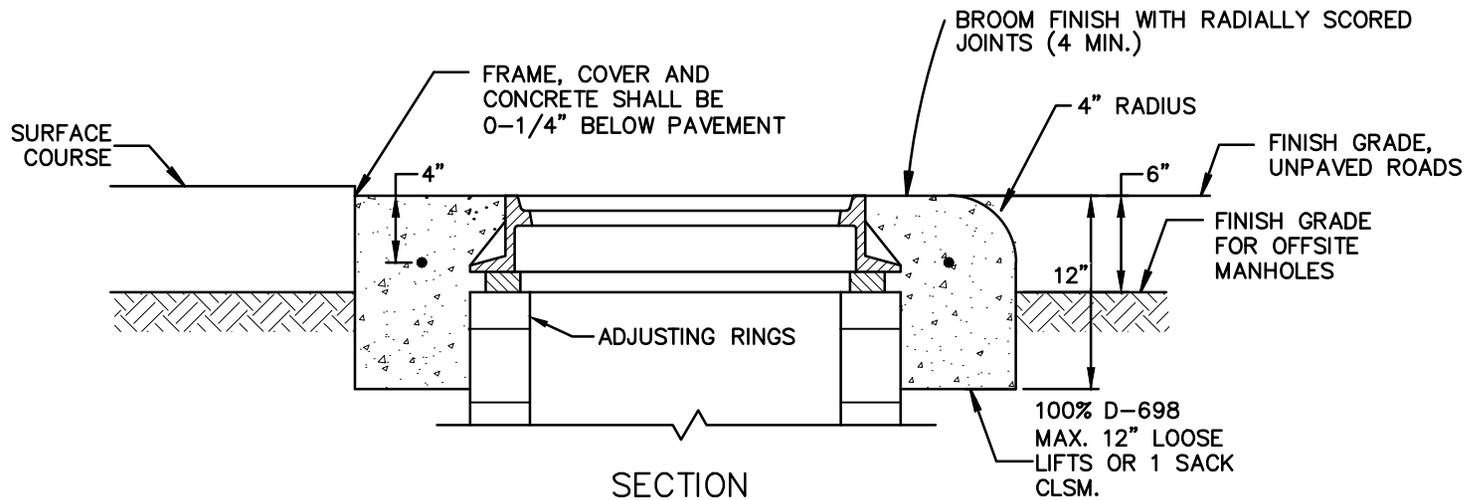




PLAN

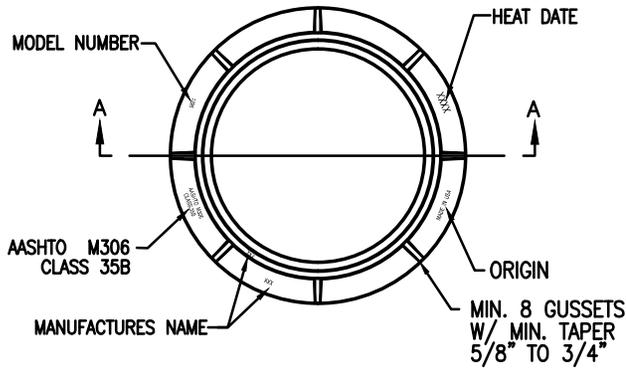
NOTES:

1. SPACERS SHALL BE STEEL OR APPROVED EQUAL.
2. A MINIMUM OF FOUR SPACERS SHALL BE USED AND EQUALLY SPACED, MAXIMUM 4" THICKNESS.
3. AN INTERNAL FORM SHALL BE USED TO PREVENT CONCRETE FROM FALLING INTO MANHOLE.
4. A MINIMUM OF ONE RISER RING SHALL BE USED.

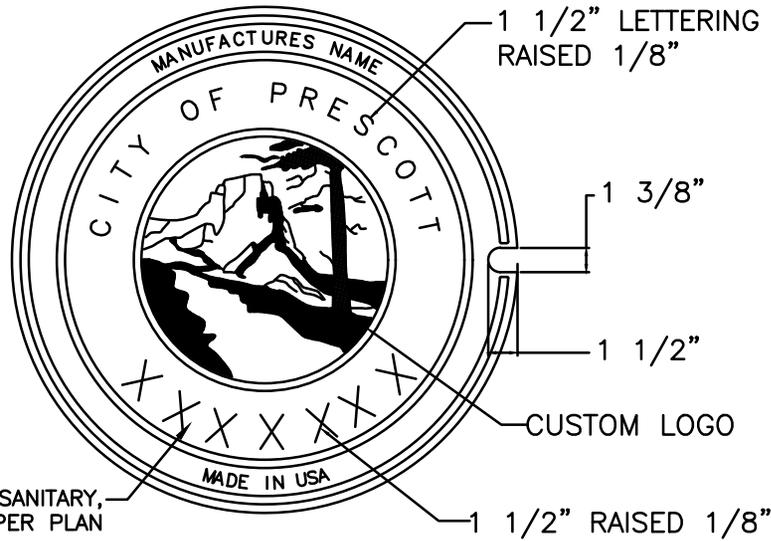


SECTION

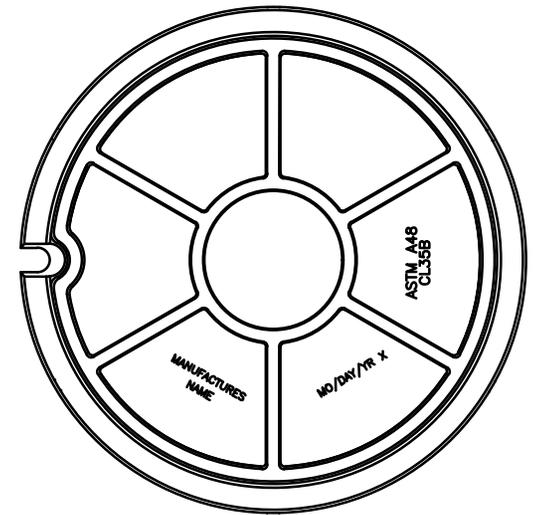
FRAME TOP VIEW



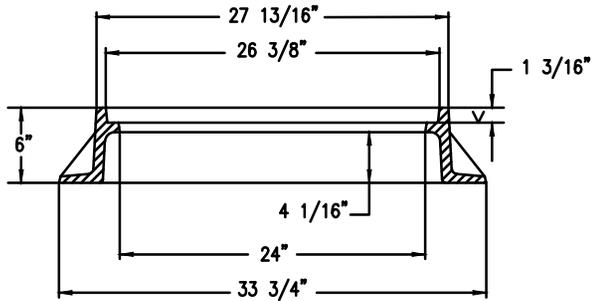
FRAME WT. (CL. 35) - 180 LBS



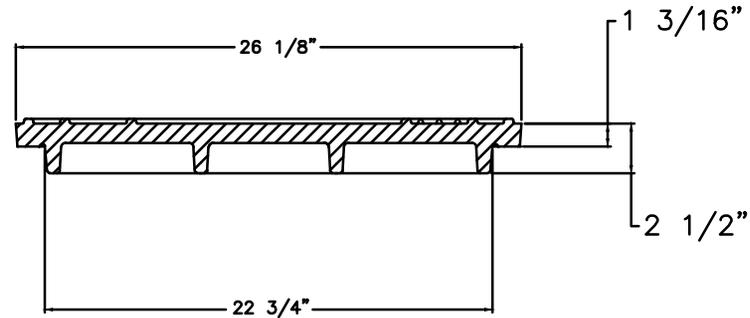
PLAN VIEW



BOTTOM VIEW



SECTION A

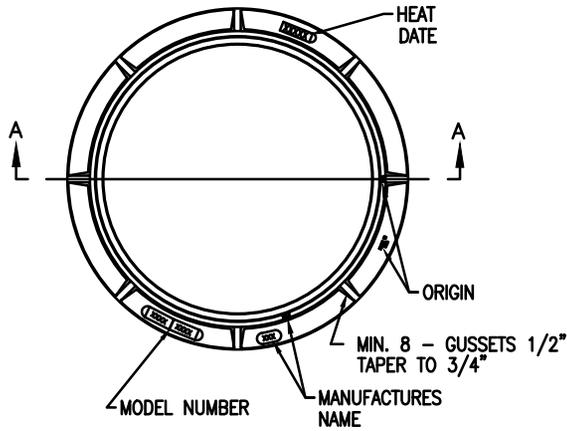


SECTION VIEW

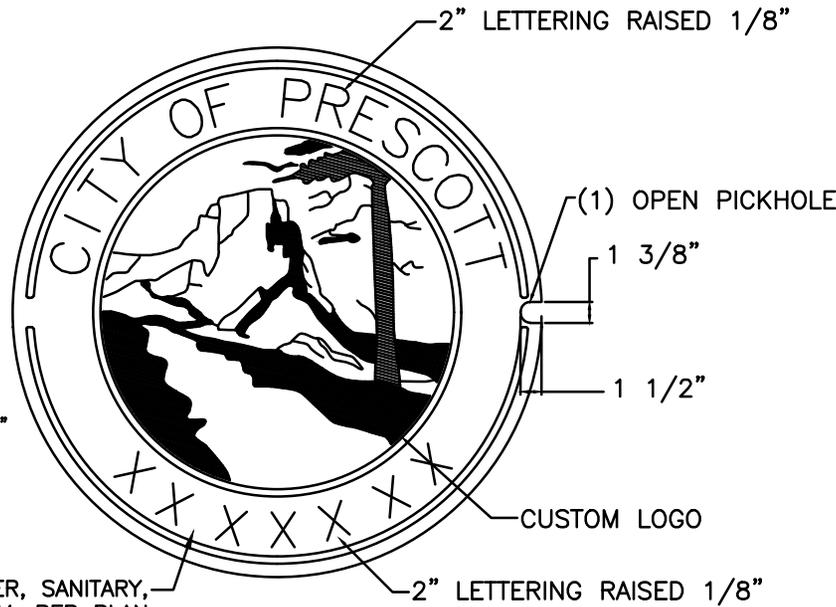
NOTE:

LETTERING ON MANHOLE COVER TO CONTAIN CITY OF PRESCOTT AND UTILITY FOR WHICH MANHOLE IS NEEDED, (I.E. "PRESCOTT SANITARY SEWER"), OR AS DIRECTED. THE TOTAL WIDTH OF INDIVIDUAL LETTERS TO BE SUCH THAT LETTERS AND WORDS ARE EQUALLY SPACED AND BALANCED TO FORM A COMPLETE CIRCLE WITH SPACES BEFORE AND AFTER THE WORD IDENTIFYING THE AGENCY INVOLVED. LETTERS TO BE 1 1/2" IN HEIGHT AND RAISED FLUSH W/ TOP OF COVER. TYPE OF LETTERS TO BE SUBMITTED FOR APPROVAL. WEIGHT OF CASTINGS SHALL BE NO MORE THAN 2% MORE OR LESS THAN THE APPROXIMATE WEIGHT SPECIFIED. CASTINGS SHALL CONFORM TO ASTM A-48, CLASS 35 AND AASHTO M306. THE BEARING SURFACES OF THE FRAMES AND COVERS SHALL BE MACHINED AND THE COVERS SHALL SEAT FIRMLY WITHOUT ROCKING. ALL DIMENSIONS SHALL HAVE A 1/16" TOLERANCE. U.S. MANUFACTURED IRON ONLY.

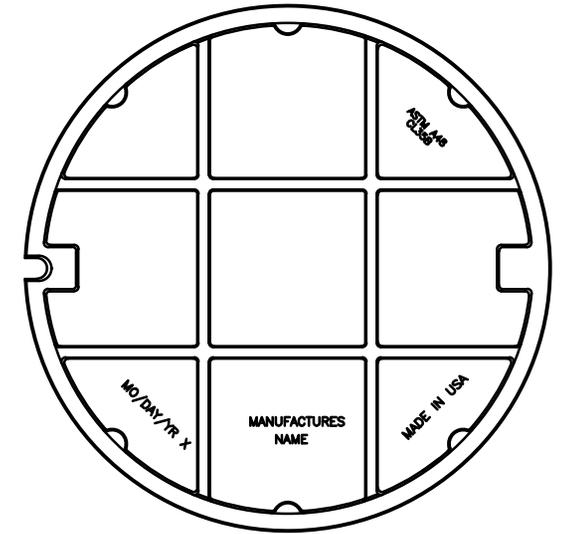
FRAME TOP VIEW



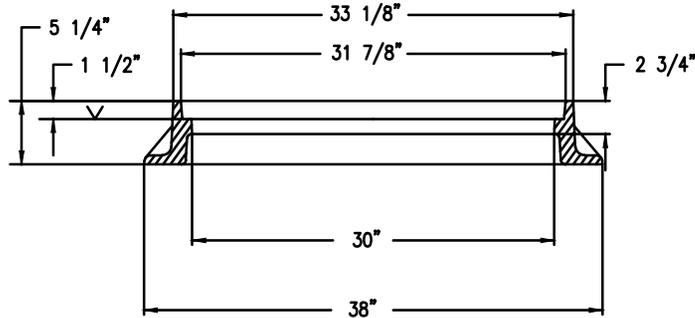
FRAME WT. (CL. 35) - 227 LBS



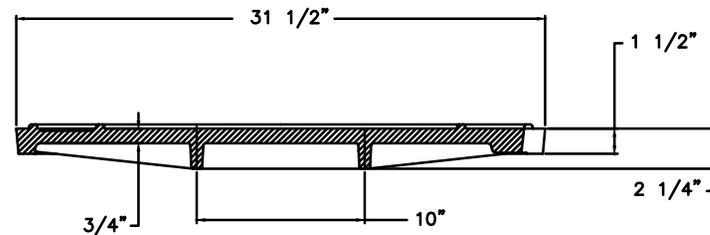
PLAN VIEW



BOTTOM VIEW



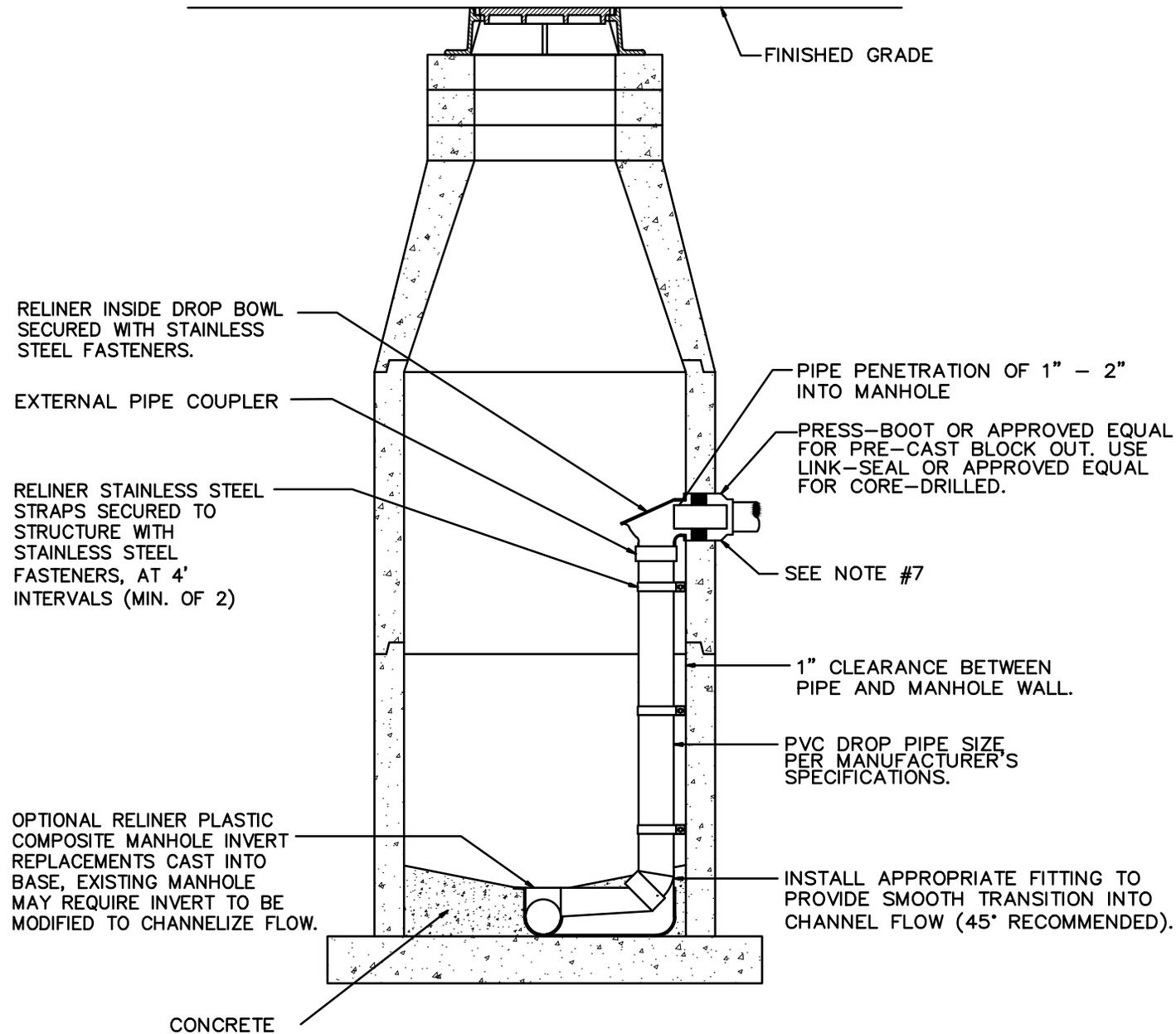
SECTION A



SECTION VIEW

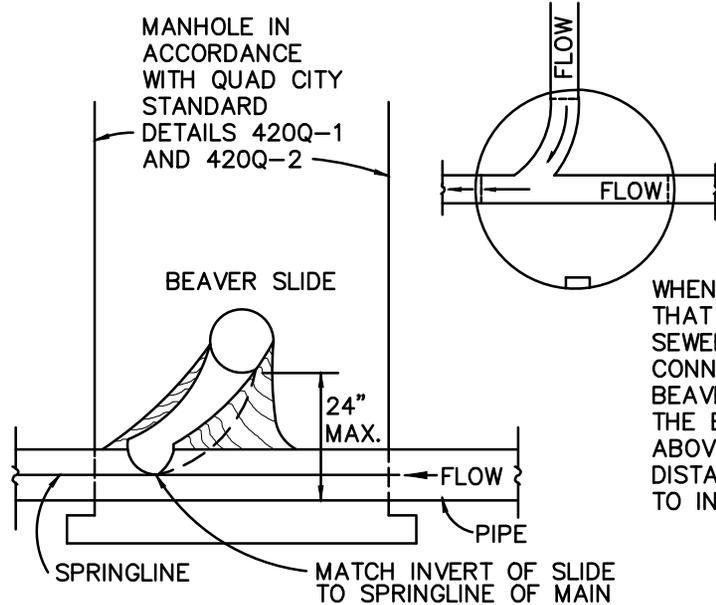
NOTE:

LETTERING ON MANHOLE COVER TO CONTAIN CITY OF PRESCOTT AND UTILITY FOR WHICH MANHOLE IS NEEDED, (I.E. "PRESCOTT SANITARY SEWER"), OR AS DIRECTED. THE TOTAL WIDTH OF INDIVIDUAL LETTERS TO BE SUCH THAT LETTERS AND WORDS ARE EQUALLY SPACED AND BALANCED TO FORM A COMPLETE CIRCLE WITH SPACES BEFORE AND AFTER THE WORD IDENTIFYING THE AGENCY INVOLVED. LETTERS TO BE 2" IN HEIGHT AND RAISED FLUSH W/ TOP OF COVER. TYPE OF LETTERS TO BE SUBMITTED FOR APPROVAL. WEIGHT OF CASTINGS SHALL BE NO MORE THAN 2% MORE OR LESS THAN THE APPROXIMATE WEIGHT SPECIFIED. CASTINGS SHALL CONFORM TO ASTM A-48, CLASS 35 AND AASHTO M306. THE BEARING SURFACES OF THE FRAMES AND COVERS SHALL BE MACHINED AND THE COVERS SHALL SEAT FIRMLY WITHOUT ROCKING. ALL DIMENSIONS SHALL HAVE A 1/16" TOLERANCE. U.S. MANUFACTURED IRON ONLY.

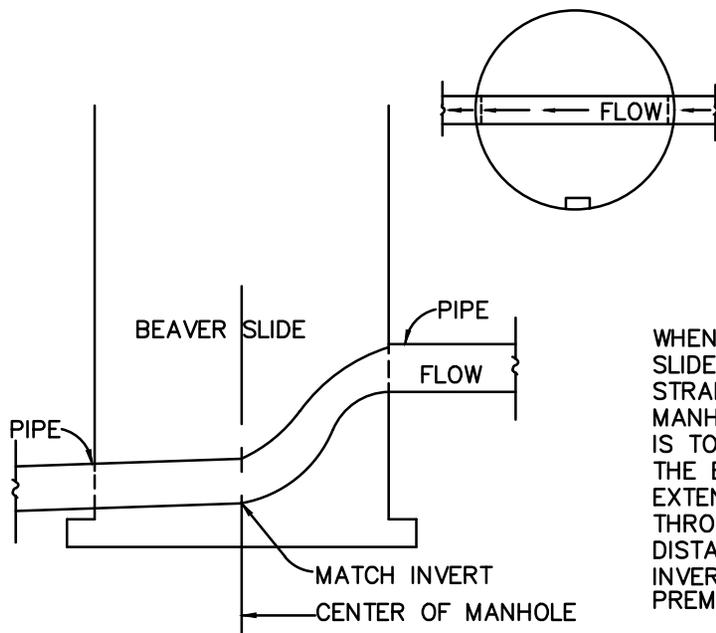


NOTES:

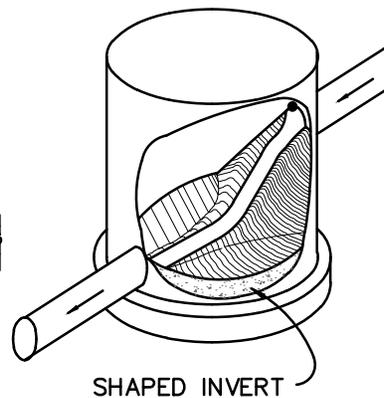
1. MANHOLE IN ACCORDANCE WITH QUAD CITY STANDARD DETAIL 420Q-1.
2. DURAN, INC. RELINER (OR APPROVED EQUAL) SHALL BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS.
3. V-NOTCH SHALL BE CUT IN INFLUENT PIPE INVERT.
4. PRESCOTT: MANHOLES SHALL BE LINED WITH SEWER SHIELD 150, SEWERGARD 210S, OR RAVEN 405.
5. PRESCOTT VALLEY: MANHOLES SHALL BE CONSTRUCTED WITH CONmicSHIELD PER APPLICABLE PVSD AND TOPV DESIGN AND CONSTRUCTION STANDARDS.
6. PRESCOTT VALLEY: MANHOLES SHALL BE COATED WITH INSECTA® CONTACT PESTICIDE OR APPROVED EQUAL.
7. WHEN CONNECTION IS FOR AN EXISTING MAIN, THE PIPE OPENING SHALL BE MADE BY CORE DRILLING MANHOLE IN ACCORDANCE WITH MAG SPECIFICATION SECTION 625.3.
8. FORCE MAIN DISCHARGE SHALL REQUIRE A HOOD ON THE DROP BOWL.



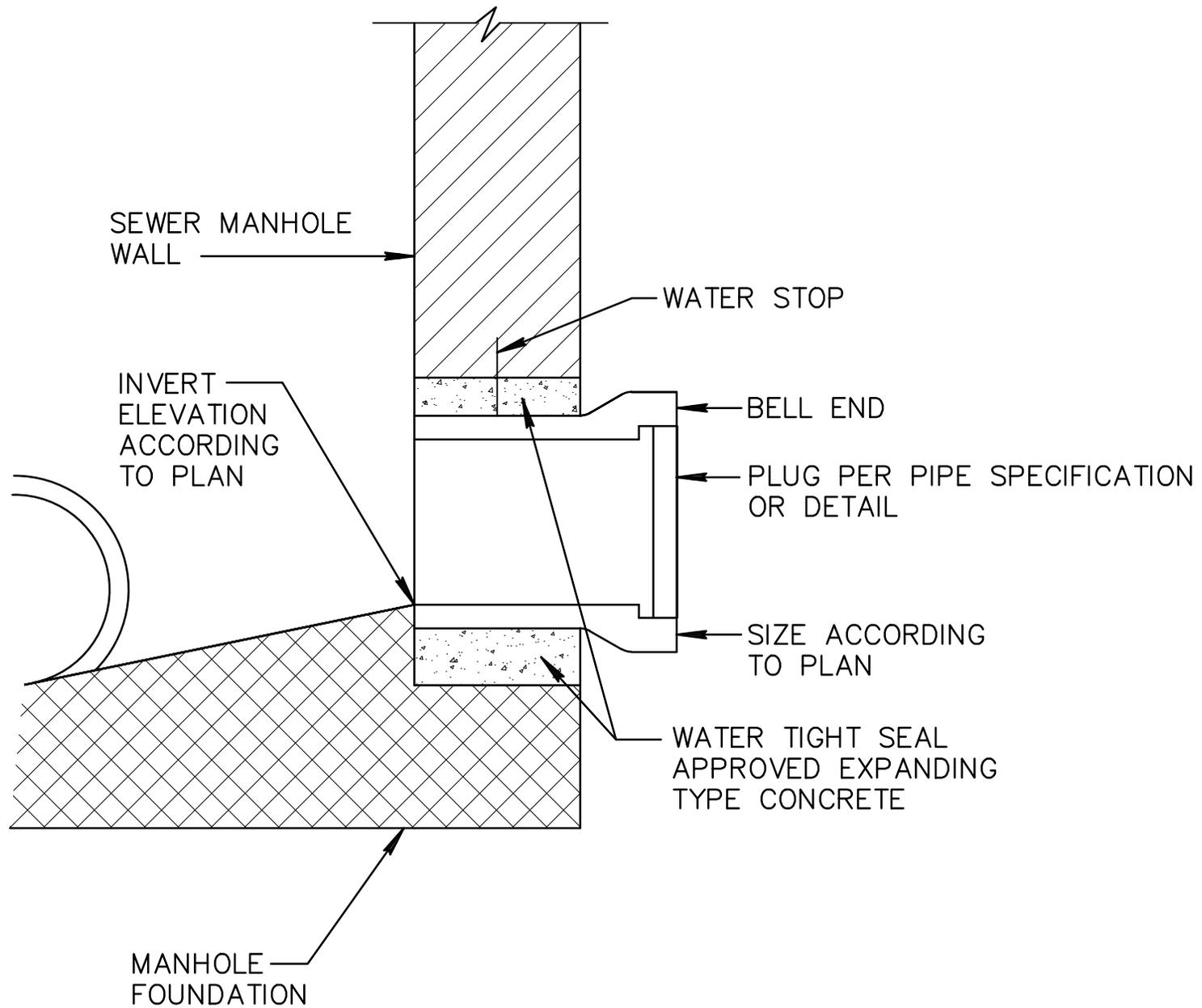
WHEN INSTALLING A BEAVER SLIDE THAT INTERCEPTS AN EXISTING SEWER AT A RIGHT ANGLE, THE CONNECTING INVERT OF THE BEAVER SLIDE IS TO INTERCEPT THE EXISTING SEWER SLIGHTLY ABOVE THE SPRINGLINE AS SHOWN. DISTANCE MEASURED FROM INVERT TO INVERT.

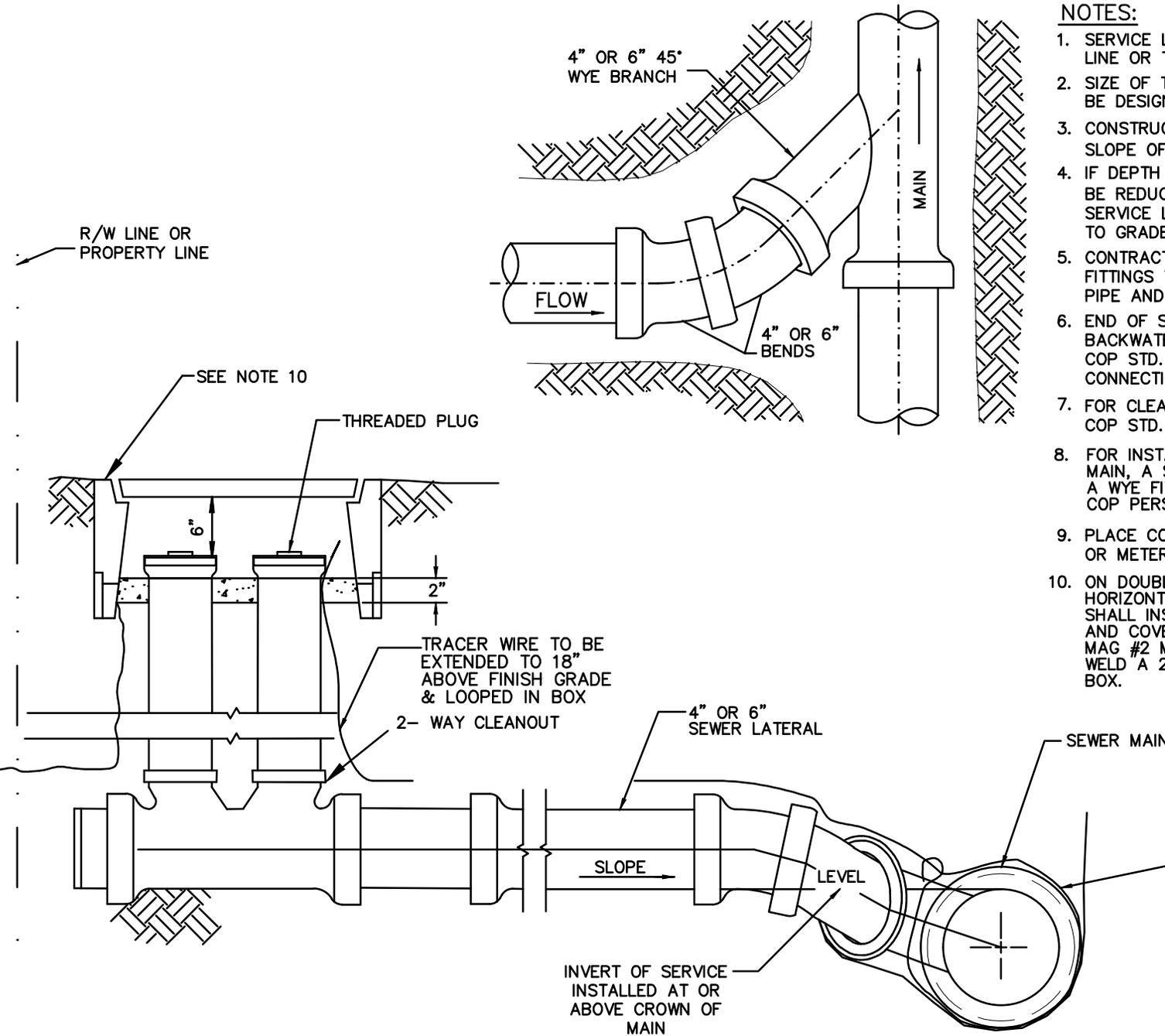


WHEN INSTALLING A BEAVER SLIDE WHERE THE FLOW IS STRAIGHT THROUGH THE MANHOLE, THE BEAVER SLIDE IS TO MATCH THE INVERT OF THE EXISTING LINE AND NOT TO EXTEND MORE THAN HALF-WAY THROUGH THE MANHOLE. DISTANCE MEASURED FROM INVERT TO INVERT. PREMIX MORTAR PROHIBITED



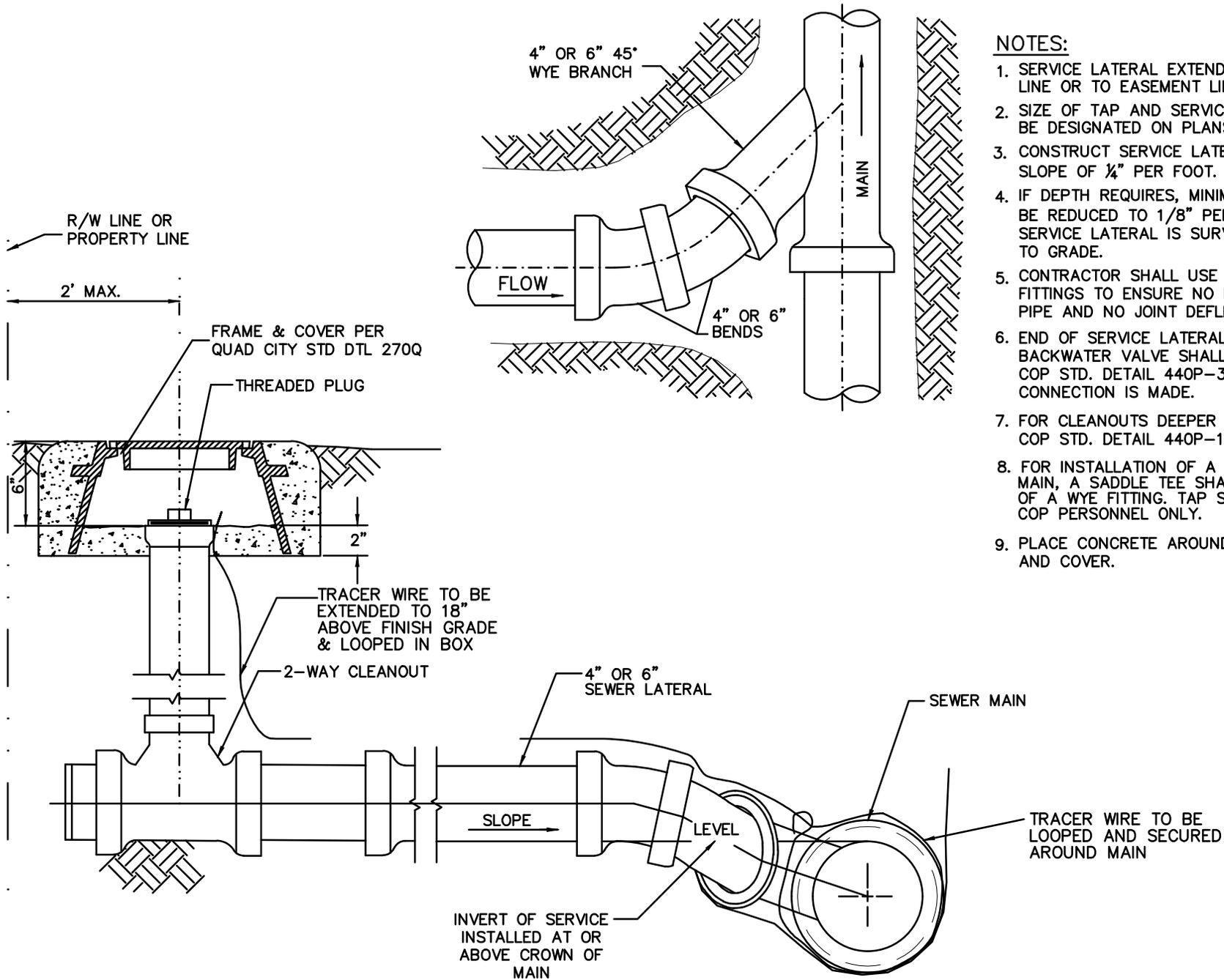
*MAX BEAVER SLIDE ELEVATION CHANGE 2' UNLESS APPROVED BY THE CITY/TOWN ENGINEER.





NOTES:

1. SERVICE LATERAL EXTENDS TO PROPERTY LINE OR TO EASEMENT LINE.
2. SIZE OF TAP AND SERVICE LATERAL SHALL BE DESIGNATED ON PLANS.
3. CONSTRUCT SERVICE LATERAL AT MINIMUM SLOPE OF $\frac{1}{4}$ " PER FOOT.
4. IF DEPTH REQUIRES, MINIMUM SLOPE CAN BE REDUCED TO $\frac{1}{8}$ " PER FOOT PROVIDED SERVICE LATERAL IS SURVEYED & STAKED TO GRADE.
5. CONTRACTOR SHALL USE THE APPROPRIATE FITTINGS TO ENSURE NO MISALIGNMENT OF PIPE AND NO JOINT DEFLECTION.
6. END OF SERVICE LATERAL TO BE SEALED. BACKWATER VALVE SHALL BE INSTALLED PER COP STD. DETAIL 440P-3 WHEN USER CONNECTION IS MADE.
7. FOR CLEANOUTS LESS THAN 24", REFER TO COP STD. DETAIL 440P-2.
8. FOR INSTALLATION OF A TAP TO AN EXISTING MAIN, A SADDLE TEE SHALL BE USED IN LIEU OF A WYE FITTING. TAPS SHALL BE PERFORMED BY COP PERSONNEL ONLY.
9. PLACE CONCRETE AROUND PIPE INSIDE FRAME OR METER BOX.
10. ON DOUBLE BARREL TWO-WAY CLEANOUT, IF HORIZONTAL SPACING ALLOWS, CONTRACTOR SHALL INSTALL FRAMES AND COVERS, IF FRAMES AND COVERS ARE NOT USED, A MINIMUM OF A MAG #2 METER BOX SHALL BE USED. STAMP OR WELD A 2" TALL LETTER "S" ON LID OF METER BOX.



NOTES:

1. SERVICE LATERAL EXTENDS TO PROPERTY LINE OR TO EASEMENT LINE.
2. SIZE OF TAP AND SERVICE LATERAL SHALL BE DESIGNATED ON PLANS.
3. CONSTRUCT SERVICE LATERAL AT MINIMUM SLOPE OF 1/4" PER FOOT.
4. IF DEPTH REQUIRES, MINIMUM SLOPE CAN BE REDUCED TO 1/8" PER FOOT PROVIDED SERVICE LATERAL IS SURVEYED & STAKED TO GRADE.
5. CONTRACTOR SHALL USE THE APPROPRIATE FITTINGS TO ENSURE NO MISALIGNMENT OF PIPE AND NO JOINT DEFLECTION.
6. END OF SERVICE LATERAL TO BE SEALED. BACKWATER VALVE SHALL BE INSTALLED PER COP STD. DETAIL 440P-3 WHEN USER CONNECTION IS MADE.
7. FOR CLEANOUTS DEEPER THAN 24", REFER TO COP STD. DETAIL 440P-1.
8. FOR INSTALLATION OF A TAP TO AN EXISTING MAIN, A SADDLE TEE SHALL BE USED IN LIEU OF A WYE FITTING. TAP SHALL BE PERFORMED BY COP PERSONNEL ONLY.
9. PLACE CONCRETE AROUND PIPE INSIDE FRAME AND COVER.

COP STANDARD DETAIL

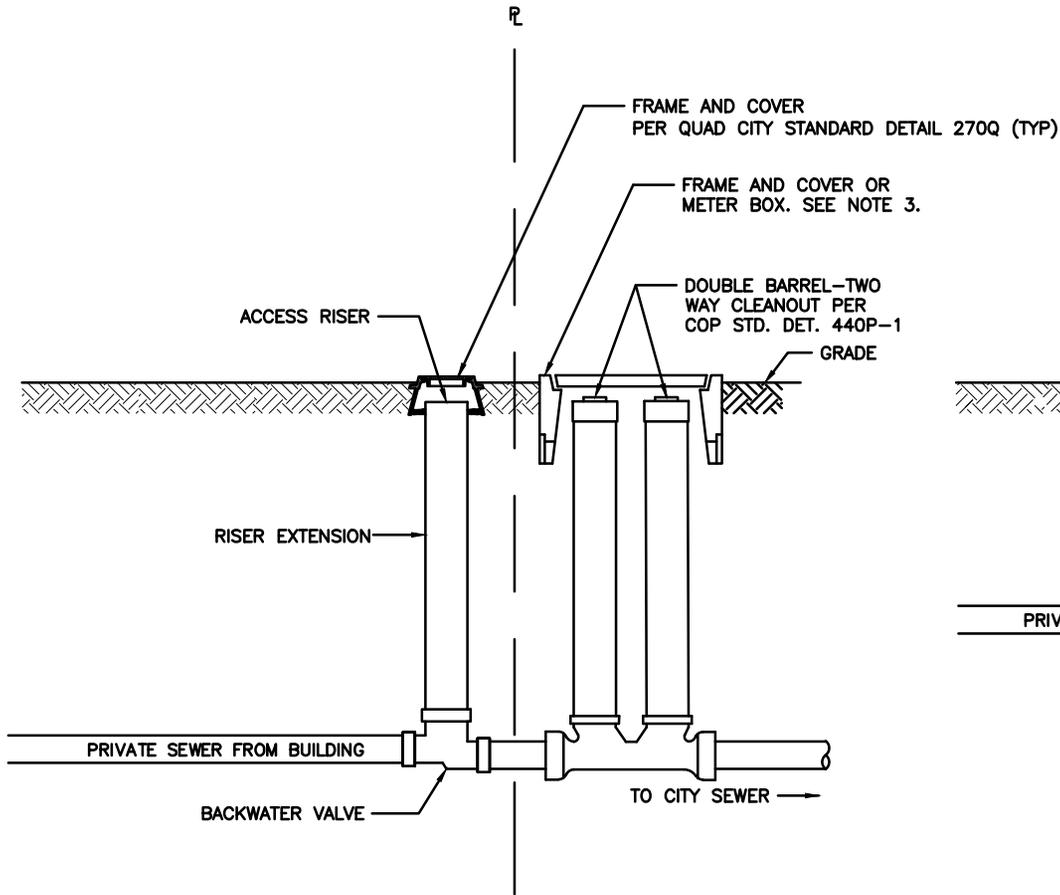
SEWER SERVICE LATERAL
2 FEET DEEP OR LESS

Charles Andrews
CITY ENGINEER

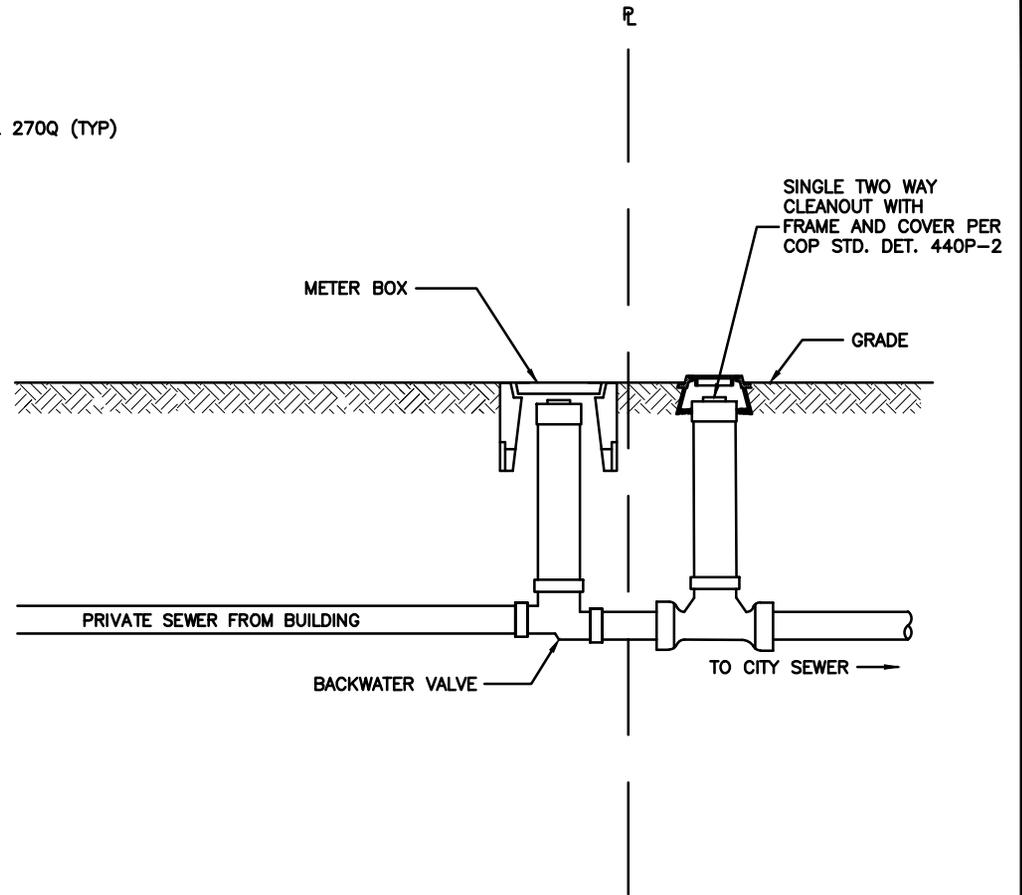
REVISED:
07/16

DETAIL No.
440P-2

GREATER THAN 24" DEEP



24" DEEP AND SHALLOWER



NOTES:

- 1) BACKWATER VALVE SHALL BE LOCATED BETWEEN THE BUILDING CLEAN-OUT AND PROPERTY LINE.
- 2) AN EXTENDABLE TYPE BACKWATER VALVE SHALL BE INSTALLED WHEN VALVES ARE DEEPER THAN 24".
- 3) ON DOUBLE BARREL TWO-WAY CLEANOUT, IF HORIZONTAL SPACING ALLOWS, CONTRACTOR SHALL INSTALL FRAMES AND COVERS. IF FRAMES AND COVERS ARE NOT USED, A MINIMUM OF A MAG #2 METER BOX SHALL BE USED. STAMP OR WELD A 2" TALL LETTER "S" ON LID OF METER BOX.

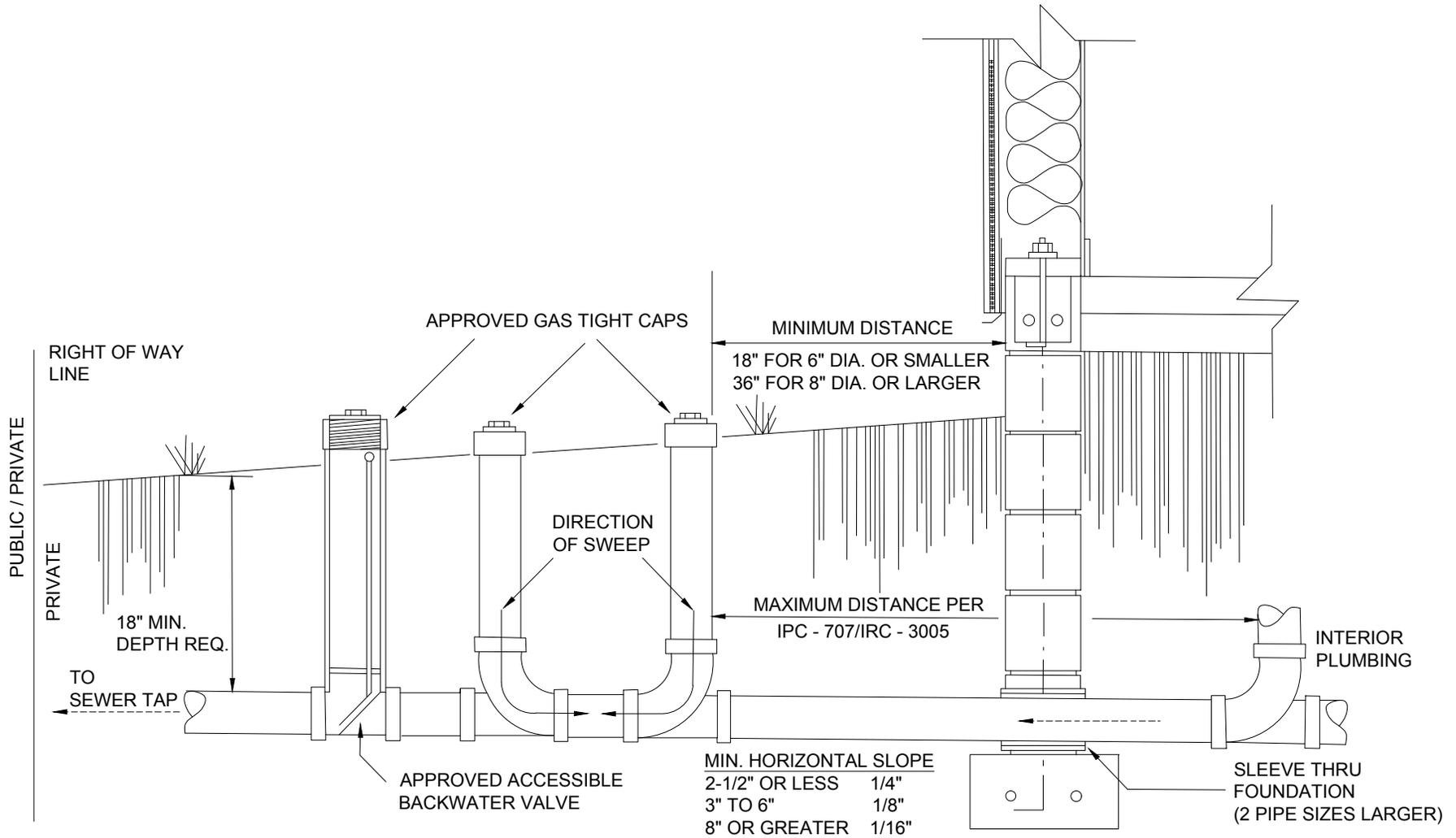
COP STANDARD DETAIL

BACKWATER VALVE

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
440P-3



**SEWER 2-WAY CLEANOUT &
BACKFLOW PREVENTION**

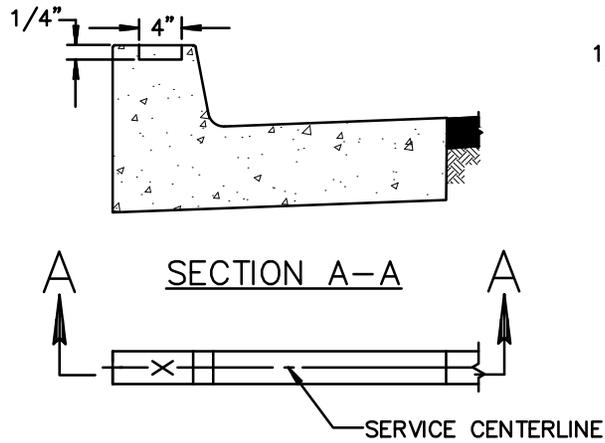
PV STANDARD DETAIL

SEWER CLEANOUT - DOUBLE

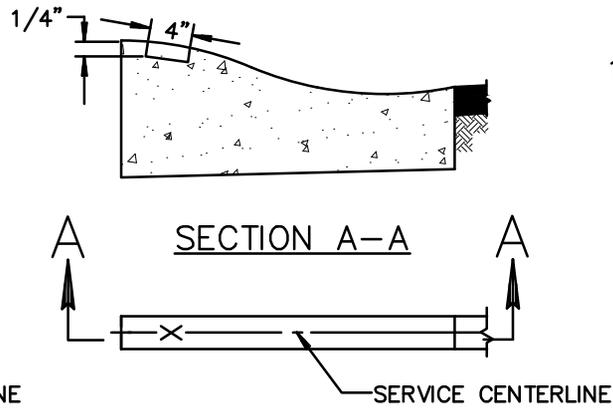
TOWN ENGINEER

REVISED:
07/16

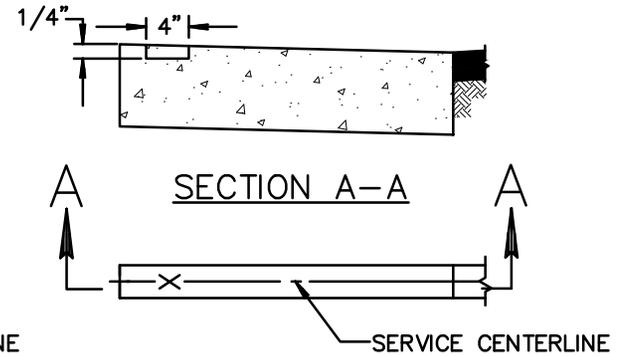
DETAIL No.
440PV



CURB STAMP TYPE A
CURB AND GUTTER



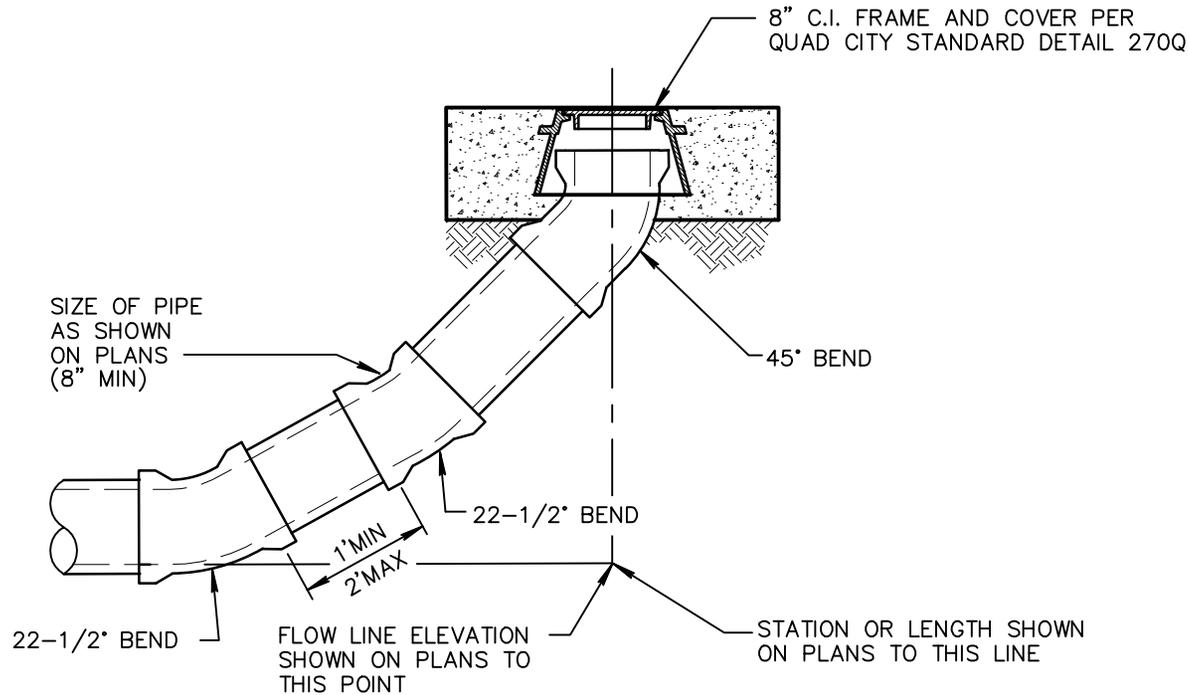
CURB STAMP TYPE C OR D
CURB AND GUTTER



CURB STAMP TYPE B
CURB AND GUTTER

NOTES:

1. STAMP TOP OF CURB WITH 4" TALL BY 1/4" DEEP "W" OR "S" TO DESIGNATE SERVICE LINE CROSSING.



NOTES:

1. NO SERVICE TAPS SHALL BE LOCATED CLOSER THAN 4 FEET DOWNSTREAM OF FIRST BEND.
2. ALL JOINTS ARE TO BE WATER TIGHT.
3. CLEAN OUTS SHALL NOT BE PLACED IN VALLEY GUTTERS, SPANDRELS, CURB & GUTTERS, CATCH BASINS, OR OTHER DRAINAGE STRUCTURES.
4. CLEAN OUTS INSTALLED OFF SITE REQUIRE CARSONITE MARKERS RUNNING PARALLEL TO THE LINE, AS DIRECTED BY AGENCY ENGINEER.
5. BEDDING AND SHADING PER QUAD CITY STANDARD DETAIL 200Q.
6. PLACE CONCRETE AROUND 45° BEND INSIDE FRAME AND COVER.

Sewer Manhole Vacuum Testing

1. Vacuum testing is required for all new or replacement sewer manholes in accordance with the American Society for Testing and Materials ASTM C-1244, Arizona Administrative Code (AAC), and instruction per this document.
2. **Exfiltration testing (watertightness) or holiday testing is not permitted.**
3. Testing must be conducted after manhole has been adjusted to final grade, at top of cone, and prior to backfill.
4. Manholes must pass vacuum test prior to any backfill taking place.
5. The developer / contractor shall ensure that testing personnel are qualified and/or directly supervised by persons competent to perform the vacuum tests.
6. Test results shall be certified by a Civil Engineer, registered in the State of Arizona. The certification report shall be provided to the Town Inspector prior to punch list inspection performed by the Town.
7. The Town Inspector shall be notified when testing will be performed and by whom at least 48 hours in advance of test. The Inspector, at his/her option, may witness testing to verify procedures are being followed correctly.
8. The vacuum gage must have a calibration sticker with date of certification. Vacuum test must be performed within one year of calibration certification date. Calibration certification of the gage must be performed a lab / firm accredited by the American Association for Laboratory Accreditation (A2LA) or Town approved equal.
9. The Manhole Vacuum Testing Certification Field Report per this document is the only format acceptable by the Town for manhole vacuum testing.
10. Air vacuum tests must be in accordance with the following procedures and Table 1. The manhole shall be placed under a vacuum of **ten (10) inches of mercury (Hg)**, the vacuum line closed, and the pump shut off. The vacuum level shall not be allowed to drop by more than one (1) inch of mercury in less than the noted times.

Table 1 - Test Criteria

| Diameter | Depth | Time |
|-----------|---------------------------------|------------|
| 48 Inches | 0 feet to 10 feet | 60 Seconds |
| 48 Inches | Greater than 10 feet to 15 feet | 70 Seconds |
| 48 Inches | Greater than 15 Feet | 75 Seconds |

| Diameter | Depth | Time |
|-----------|---------------------------------|-------------|
| 60 Inches | 0 feet to 10 feet | 75 Seconds |
| 60 Inches | Greater than 10 feet to 15 feet | 90 Seconds |
| 60 Inches | Greater than 15 Feet | 105 Seconds |

Sewer Manhole Vacuum Testing Certification Report

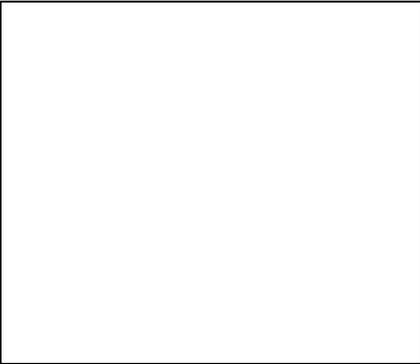
Date _____
 Project Name _____
 Phase if Available _____
 CIP Number if Available _____
 Permit Number _____
 Address / Location _____
 Developer / Contactor Name _____
 Gage Calibration Date _____

The following vacuum tests were performed under my supervision or as noted, and "To the best of my knowledge and belief", are found to be in conformance with Town Manhole Testing Requirements."

Civil Engineer Registered in the State of Arizona
 Name _____
 Signature _____
 Date _____

If witnessed by Town Inspector
 Name _____
 Signature _____
 Date _____

Stamp/Seal



| Manhole # | Diameter (Inches) | Depth (Feet) | State Material Concrete Precast Concrete Poured Other | Pass (Check) | Fail (Check) |
|-----------|-------------------|--------------|--|-----------------|-----------------|
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | |

TOWN OF PRESCOTT VALLEY DESIGN AND CONSTRUCTION STANDARDS

SANITARY SEWER AND STORM DRAIN CLEANING

Statement of Purpose

All new, repaired and/or modified sanitary sewer and storm drainage infrastructure is subject to cleaning and follow-up TV inspection prior to approval of as-built drawings and final acceptance by the Town of Prescott Valley (Town). This specification outlines Town requirements for cleaning protocol.

PART 1 - GENERAL

1.01 BACKGROUND SUMMARY

- A. Section includes sanitary sewer and storm drainage (SSSD) line cleaning, manhole cleaning, and internal obstruction removal.

1.02 SYSTEM DESCRIPTION

- A. Cleaning shall remove sediment, rocks, debris, roots, grease accumulations and obstructions from length of sewer and manholes to be lined.
- B. Cleaning of sewer and manhole walls in vicinity of lining shall remove grease, scale, encrustation and loose mortar so that no foreign intrusion shall cause imperfections in lining (e.g. bumps, folds, dimples).
- C. Sewer cleaning methods shall be washing with high pressure water or other as approved by the Town of Prescott Valley (Town).

1.03 SUBMITTALS

Submit letter that identifies methods that will be used to remove sediment, debris, grease, scale, encrustations, loose concrete, and roots throughout section of sewer to be cleaned. The letter shall include the following:

- A. Detailed description of cleaning process.
- B. Schedule of activities.
- C. List of the actions to mitigate impact to Town during cleaning operation.

PART 2 - PRODUCTS

2.01 MATERIALS

Do not use chemicals without written approval of the Town.

Continued Next Page

2.02 EQUIPMENT

High-Velocity Hydraulic (Hydro-Cleaning) Equipment: Equipment shall be capable of removing dirt, grease rocks, sand, roots, and other materials and obstructions from sewer lines and manholes.

- A. Nozzles shall be capable of producing scouring action from 15 to 45 degrees in all size lines designated to be cleaned.
- B. Equipment shall carry its own water tank, auxiliary engines, and high pressure water pump.
- C. Combination Unit Pump: Capable of pumping at a minimum of 50 gallons per minute (gpm) up to 80 gpm at a minimum of 2,000 pounds per square inch (psi) measured at beginning of hose reel.
- D. Water Pump: Able to run a minimum of 2,000 psi discharge pressure to the nozzle while pulling full vacuum.

2.03 WATER

- A. When water from fire hydrants is necessary, the contractor will be required to apply for a Hydrant Meter with the Town.
- B. Provide temporary piping, valves, certified reduced pressure zone assembly, and other required items for handling potable water and wastewater.

PART 3 - EXECUTION

3.01 EXAMINATION

Contractor shall be aware of flow conditions, and be able to identify potential access problems to sewer access points.

3.02 APPLICATION

- A. Line Cleaning: Clean designated SSSD lines using approved methods and equipment.
- B. Manhole Cleaning: Include entire manhole interior, including manhole benches and walls. Incorporate into line cleaning operation by scouring walls with high velocity nozzle after pipe segment cleaning operation is complete.
- C. Removal of Debris, Sewage and Solids: Remove and dispose. Do not discharge to other Town infrastructure such as manholes, ditches, catch basins, storm drains, etc. Disposal costs and regulatory compliance is the responsibility of the Contractor.

3.03 FIELD QUALITY CONTROL

- A. Inspection: Provide television inspection per applicable PVSD.

3.04 CLEANING

- A. Keep premises free from accumulations of waste materials, rubbish and other debris resulting from work.
- B. Remove waste materials, rubbish, and debris from and about premises.
- C. Remove tools, construction equipment and machinery, and surplus materials.
- D. Restore to original condition portions of site not designated for alterations by any Contract Documents.

TOWN OF PRESCOTT VALLEY DESIGN AND CONSTRUCTION STANDARDS

TELEVISION INSPECTION

Statement of Purpose

All new public sewer and storm construction is subject to Television (TV) inspection prior to approval of as-built drawings and final acceptance by the Town of Prescott Valley (Town). Said requirement is based on Arizona Department of Environmental Quality (ADEQ) regulations. The Town also extends this requirement to: 1) all sewer and storm drainage infrastructure that has undergone repair or modification and 2) all sewer and storm drainage infrastructure that is situated in areas where nearby excavation is occurring or has occurred. Please note that the requirement for TV inspection of storm drainage only pertains to pipes 24 inches or larger.

PART 1 - GENERAL

1.01 BACKGROUND SUMMARY

- A. This section includes TV inspection of sanitary sewer and storm drainage (SSSD) infrastructure.
 - 1. Inspect SSSD interiors using color, closed-circuit television (CCTV) camera. Document inspection on DVD with audio location and date information, DVD title information, and continuous tape counter. Provide an electronic version in PDF and a hard copy of inspection reports.
 - 2. Additional TV inspections may be required at other stages of operation if necessary to meet this specification.
- B. Related Sections: Refer to the following section for related work:
 - 1. "Sanitary Sewer and Storm Drainage Cleaning" per applicable PVSD.

1.02 SEQUENCE OF WORK

- 1. TV inspection must be performed prior to as-built acceptance.
- 2. TV inspection shall occur no earlier than seven (7) days after completion of backfill compaction and after mirror and leakage testing.
- 3. Clean SSSD lines and manholes per applicable PVSD prior to TV inspection. All pipes and manholes shall be free from rocks, mud, construction debris, or any other objects which hinder or limit the inspection.
- 4. TV inspection and cleaning shall be coordinated with Town staff a minimum of two (2) Town workdays in advance.
- 5. TV inspection shall occur prior to any final grading or asphalt / concrete work in the Right of Way.
- 6. On the day prior to TV inspection, the contractor shall supply adequate water to each section of pipe such that water runs through each downstream manhole. Water is necessary in order to provide the necessary visual indicators of sag (SAG) in the pipe / manholes and to properly mirror inspect the line. Please note that SAG in a pipe is also referred to in the industry as a "belly".

Continued Next Page

1.03 SUBMITTALS

- A. Quality Assurance: Submit one example DVD of previous sewer inspection work that shows operational and structural defects in SSSD, complete with audio commentary and inspection report(s). Prior to submittal finalize the DVD to prevent rerecording.
 - 1. DVD and inspection reports will be reviewed to determine if quality of CCTV image is acceptable, if defects were properly identified and documentation was in accordance to the specifications outlined in this PVSD.
 - 2. Modify equipment and/or inspection procedures to achieve report material of acceptable quality.
 - 3. Do not commence work prior to approval of report material quality by Town inspection staff.
- B. Inspection Reports: Unless otherwise indicated, submit inspection reports to the Town that include the following as a minimum:
 - 1. Project title
 - 2. Name of: Town of Prescott Valley
 - 3. Street name or general location
 - 4. Time of day
 - 5. Manhole to manhole pipe section with Town Geographical Information System (GIS) designations. Reference GIS manhole numbers in report.
 - 6. Pipe segment length
 - 7. Pipe material
 - 8. Line size
 - 9. Compass direction of viewing
 - 10. Direction of camera's travel
 - 11. Pipe depth
 - 12. Operator name and certification I.D.
 - 13. Direction of normal water flow.
 - 14. Tape counter reading at beginning and end of each manhole to manhole pipe segment.
 - 15. SAG documentation. A mechanical gauge will be mounted in front of the camera to show the depth of any standing water due to SAG in the invert. The gauge shall clearly indicate markings of 1/4 " increments. Corrective action requirements for SAG are described per Table 1.
- C. DVDs: Prior to submittal, finalize the DVD to prevent re-recording. DVDs must be readable with standard Windows Media Player viewing software.
- D. Maintain copy of all inspection documentation (DVDs, databases, and reports) for duration of work and warranty period.
- E. The Contractor shall bear all costs incurred in correcting deficiencies found during the TV inspection and shall bear the cost of follow-up TV verification for repairs. Only the repaired reaches shall require follow-up TV inspection, mirror and leakage testing.
- F. The Contractor shall provide a TV warranty inspection no sooner than 22 months or later than 23 months after initial acceptance. Said requirement is based on two year warranty for construction projects. The Contractor is responsible for all warranty repairs and TV re-inspection of repaired areas until the Town deems the repairs acceptable.

Continued Next Page

Table 1 - Corrective Action Requirements for SAG

| Description | Observed SAG | Correction Action Required |
|----------------------------------|--|---|
| 8" to 12" Dia. Pipe | Less than or equal to ½" | None |
| 8" to 12" Dia. Pipe | Greater than ½" but less than or equal to 1" | Yes if longer than 10' or more than 3 occurrences in 100' |
| 12" to 24" Dia. Pipe | Less than or equal to 1" | None |
| 12" to 24" Dia. Pipe | Greater than 1" but less than or equal to 1 ½" | Yes if longer than 20' or more than 3 occurrence in 100' |
| Greater than 24" Dia. Pipe | Greater than 1 ½" | Yes |
| Pipe entering or exiting manhole | Any | Yes |

PART 2 - PRODUCTS

2.01 MATERIALS AND EQUIPMENT

- A. DVD: 120 minute minimum, high-quality color, type DVD-R, DVD-RW, or DVD+R
 - 1. Audio portion of composite DVD shall be sufficiently free from electrical interference and background noise to provide complete intelligibility of oral report.
 - 2. Store in upright position with temperature range of 45 to 80 degrees F (7 to 27 degrees C) in an appropriate CD or DVD case to prevent scratches.
 - 3. Identify each disk with tape labels showing Town's name, Project Name, Street name or general location, Contractor's name, Operator name and each manhole-to-manhole pipe segment of sewer line represented on DVD. Contractor shall provide an index or table of contents if more than one segment is on the disk.
- B. Television Inspection Camera(s): Equipped with rotating head, capable of 90-degree rotation from horizontal and 360-degree rotation about its centerline.
 - 1. Minimum Camera Resolution: 400 vertical lines and 460 horizontal lines.
 - 2. Camera Lens: Not less than 140 degree viewing angle, with automatic or remote focus and iris controls.
 - 3. Focal Distance: Adjustable through range of 2 inches to infinity.
 - 4. Camera(s) shall be intrinsically safe and operative in 100 percent humidity conditions or flammable / explosive conditions.
 - 5. Lighting Intensity: Remote-controlled and adjusted to minimize reflective glare.
 - 6. Lighting and Camera Quality: Provide clear, in-focus picture of entire inside periphery of sewer.
- C. Footage Counter: Measures distance traveled by camera in sewer, accurate to plus or minus 2 feet (0.6 m) in 1,000 feet (305 m). Onscreen text must be clearly legible.
- D. DVD Titling: Each segment shown on the DVD should have its own Chapter titled with the beginning and end point of the pipe segment.

Continued Next Page

PART 3 - EXECUTION

3.01 INSPECTION REQUIREMENTS

- A. Access: Town staff shall have access to observe televised operations at all times.
- B. DVD Commentary: Record the following information on audio track of inspection DVD: narrative of location, direction of view, manhole numbers, pipe diameter, material, date, time of inspection, and location of laterals and other key features.
 - 1. DVD shall visually display this information at beginning and end of each manhole-to-manhole pipe segment.
 - 2. DVD between manholes shall visually display length in feet from starting point of given segment.
- C. SSSD Identification: DVD and inspection documentation shall include line and manhole identifiers shown on Drawings provided by Town.
- D. Image Perspective: Camera image shall be down center axis of pipe when camera is in motion.
 - 1. Provide 360-degree sweep of pipe interior at joints and points of interest, to more fully document existing condition of sewer.
 - A. Points of interest include, but are not limited to the following: defects, cracks, voids, joints, laterals, encrustations, mineral deposits, debris, SAG, sediment, and any location determined not to be clean or part of an improper liner installation, and defects in liner that include, but are not limited to bumps, folds, tears, and dimples.
 - 2. Contractor shall provide still photos of all call-outs or deficiencies as part of the inspection report.
 - 3. Cabling system employed to transport camera and transmit its signal shall not obstruct camera's view.
- E. SSSD Reach Length: Physically measure and record length of each sewer segment of pipe from centerline of its terminal manholes.
- F. Inspection Rate: Camera shall be pulled through sewer in either direction, but both inspections are to be in same direction. Maximum rate of travel shall be 30 feet per minute when recording.

3.04 FIELD QUALITY CONTROL

- A. Town and Engineer of Record will review DVD video and reports to ensure compliance with requirements listed in this specification.
- B. If SSSD line is not adequately clean, it shall be re-cleaned and TV inspected by Contractor at no additional cost to the Town.
- C. All TV pipe inspection operators must be certified by the Pipeline Assessment Certification Program (PACP) or the National Association of Sewer Service Companies (NASSCO).

ConmicShield® Additive

Specifications for Sanitary Sewer Structures Utilizing
Precast Concrete, Cast-In-Place and Shotcrete

Precast Concrete Manholes / Sewer Structures

- Antimicrobial additive, ConmicShield®, shall be used to render the concrete uninhabitable for bacteria growth.
- The liquid antibacterial additive shall be an EPA registered material and the registration number shall be submitted for approval prior to use in the project.
- The amount to be used shall be as recommended by the manufacturer of the antibacterial additive. This amount shall be included in the total water content of the concrete mix design.
- The additive shall be added into the concrete mix water to insure even distribution of the additive throughout the concrete mixture.
- The antibacterial additive shall have successfully demonstrated prevention of MIC in sanitary sewers for ten or more years.
- The antibacterial shall be used by factory certified precast concrete plants.

Acceptance: Acceptance shall be a letter of certification from the precaster to the Town and Engineer of Record (EOR) stating that the correct amount and correct mixing procedure were followed for all antimicrobial concrete.

Quality Assurance: The precaster shall retain two labeled specimens from each production run. One set shall be retained by the precaster and the other set shall be sent to CONSHIELD Technologies, Inc. or independent laboratory as directed by the EOR for verification on a random or as needed basis.

Field Repairs: Field repairs to the precast concrete shall be made using ConmicShield® Joint Set Grout pre-portioned and factory packaged that requires the addition of no other components. This repair grout may be used for filling joints, lift holes, damaged areas, benches and similar.

Product Surface Marking: The name of the antimicrobial additive shall be plainly stenciled on the exterior and interior of each piece. The contractor shall spray CS IDENTIFIER™, green colored sealer, onto the interior surface after installing.

Cast-in-Place Manholes / Sewer Structures

- Antimicrobial additive, ConmicShield®, shall be used to render the concrete uninhabitable for bacteria growth.
- The liquid antibacterial additive shall be an EPA registered material and the registration number shall be submitted for approval prior to use in the project.
- The amount to be used shall be as recommended by the manufacturer of the antibacterial additive. This amount shall be included in the total water content of the concrete mix design.
- The additive shall be added into the concrete mix water to insure even distribution of the additive throughout the concrete mixture.
- The ready-mix supplier shall submit a letter of certification to the Town and EOR stating that the correct amount and correct mixing procedure were followed for all antimicrobial concrete.
- The antibacterial additive shall have successfully demonstrated prevention of MIC in sanitary sewers for ten or more years.
- The antibacterial shall be used by factory certified plants and contractors.
- After the concrete takes initial set, ConmicShield® color identifier-indicator (CS Identifier®) shall be applied to the interior surface.

Acceptance: Acceptance shall be a letter of certification from the concrete provider to the Town and EOR stating that the correct amount and correct mixing procedure were followed for all antimicrobial concrete.

Quality Assurance: The concrete producer shall retain two labeled specimens from each production run. One set shall be retained by the concrete producer and the other set shall be sent to CONSHIELD Technologies, Inc. or independent laboratory as directed by the engineer for verification on a random or as needed basis.

Field Repairs: Field repairs to the precast concrete shall be made using ConmicShield® Joint Set Grout pre-portioned and factory packaged that requires the addition of no other components. This repair grout may be used for filling joints, lift holes, damaged areas, benches and similar.

Continued Next Page

PV STANDARD DETAIL

CONSHIELD ADDITIVE

TOWN ENGINEER

REVISED:
07/16

DETAIL No.
446PV

Shotcrete for Sewer Structures

- Antimicrobial additive, ConmicShield® shall be used to render the shotcrete uninhabitable for bacteria growth.
- The liquid antibacterial additive shall be an EPA registered material and the registration number shall be submitted for approval prior to use in the project.
- The amount to be used shall be as recommended by the manufacturer of the antibacterial additive. This amount shall be included in the total water content of the shotcrete mix design for wet mix shotcrete.
- For both wet and dry shotcrete, the additive shall be added into the shotcrete mix water to ensure even distribution of the additive throughout the shotcrete.
- The contractor performing the shotcrete work shall submit a letter of certification to the Town and EOR stating that the correct amount and correct mixing procedure was followed for all antimicrobial shotcrete.
- The antibacterial additive shall have successfully demonstrated prevention of MIC in sanitary sewers for ten or more years.
- The antibacterial shall be used by factory certified applicators.
- For rehabilitation of existing sewer structures, ConmicShield® solution shall be spray applied to the cleaned and prepared interior surface prior to applying shotcrete to ensure removal of all residual bacteria.
- ConmicShield® color identifier-indicator (CS IDentifier ®) shall be spray applied to the surface after the initial set.

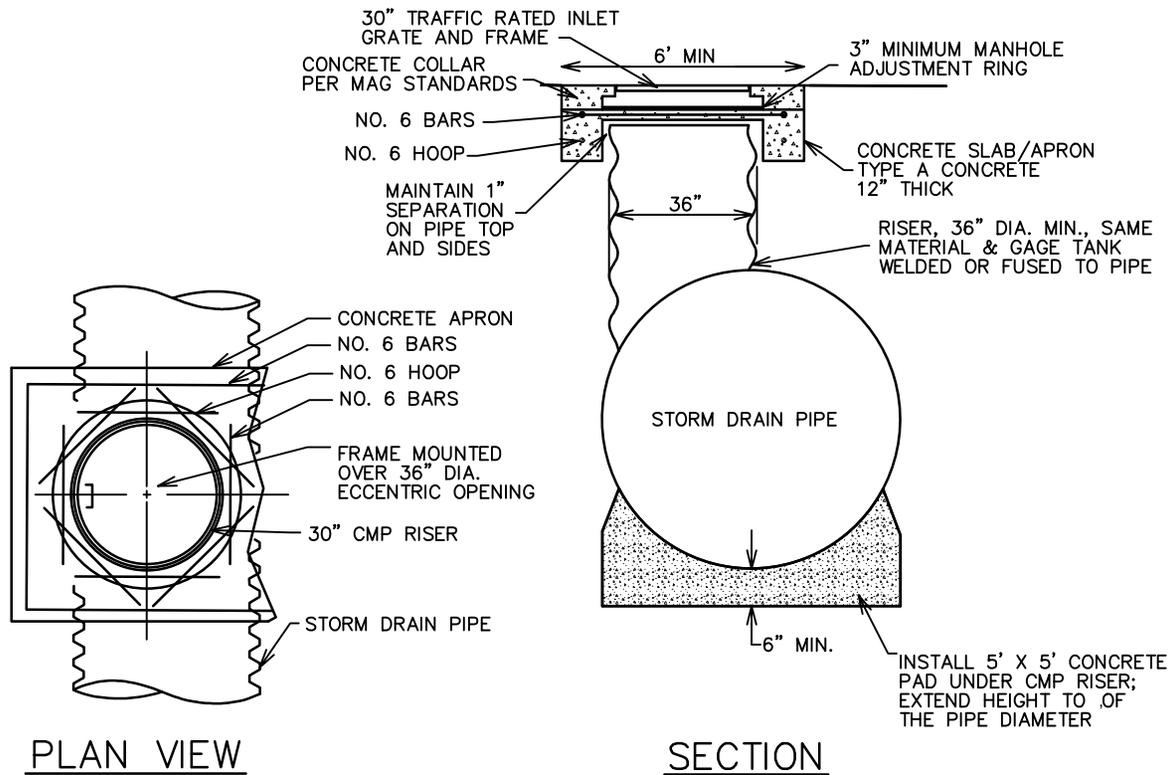
Acceptance: Acceptance shall be a letter of certification from the concrete provider to the Town and EOR stating that the correct amount and correct mixing procedure were followed for all antimicrobial concrete.

Quality Assurance: The contractor shall retain two labeled specimens from each batch. One set shall be retained by the contractor and the other set shall be sent to CONSHIELD Technologies, Inc. or independent laboratory as directed by the EOR for verification on a random or as needed basis.

Field Repairs: Field repairs to the precast concrete shall be made using ConmicShield® Joint Set Grout pre-portioned and factory packaged that requires the addition of no other components. This repair grout may be used for filling joints, lift holes, damaged areas, benches and similar.

ConMICShield® liquid antimicrobial admixture shall be obtained from ConShield Technologies, Inc. Active ingredient EPA Registration 75174-2-47000.

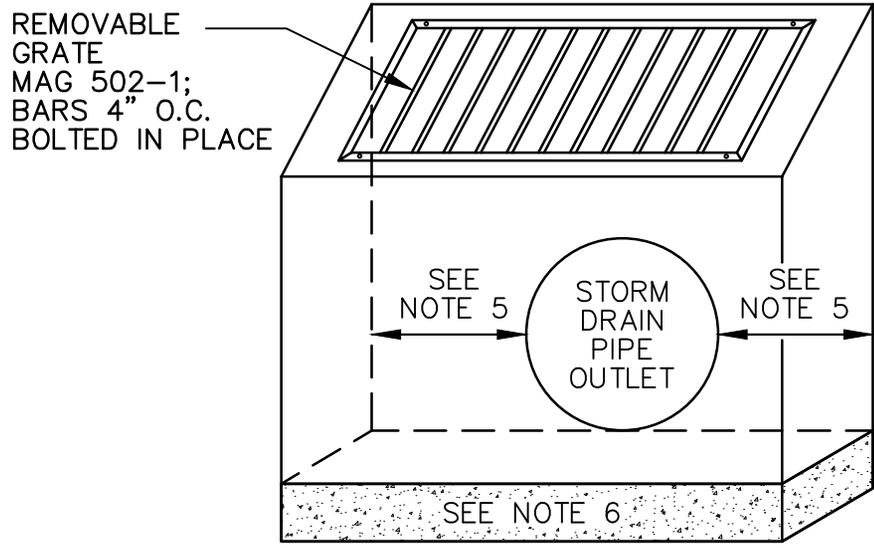
ConShield Technologies Inc.
541 Tenth Street NW #233
Atlanta, GA 30318-5713
Call Toll Free: 877-543-2094
Fax: 770-438-2131
<http://www.conshield.com> info@conshield.com



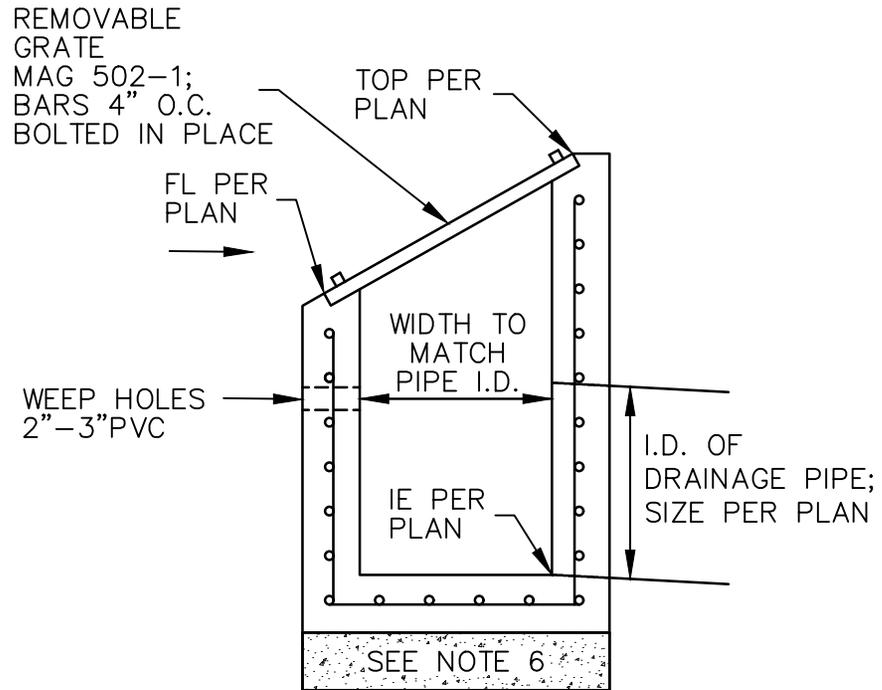
- NOTES:
1. USE ADJUSTING BLOCKS AS REQUIRED TO BRING FRAME TO GRADE
 2. ALL MATERIALS MUST BE CORROSION RESISTANT.
 3. PREFABRICATED TEES ARE PERMITTED INSTEAD OF CMP ACCESS RISERS

CMP MANHOLE ACCESS RISER

| | | | | |
|--------------------|--------------------------|---------------|-------------------|-----------------------|
| PV STANDARD DETAIL | CMP MANHOLE ACCESS RISER | TOWN ENGINEER | REVISED: 07/16 | DETAIL No. 523PV-3 |
|--------------------|--------------------------|---------------|-------------------|-----------------------|



FRONT VIEW



SIDE VIEW

NOTES:

1. HIGH POINT OF HEADWALL SHALL NOT PROJECT MORE THAN 3" ABOVE GROUND.
2. ALL CONCRETE SHALL BE CLASS A.
3. ALL REBAR SHALL BE NO. 4, 12" O.C. AND 3" CLEAR TO INSIDE OF WALLS AND FLOOR.
4. ALL CONCRETE THICKNESS SHALL BE 6".
5. 12" FROM OUTSIDE OF PIPE TO OUTSIDE OF CONCRETE.
6. MINIMUM 6" A.B. UNDER CONCRETE, COMPACTED TO 95%.

| | | | | |
|--------------------|----------------------|---------------|-------------------|-----------------------|
| PV STANDARD DETAIL | INLET DROP STRUCTURE | TOWN ENGINEER | REVISED: 07/16 | DETAIL No. 523PV-6 |
|--------------------|----------------------|---------------|-------------------|-----------------------|

Hydroseed Requirements

Hydroseeding consists of scarifying the surface of disturbed areas and applying a mixture of fiber, seed, fertilizer, and stabilizing emulsion.

Contractor / developer shall utilize the seed mix noted per Table 1 for restoration unless otherwise approved by the Town. The Town Inspector shall be notified when hydroseeding will be performed and by whom at least 48 hours in advance. The Inspector, at his/her option, may witness application to verify procedures are being followed correctly.

The developer / contractor is responsible for providing certification of seed mix to the Town Inspector during witness activities or if no witness occurs, prior to punch list inspection performed by the Town. Stabilizing emulsion and fertilizer mix shall be determined by the contractor to insure germination at the rates specified by the Town.

Acceptance criteria shall be 70% seed germination within a one year period. Areas not evidencing a minimum of 70% germination within the one year period, as determined by the Town, shall be re-seeded at no cost to the Town. When acceptable final stabilization has been determined, if required, a Notice of Termination shall be filed by the contractor.

Table 1 - Southwest Native Grass Seed Mix

| Botanical Name | Common Name |
|------------------------|---------------------|
| Bouteloua curtipendula | Sideoats Grama |
| Bouteloua gracilis | Blue Grama |
| Setaria vulpiseta | Plains Bristlegrass |
| Sporobolus airoides | Alkali Sacaton |
| Achnatherum hymenoides | Indian Ricegrass |
| Muhlenbergia wrightii | Spike Muhly |
| Sporobolus cryptandrus | Sand Dropseed |

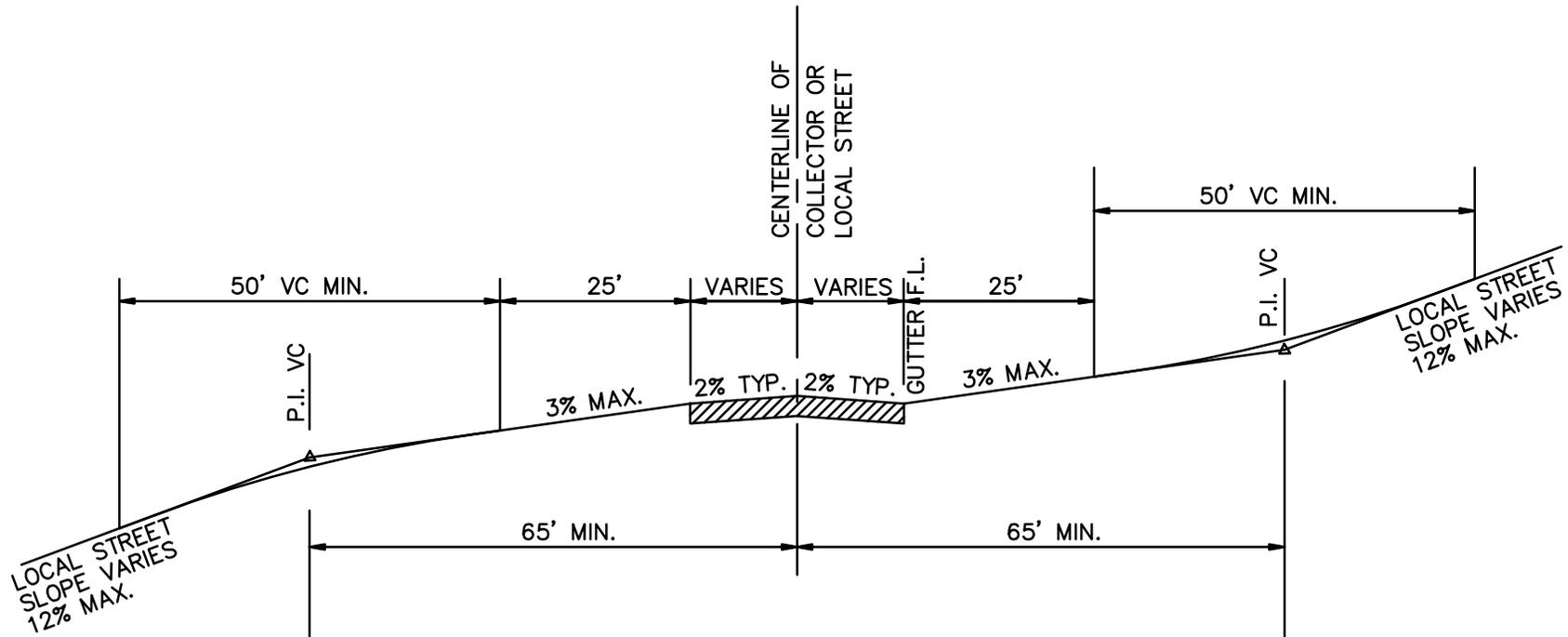
The seed mix shall be dispersed at a minimum of 8 pounds per acre. The Southwest Native Grass Seed Mix is available from a number of vendors. Two of them, current per development of this PVSD, are as follows:

Outside Pride

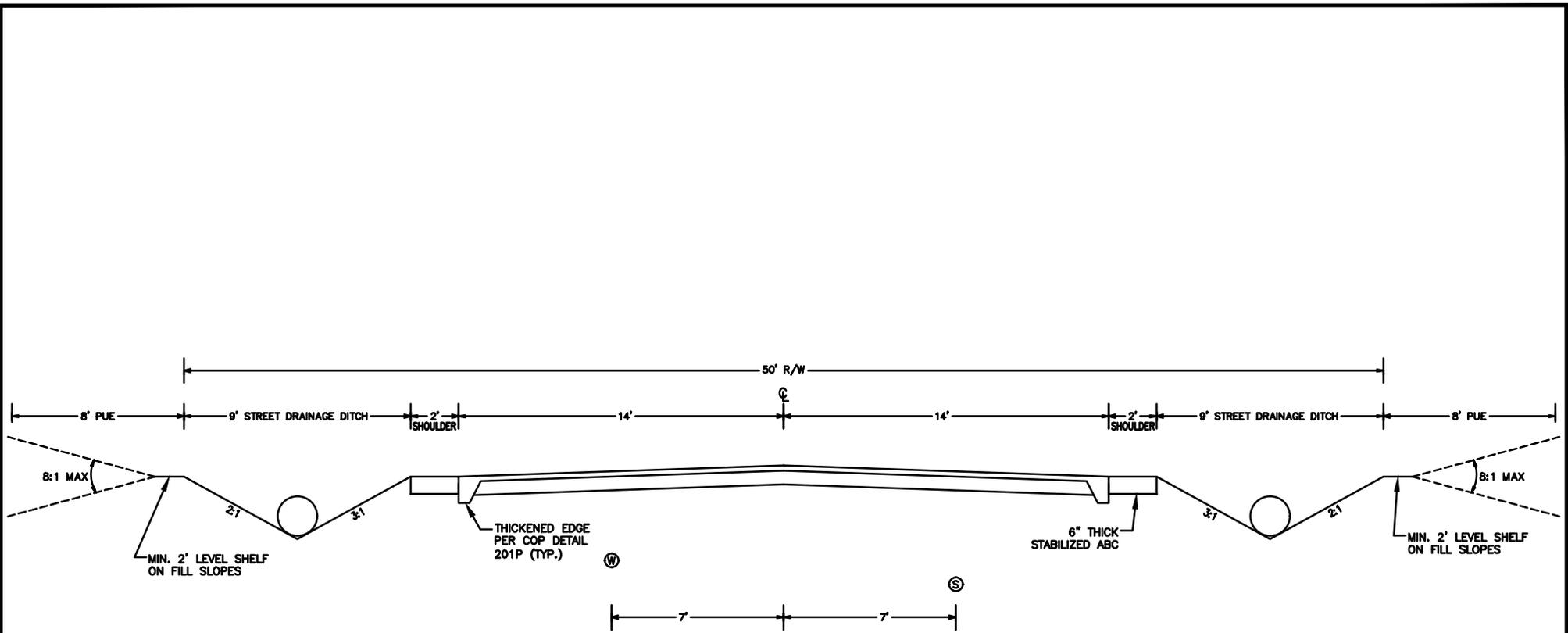
<http://www.outsidepride.com>

Seedland

<http://www.seedland.com>

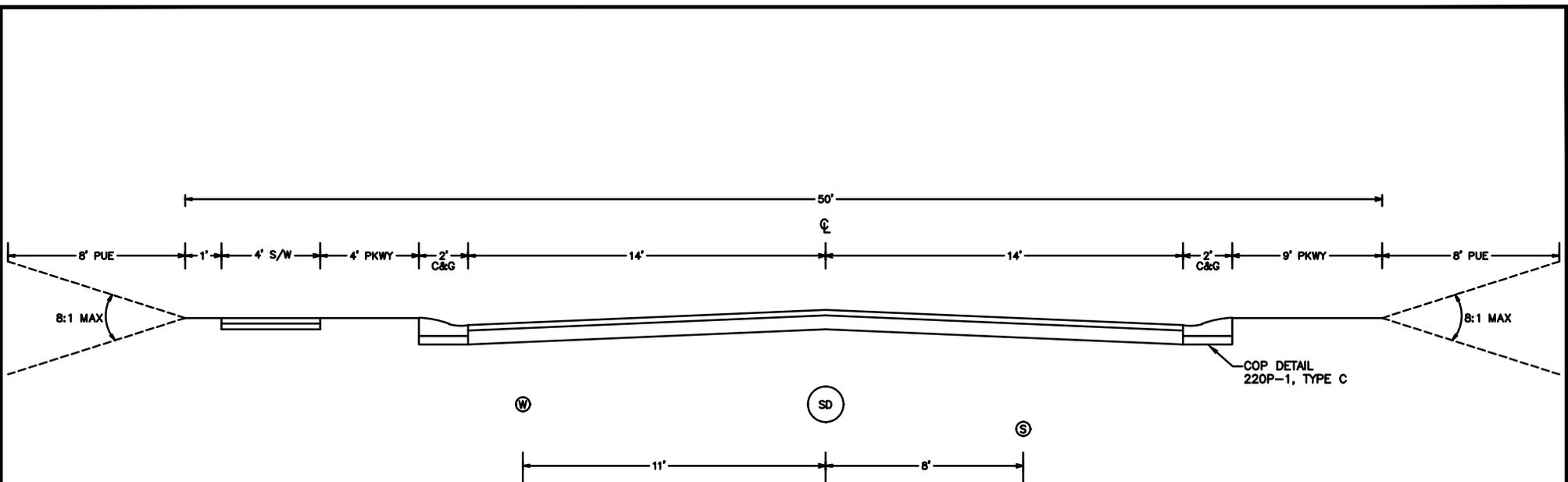


THIS STANDARD SHALL BE APPLIED TO EACH MINOR LEG OF AN INTERSECTION THAT EXCEEDS 8% RUNNING SLOPE WITHIN THE INTERSECTION. 3% MAY BE ALLOWED WITHIN THE INTERSECTION TO BE DETERMINED BY AGENCY ENGINEER. GRADES SHOWN ARE AT MINOR STREET CENTERLINE. INDIVIDUAL CONSIDERATION SHALL BE GIVEN AT THE CURB LINE TO INSURE POSITIVE DRAINAGE AT THE VALLEY GUTTER. SUITABLE MEASURES SHALL BE TAKEN WHERE NECESSARY TO INSURE THAT THE PROPER DRAINAGE PATTERN IS OBTAINED AT THE INTERSECTION. LARGER SCALE INTERSECTION DETAILS MAY BE REQUIRED.

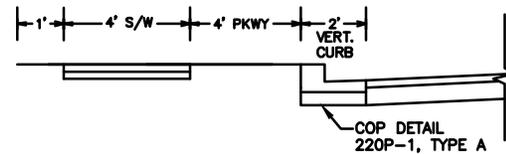
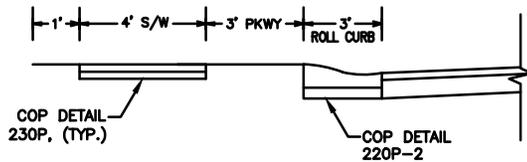


DESIGN SPEED = 30MPH
 POSTED SPEED = 25MPH

- NOTES:
1. PLACE "NO PARKING" SIGNS ON BOTH SIDES OF STREET AT 300' SPACING.
 2. MINIMUM 3" AC OVER 6" ABC PAVEMENT STRUCTURE OR PER APPROVED PAVEMENT DESIGN REPORT, WHICHEVER IS GREATER.



DESIGN SPEED = 30MPH
 POSTED SPEED = 25MPH



NOTES:

1. PLACE "NO PARKING" SIGNS ON SIDEWALK SIDE.
2. VERTICAL CURB SHALL BE USED AT CURB RETURNS, ADJACENT TO COMMON AREAS, AND OTHER AREAS TO RESTRICT VEHICLE ACCESS.
3. ROLLED CURB SHALL BE USED ADJACENT TO RESIDENTIAL LOTS.
4. SIX INCH HIGH ROLLED CURB, COP DETAIL 220P, MAY BE USED TO INCREASE STREET DRAINAGE CAPACITY.
5. MINIMUM 4" AC OVER 6" ABC PAVEMENT STRUCTURE OR PER APPROVED PAVEMENT DESIGN REPORT, WHICHEVER IS GREATER.

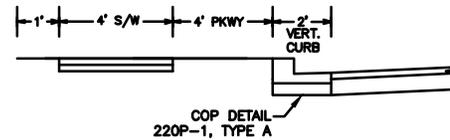
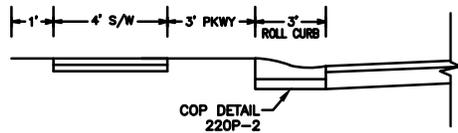
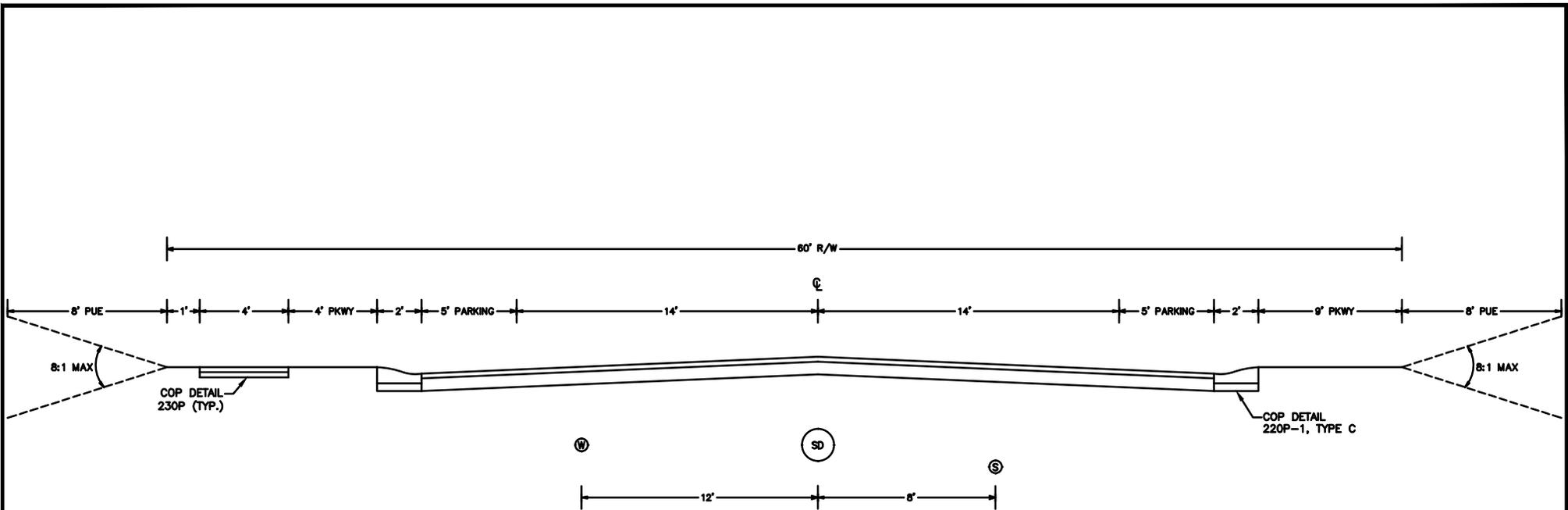
COP STANDARD DETAIL

LOCAL RESIDENTIAL STREET

Charles Andrews
 CITY ENGINEER

REVISED:
 07/16

DETAIL No.
 602P



DESIGN SPEED = 30MPH
 POSTED SPEED = 25MPH

NOTES:

1. VERTICAL CURB SHALL BE USED AT CURB RETURNS, ADJACENT TO COMMON AREAS, AND OTHER AREAS TO RESTRICT VEHICLE ACCESS.
2. ROLLED CURB SHALL BE USED ADJACENT TO RESIDENTIAL LOTS.
3. SIX INCH HIGH ROLLED CURB, COP DETAIL 220P-2, MAY BE USED TO INCREASE STREET DRAINAGE CAPACITY.
4. MINIMUM 4" AC OVER 6" ABC PAVEMENT STRUCTURE OR PER APPROVED PAVEMENT DESIGN REPORT, WHICHEVER IS GREATER.

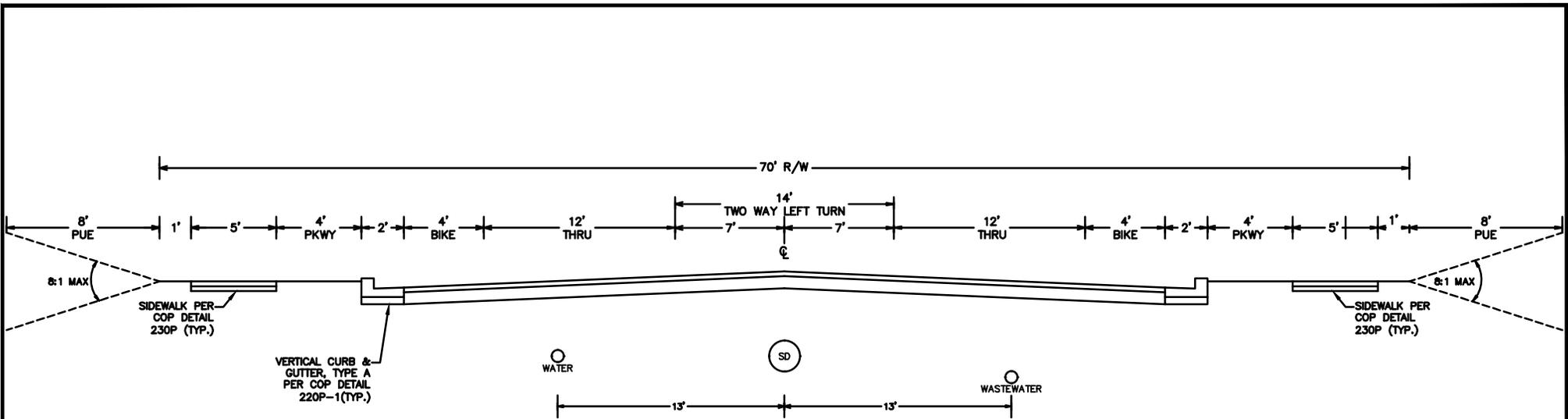
COP STANDARD DETAIL

RESIDENTIAL COLLECTOR

Charles Andrews
 CITY ENGINEER

REVISED: 07/16

DETAIL No. 603P

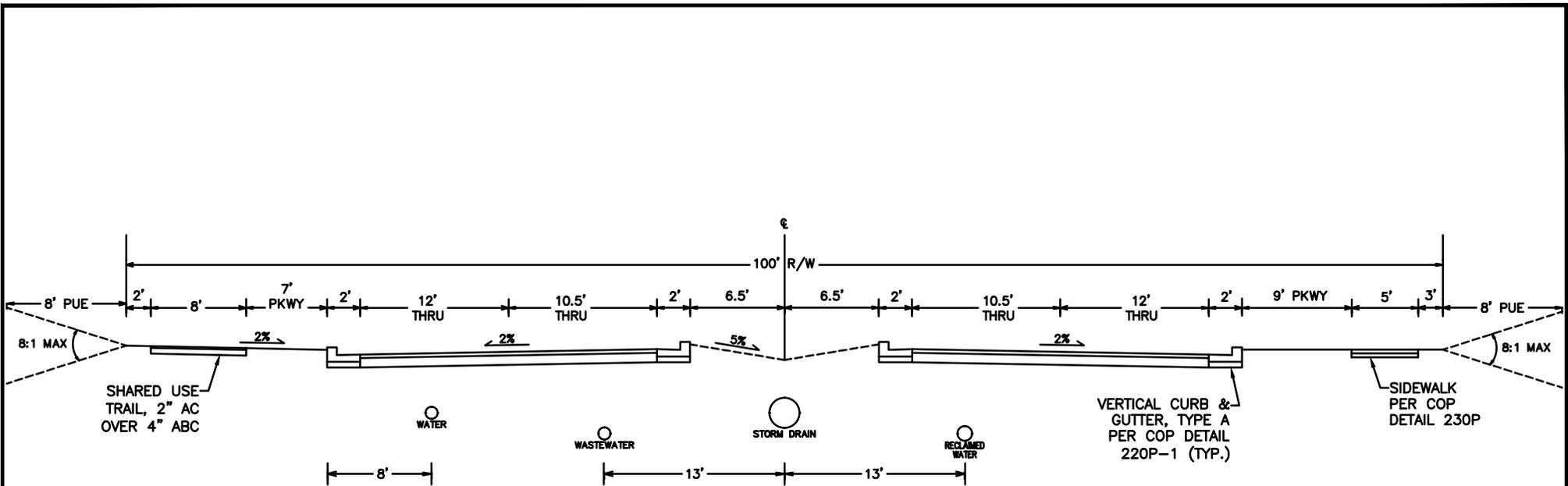


DESIGN SPEED = 40MPH
 POSTED SPEED = 30 TO 35MPH

NOTE:

MINIMUM 5" AC OVER 8" ABC PAVEMENT
 STRUCTURE OR PER APPROVED PAVEMENT DESIGN
 REPORT, WHICHEVER IS GREATER.

| | | | | |
|---------------------|---------------------------------|---|-------------------|--------------------|
| COP STANDARD DETAIL | COMMERCIAL/INDUSTRIAL COLLECTOR | <i>Charles Andrews</i> CITY ENGINEER | REVISED: 07/16 | DETAIL No. 604P |
|---------------------|---------------------------------|---|-------------------|--------------------|



DESIGN SPEED = 45MPH
 POSTED SPEED = 35 TO 40MPH

NOTE:

MINIMUM 6" AC OVER 10" ABC PAVEMENT
 STRUCTURE OR PER APPROVED PAVEMENT
 DESIGN REPORT, WHICHEVER IS GREATER.

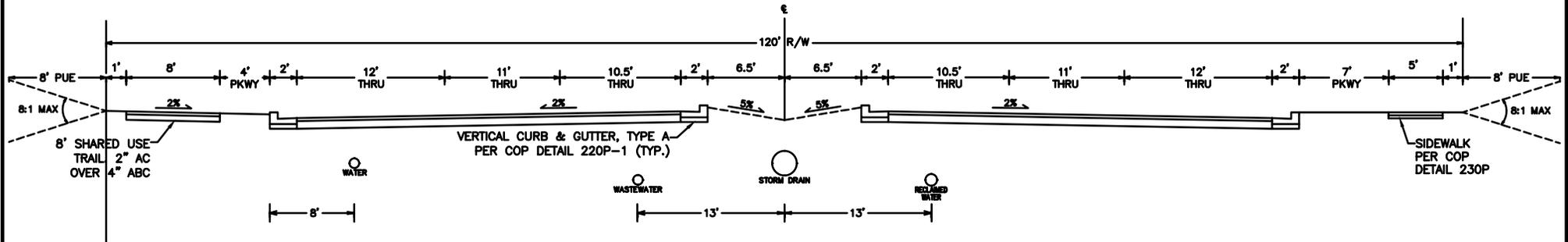
COP STANDARD DETAIL

MINOR ARTERIAL

Charles Andrews
 CITY ENGINEER

REVISED:
 07/16

DETAIL No.
 605P



DESIGN SPEED = 55MPH
 POSTED SPEED = 45MPH

NOTE:

MINIMUM 6" AC OVER 10" ABC PAVEMENT
 STRUCTURE OR PER APPROVED PAVEMENT
 DESIGN REPORT, WHICHEVER IS GREATER.

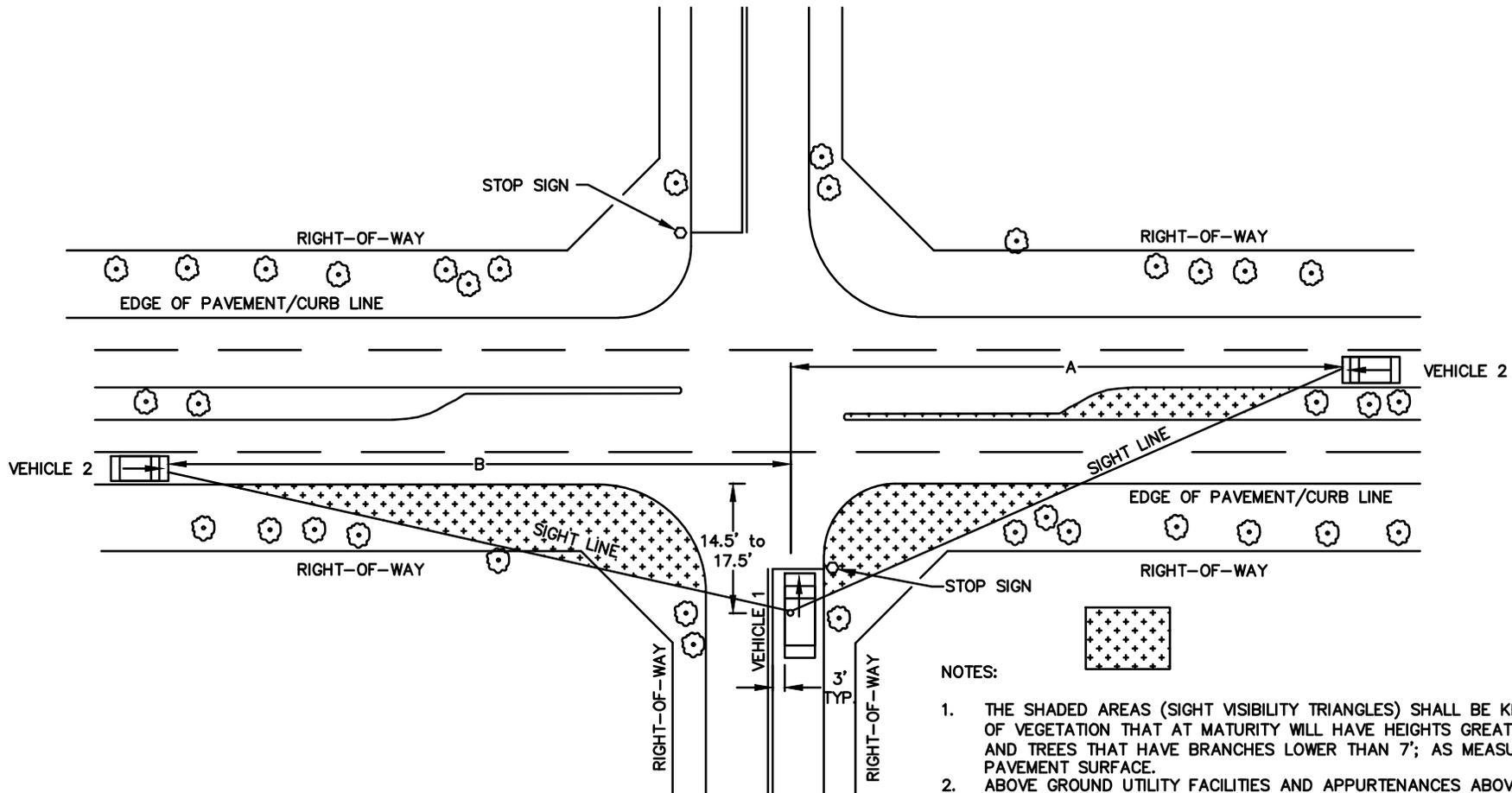
COP STANDARD DETAIL

MAJOR ARTERIAL

Charles Andrews
 CITY ENGINEER

REVISED:
 07/16

DETAIL No.
 606P



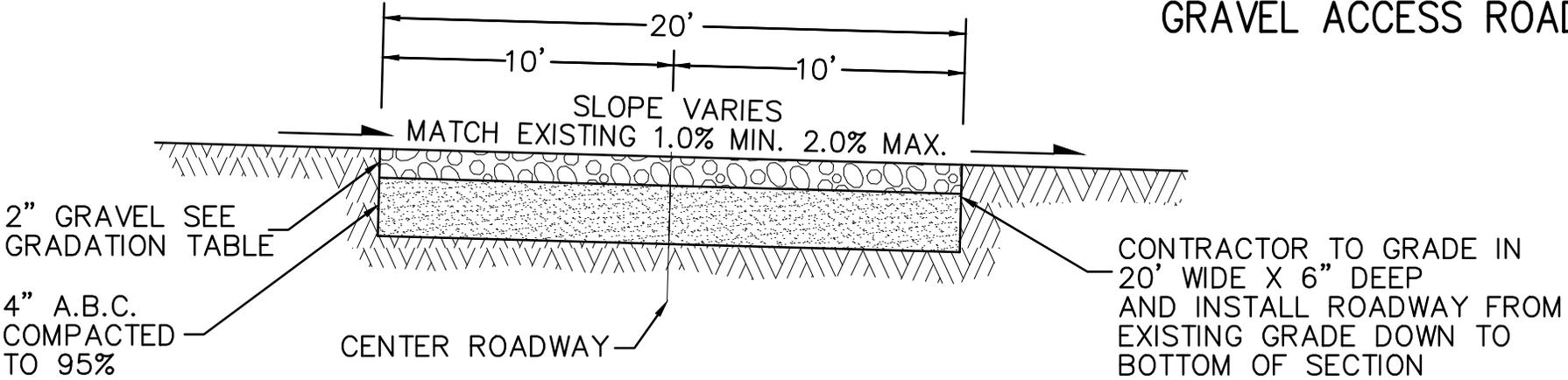
NOTES:

1. THE SHADED AREAS (SIGHT VISIBILITY TRIANGLES) SHALL BE KEPT CLEAR OF VEGETATION THAT AT MATURITY WILL HAVE HEIGHTS GREATER THAN 3' AND TREES THAT HAVE BRANCHES LOWER THAN 7'; AS MEASURED FROM PAVEMENT SURFACE.
2. ABOVE GROUND UTILITY FACILITIES AND APPURTENANCES ABOVE 3 FEET IN HEIGHT SHALL NOT BE LOCATED WITHIN THE SIGHT VISIBILITY TRIANGLES.
3. SIGNAGE APPROVED BY THE CITY FOR USE IN THE ROW MAY BE LOCATED WITHIN THE SIGHT VISIBILITY TRIANGLES.
4. THE LINE OF SIGHT SHALL BE SHOWN AT INTERSECTIONS ON ALL LANDSCAPING PLANS, GRADING PLANS, AND TENTATIVE TRACT PLANS WHERE SAFE SIGHT DISTANCE IS QUESTIONABLE. IN CASES WHERE AN INTERSECTION IS LOCATED ON A VERTICAL CURVE, A PROFILE OF THE SIGHT LINE MAY BE REQUIRED.
5. TO ESTABLISH THE LINE OF SIGHT, VEHICLE 1 SHOULD BE POSITIONED SO THAT THE DRIVERS EYE IS 14.5 TO 17.5' BACK FROM THE EDGE OF PAVEMENT/ FACE OF CURB AND 3.5' ABOVE THE PAVEMENT. DRIVER IS ASSUMED TO BE 3.0' RIGHT OF CENTER LINE IN LANE.
6. APPROACH VEHICLE (VEHICLE 2) IS POSITIONED IN THE CENTER OF ITS LANE AND ASSUMED TO BE 4.25' ABOVE THE PAVEMENT.
7. DRAWING DEPICTS TYPICAL PASSENGER CAR SITUATION WITHOUT GRADES. ADJUSTMENTS FOR GRADES SHALL BE MADE PER AASHTO.
8. EASEMENTS TO BE SHOWN AND DIMENSIONED ON FINAL PLAT.

A = SIGHT DISTANCE TO RIGHT FOR VEHICLE 1
 B = SIGHT DISTANCE TO LEFT FOR VEHICLE 1

| DESIGN SPEED (85th PERCENTILE) OF INTERSECTING ROADWAY | SIGHT DISTANCE FOR PASSENGER VEHICLE 1 TO TURN LEFT OR RIGHT | | | | |
|---|--|--------|------------------------|-----------------|------|
| | A | | | | B |
| | 1-LANE | 2-LANE | 3-LANE/2-LANE & MEDIAN | 3-LANE & MEDIAN | |
| 25 MPH | 280' | 295' | | | 240' |
| 30 MPH | 335' | 355' | | | 290' |
| 35 MPH | 390' | 415' | 440' | 465' | 335' |
| 40 MPH | 445' | 475' | 500' | 530' | 385' |
| 45 MPH | 500' | 530' | 565' | 600' | 430' |
| 50 MPH | 555' | 590' | 625' | 665' | 480' |
| 55 MPH | 610' | 650' | 690' | 730' | 530' |
| 60 MPH | | 710' | 750' | 795' | 575' |
| 65 MPH | | 765' | 815' | 860' | 625' |

GRAVEL ACCESS ROAD



DETAIL "A"

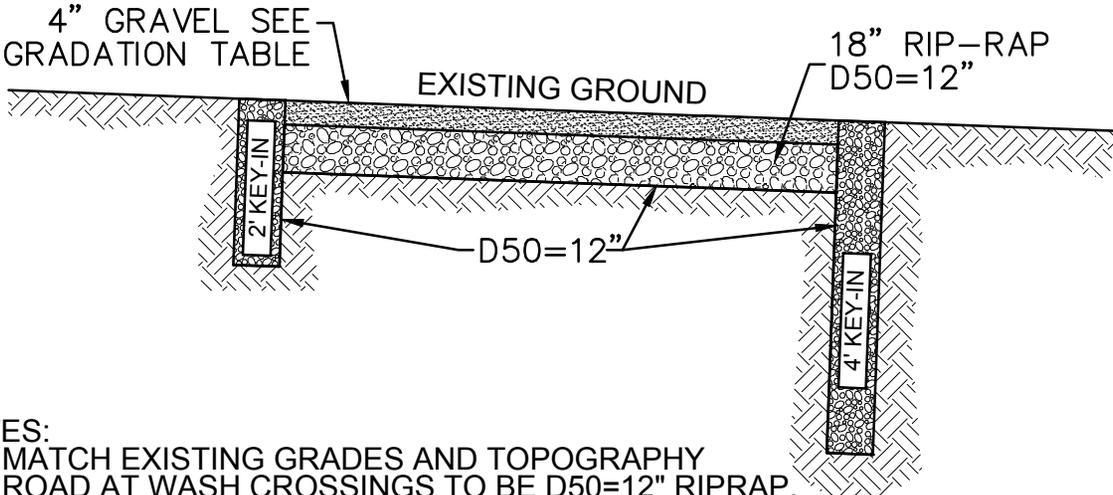
Not To Scale

Crushed Aggregate* Gradation Table

| Sieve Sizes (square openings) | Percentage by Weight Passing Sieve |
|----------------------------------|---------------------------------------|
| 2" | 100 |
| 1.5" | 50-90 |
| 1.25" | 30-50 |
| 1" | 20-30 |
| 0.75" | 0-20 |

*CRUSHED AGGREGATE SHALL CONSIST OF CRUSHED ROCK WITH 75% OF THE SPECIFIED SIZE HAVING AT LEAST ONE FRACTURED FACE TESTED IN ACCORDANCE WITH ARIZ-212.

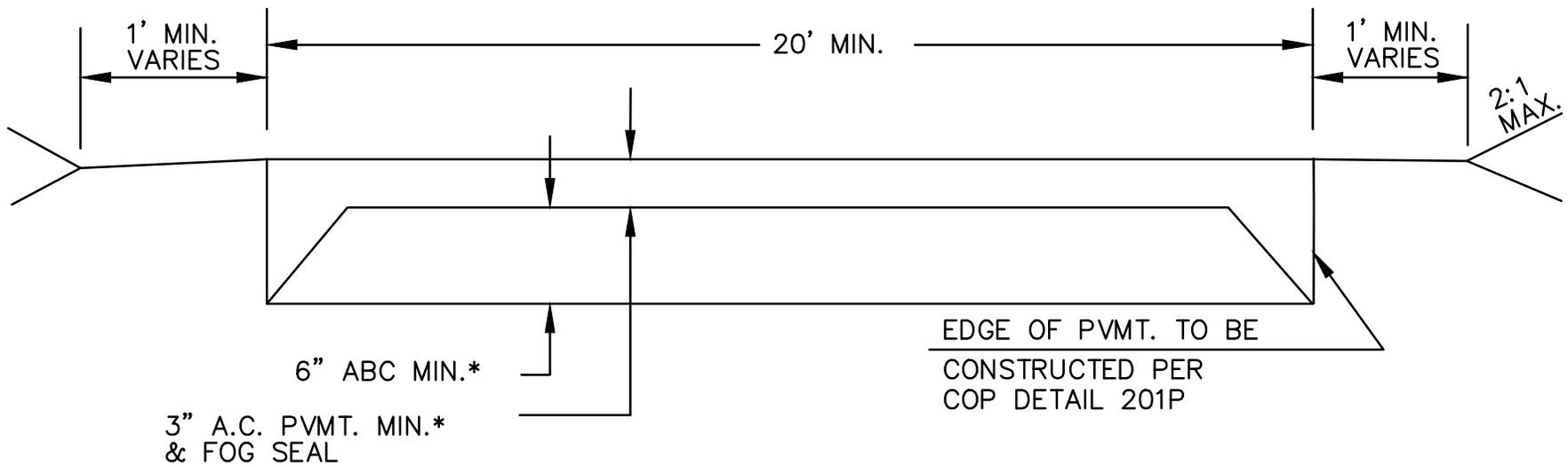
GRADATION TABLE FOR ROAD SECTIONS A & B



DETAIL "B" - IN LOW WATER CROSSING

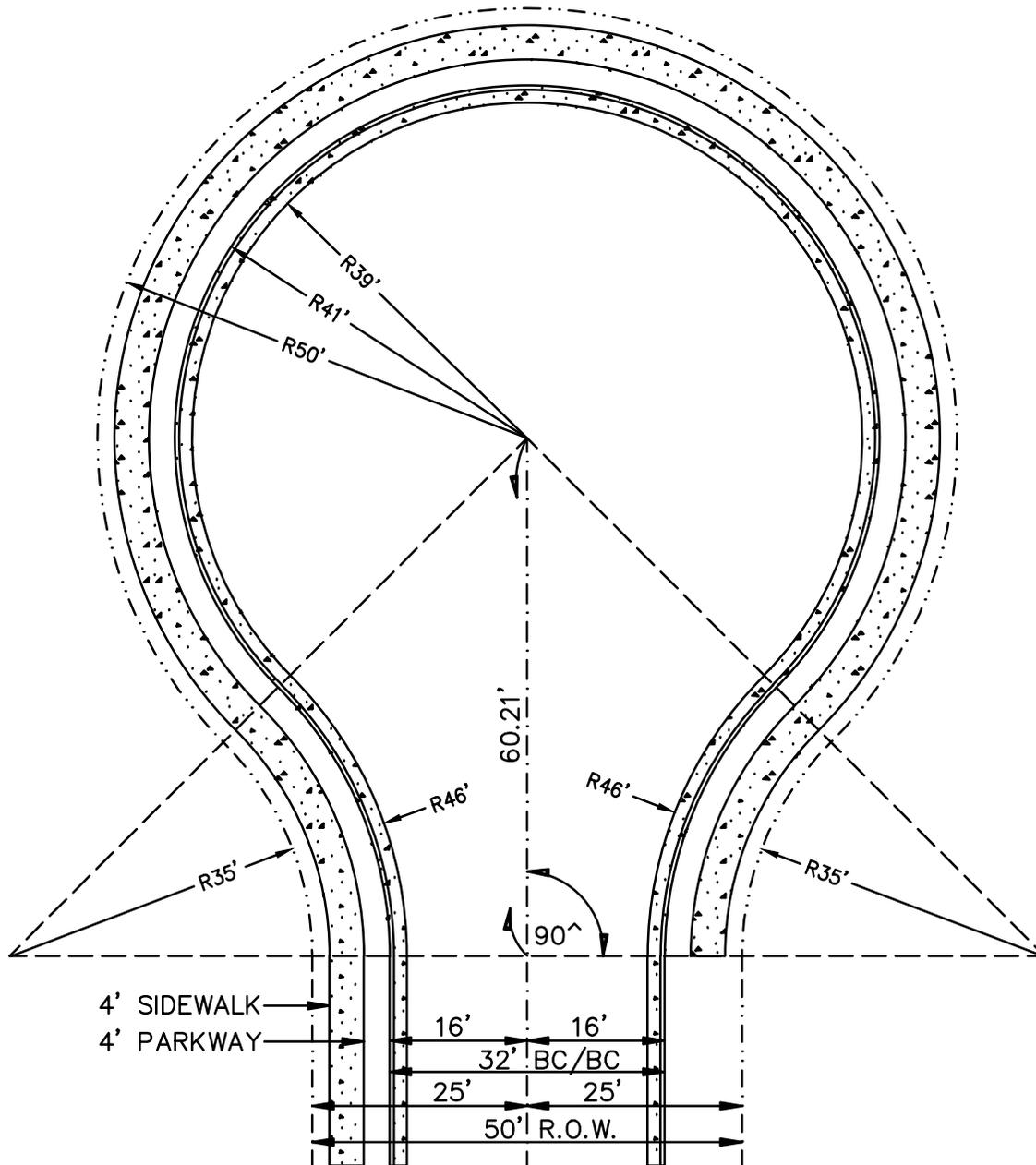
Not To Scale

- NOTES:
1. MATCH EXISTING GRADES AND TOPOGRAPHY
 2. ROAD AT WASH CROSSINGS TO BE D50=12" RIPRAP, 2'-WIDE KEY-IN TO DEPTH, 2' UPSTREAM AND 4' DOWNSTREAM
 3. FILL VOIDS WITH NATIVE MATERIAL AFTER PLACEMENT



TYPICAL CROSS SECTION
(NO SCALE)

* NOTE:
ACTUAL STRUCTURAL SECTION SHALL
BE DETERMINED BY GEO-TECHNICAL
ENGINEER & DESIGNED FOR HEAVY
EQUIPMENT



NOTE:

ADD 1' TO BACK OF CURB
DIMENSION WHERE 6" ROLL
CURB IS USED.

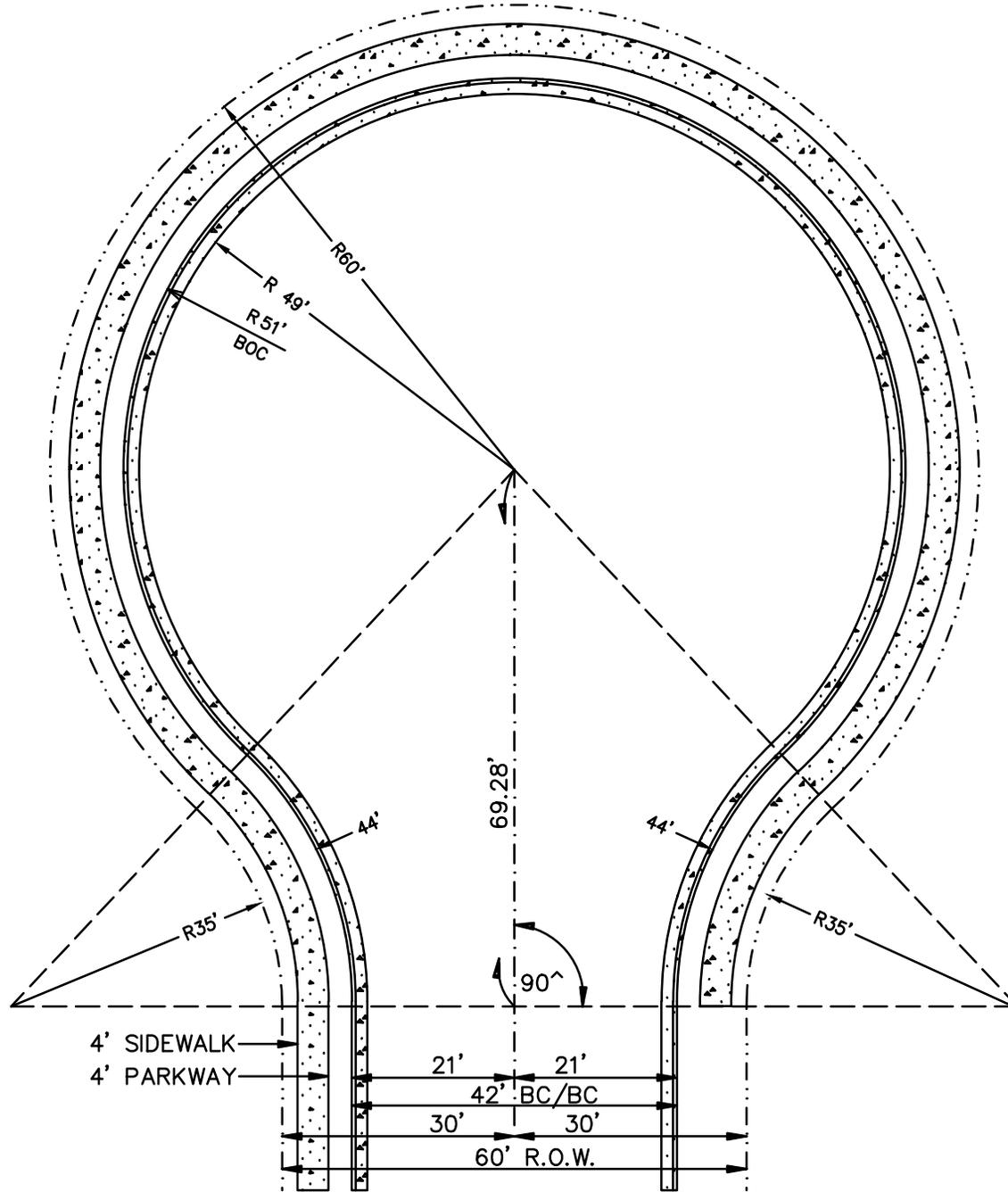
COP STANDARD DETAIL

CUL-DE-SAC FOR
50' R.O.W. STREET

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
620P-1



NOTE:

ADD 1' TO BACK OF CURB DIMENSION WHERE 6" ROLL CURB IS USED.

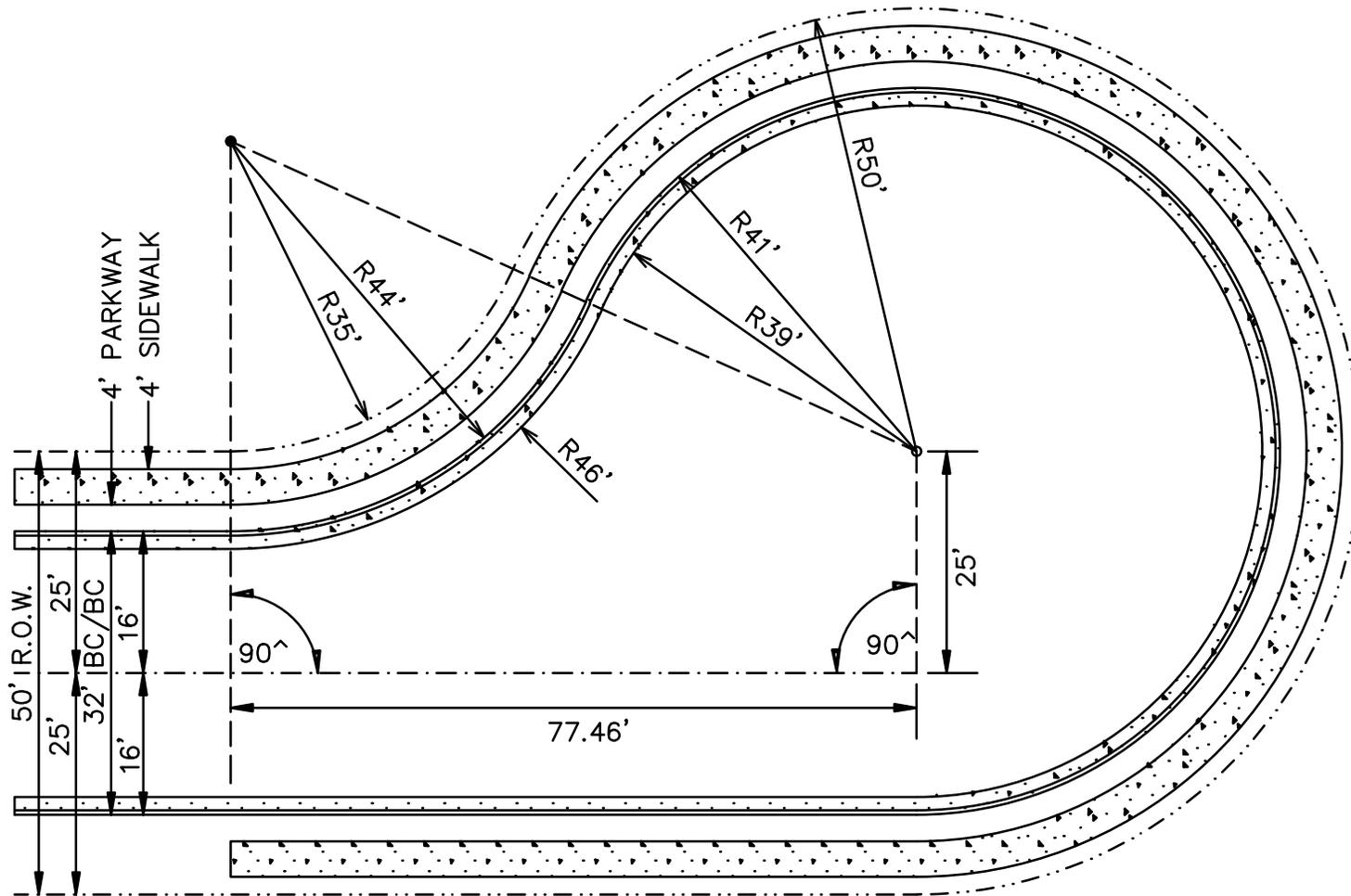
COP STANDARD DETAIL

CUL-DE-SAC FOR
60' R.O.W. STREET

Charles Andrews
CITY ENGINEER

REVISED:
07/16

DETAIL No.
620P-2



NOTE:
 ADD 1' TO BACK OF
 CURB DIMENSION WHERE
 6" ROLL CURB IS USED.

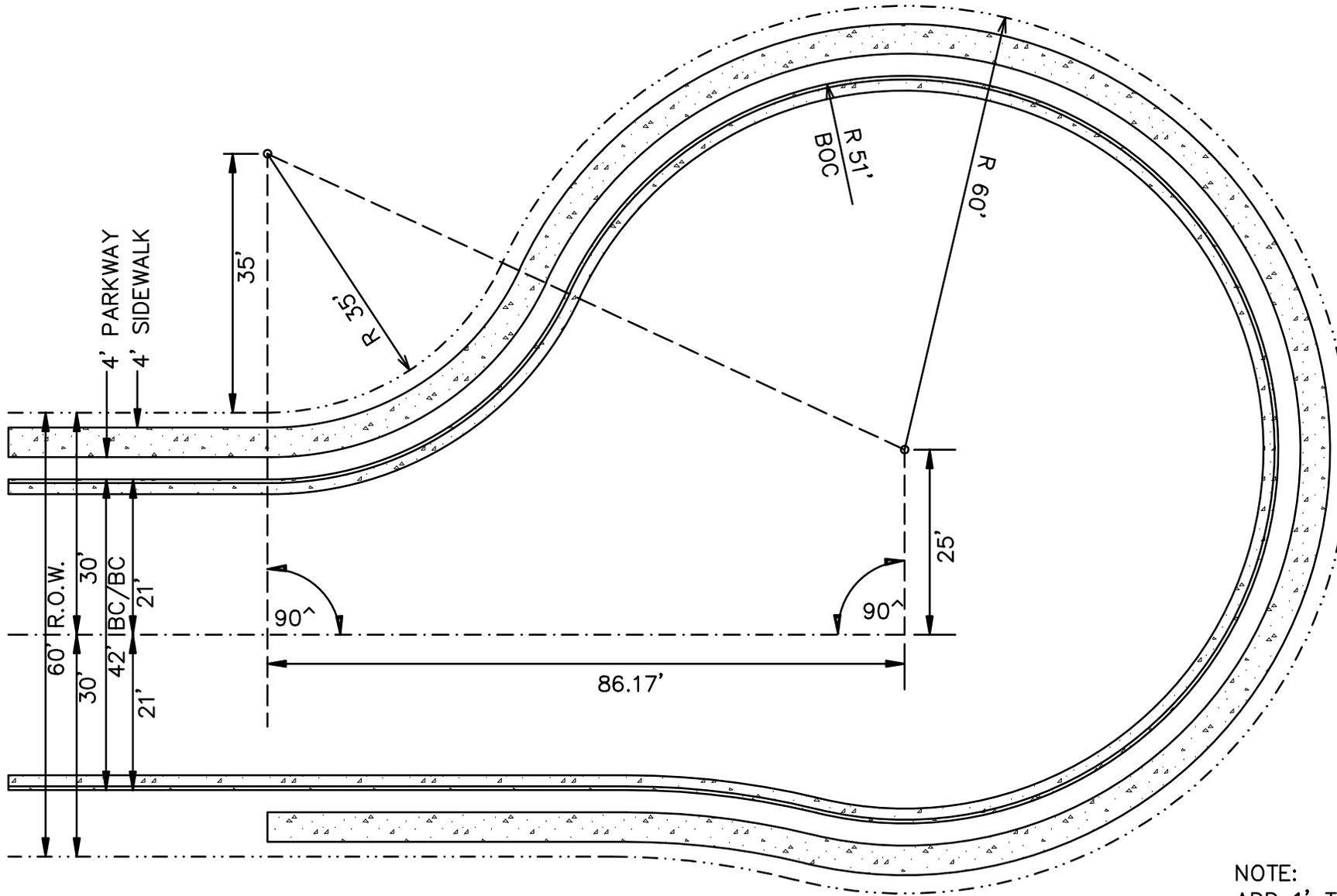
COP STANDARD DETAIL

OFFSET CUL-DE-SAC FOR 50'
 R.O.W. STREET

Charles Andrews
 CITY ENGINEER

REVISED:
 07/16

DETAIL No.
 621P-1



NOTE:
 ADD 1' TO BACK OF
 CURB DIMENSION WHERE
 6" ROLL CURB IS USED.

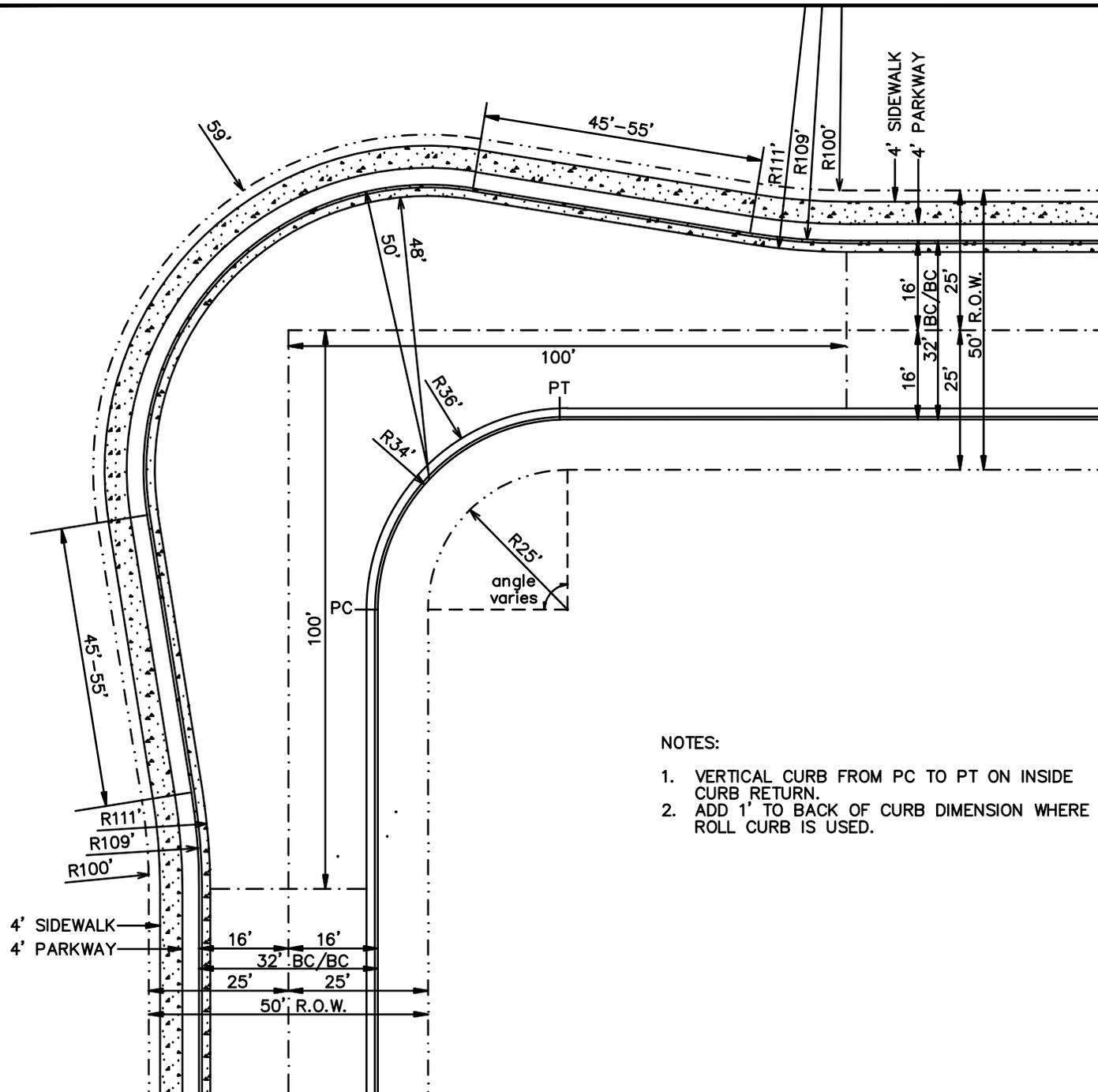
COP STANDARD DETAIL

OFFSET CUL-DE-SAC FOR 60'
 R.O.W. STREET

Charles Andrews
 CITY ENGINEER

REVISED:
 07/16

DETAIL No.
 621P-2



NOTES:

1. VERTICAL CURB FROM PC TO PT ON INSIDE CURB RETURN.
2. ADD 1' TO BACK OF CURB DIMENSION WHERE 6" ROLL CURB IS USED.

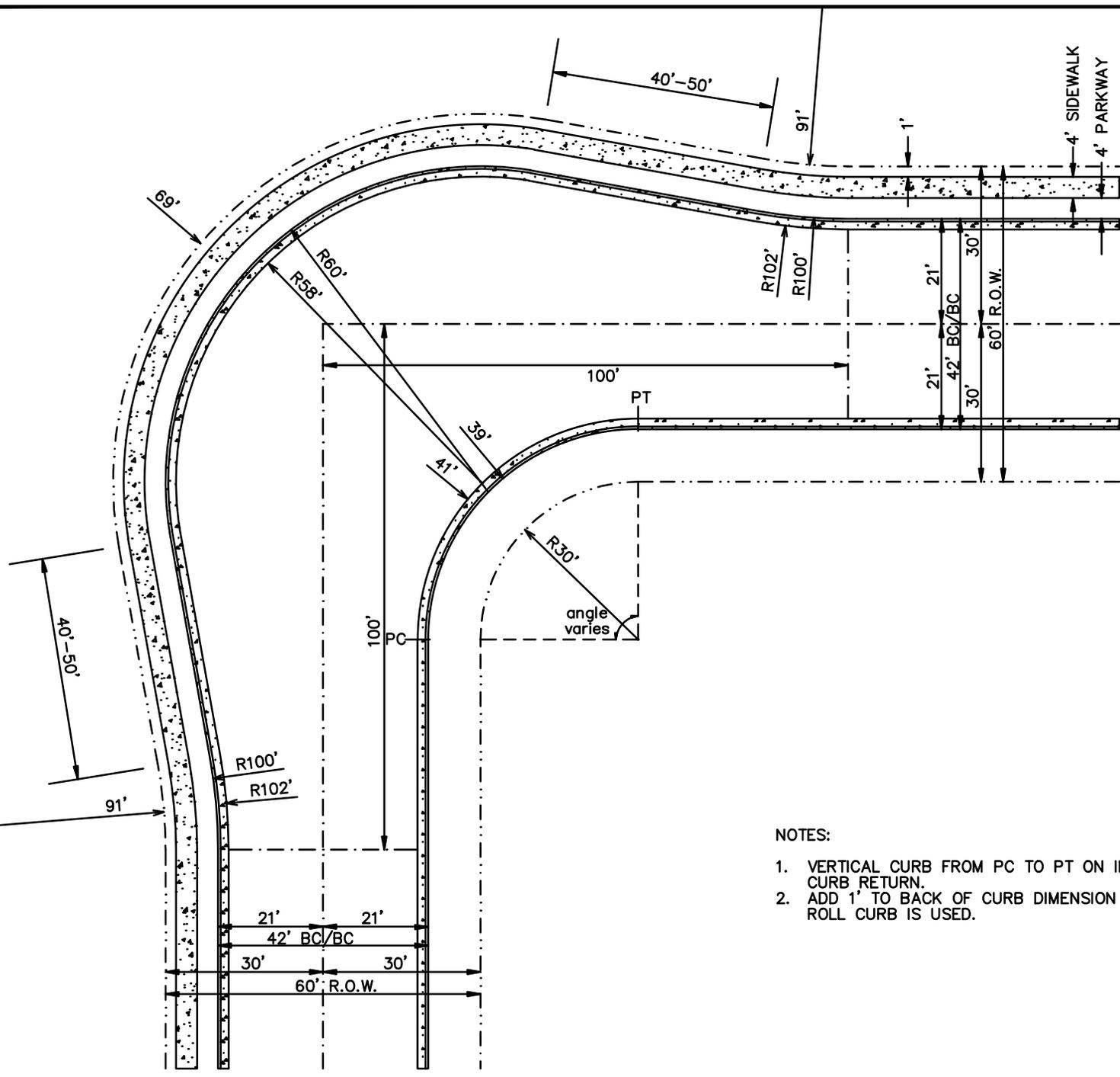
COP STANDARD DETAIL

KNUCKLE FOR 50' R.O.W.

Charles Andrews
CITY ENGINEER

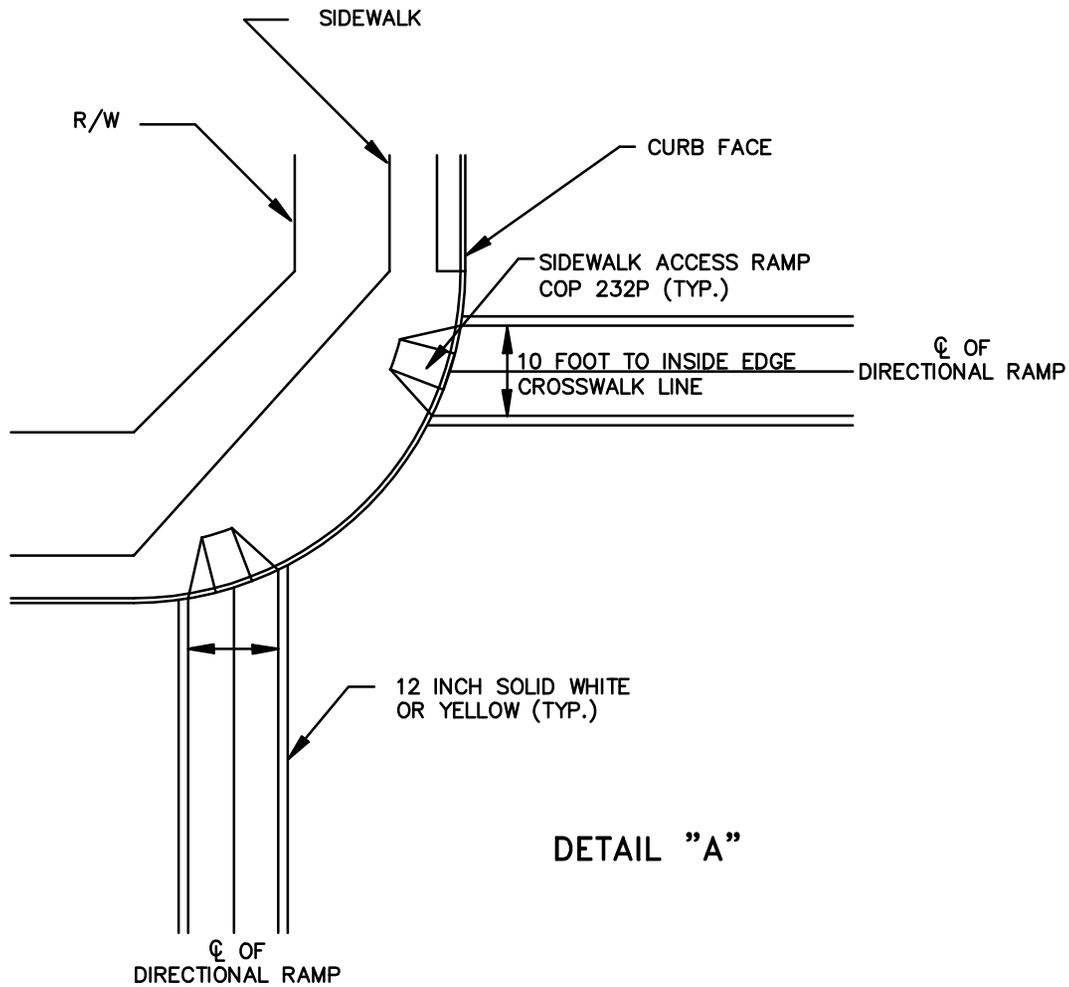
REVISED:
07/16

DETAIL No.
622P-1

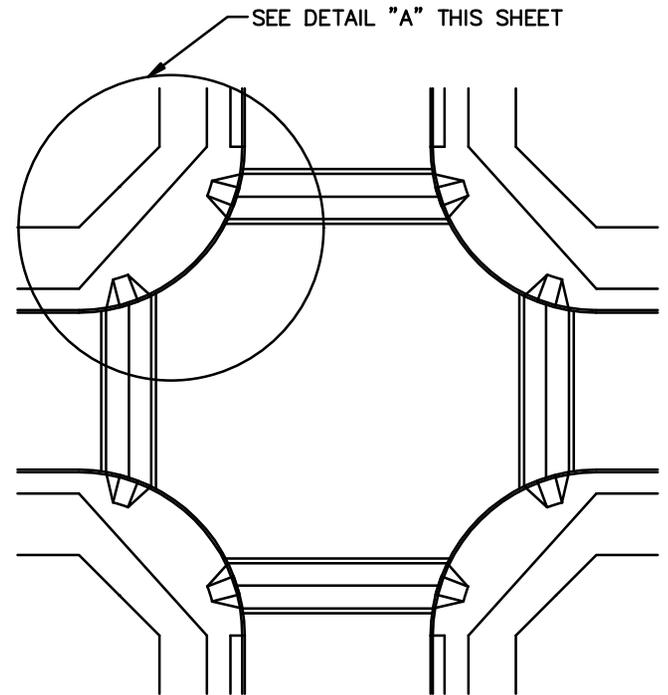


NOTES:

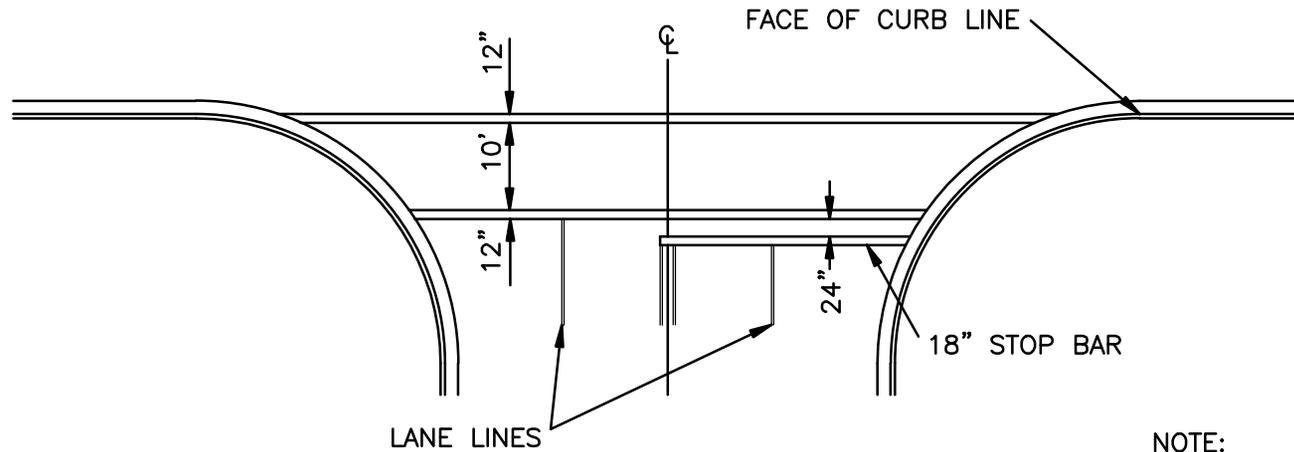
1. VERTICAL CURB FROM PC TO PT ON INSIDE CURB RETURN.
2. ADD 1' TO BACK OF CURB DIMENSION WHERE 6" ROLL CURB IS USED.



DETAIL "A"



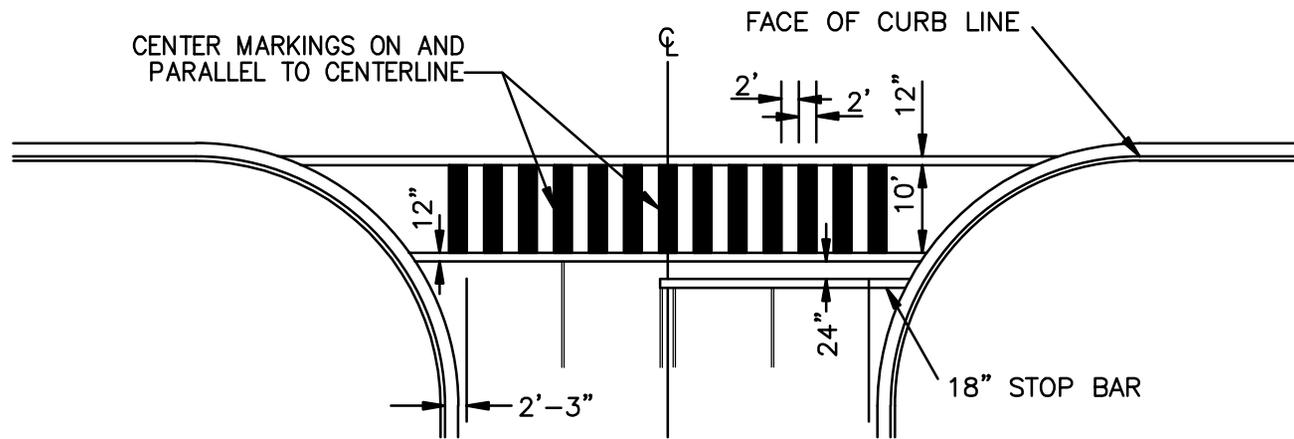
NOTE: ALL LANE LINES TO TERMINATE AT STOP BAR



STANDARD CROSSWALK MARKING

WIDTH OF LINES 12" STANDARD
WIDTH OF WALK 10' STANDARD

NOTE:
CROSSWALKS AND STOP BAR
MARKINGS SHALL BE 90 MIL
(0.090 INCH) THICK ALKYD
EXTRUDED THERMOPLASTIC.



24" BLOCK (LONGITUDINAL)

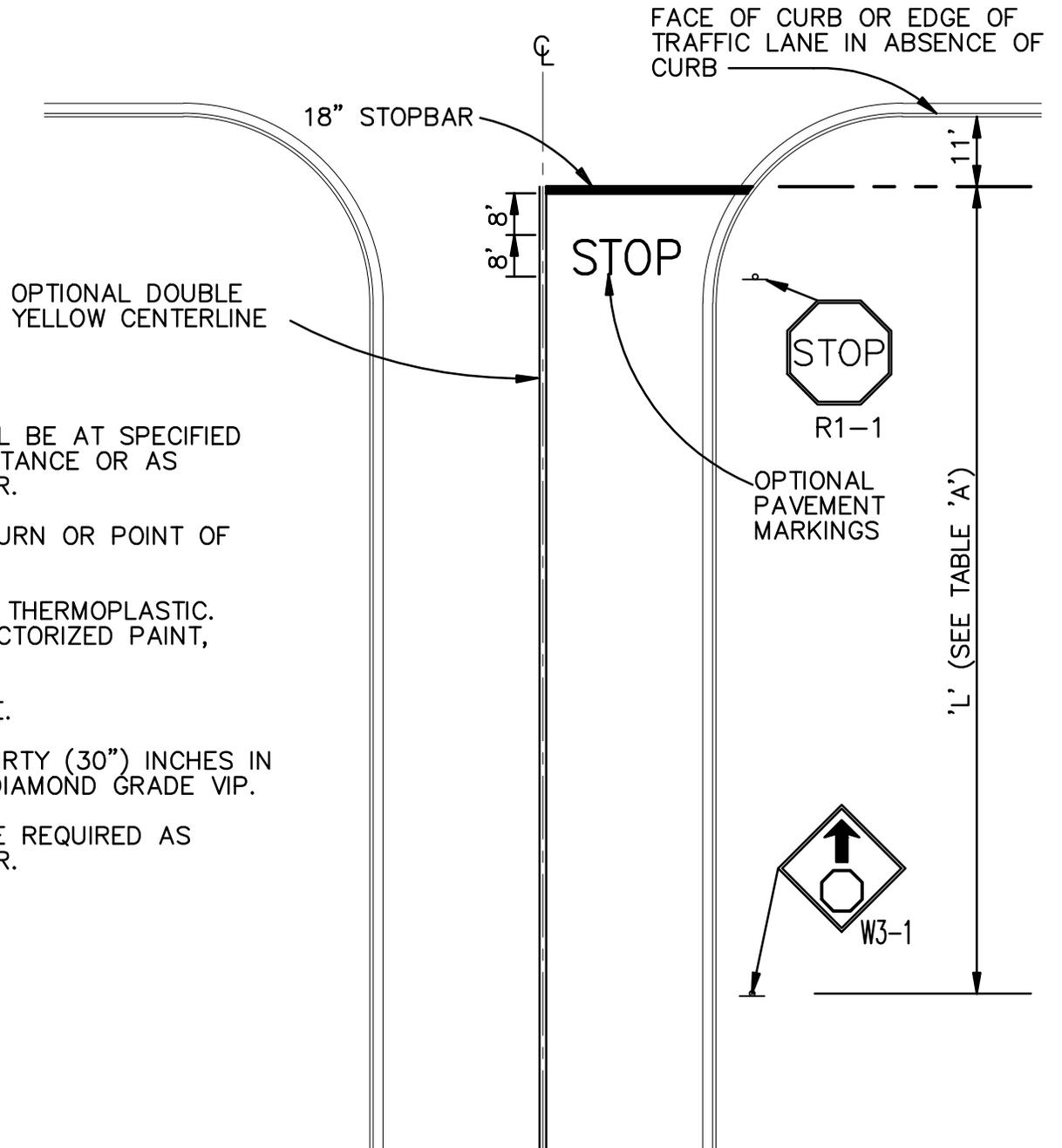
USE ONLY WHERE SPECIFIED

TABLE 'A'
'L' VALUES (FT.)

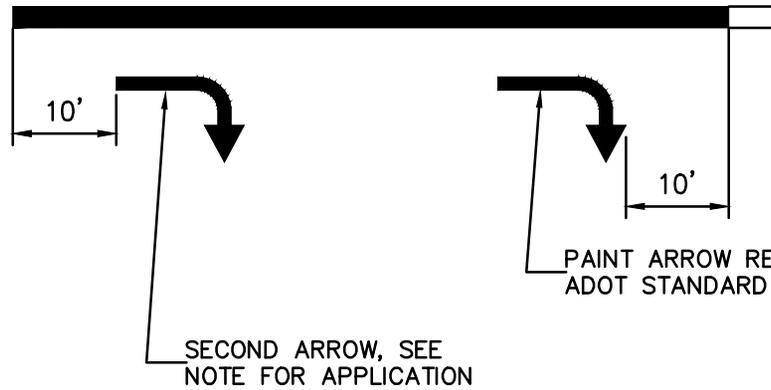
| MPH | DOWNGRADES | | | |
|------------------------|------------|-----|-----|-----|
| | 0% | 3% | 6% | 9% |
| 25 OR NOT POSTED | 250 | 260 | 270 | 290 |
| 30 | 305 | 315 | 335 | 355 |
| 35 | 360 | 380 | 400 | 430 |

NOTES:

1. THE USE OF W3-1 'STOP AHEAD' SIGNS SHALL BE AT SPECIFIED LOCATIONS DEMONSTRATING LIMITED SIGHT DISTANCE OR AS DIRECTED BY THE AGENCY'S TRAFFIC ENGINEER.
2. STOP SIGN NORMALLY LOCATED AT CURB RETURN OR POINT OF OPTIMUM VISIBILITY.
3. STOP BARS SHALL BE REFLECTORIZED 90 MIL THERMOPLASTIC. DOUBLE YELLOW CENTERLINE SHALL BE REFLECTORIZED PAINT, 24-28 MIL.
4. CENTER PAVEMENT MARKINGS IN TRAVEL LANE.
5. R1-1 STOP SIGN SHALL BE A MINIMUM OF THIRTY (30") INCHES IN WIDTH. ALL SIGN SHEETING MATERIAL TO BE DIAMOND GRADE VIP.
6. OPTIONAL "STOP" PAVEMENT MARKING MAY BE REQUIRED AS DIRECTED BY THE AGENCY'S TRAFFIC ENGINEER.



TURN LANE ARROWS

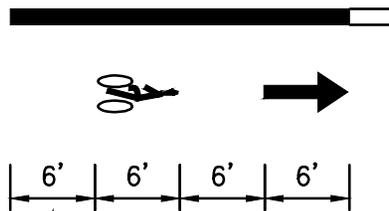


NOTE: IF TURN LANE EXCEEDS 100', ADD SECOND ARROW 10' FROM BACK END OF LANE LANE.

PAIN T ARROW REQUIRED
ADOT STANDARD DRAWING M-10

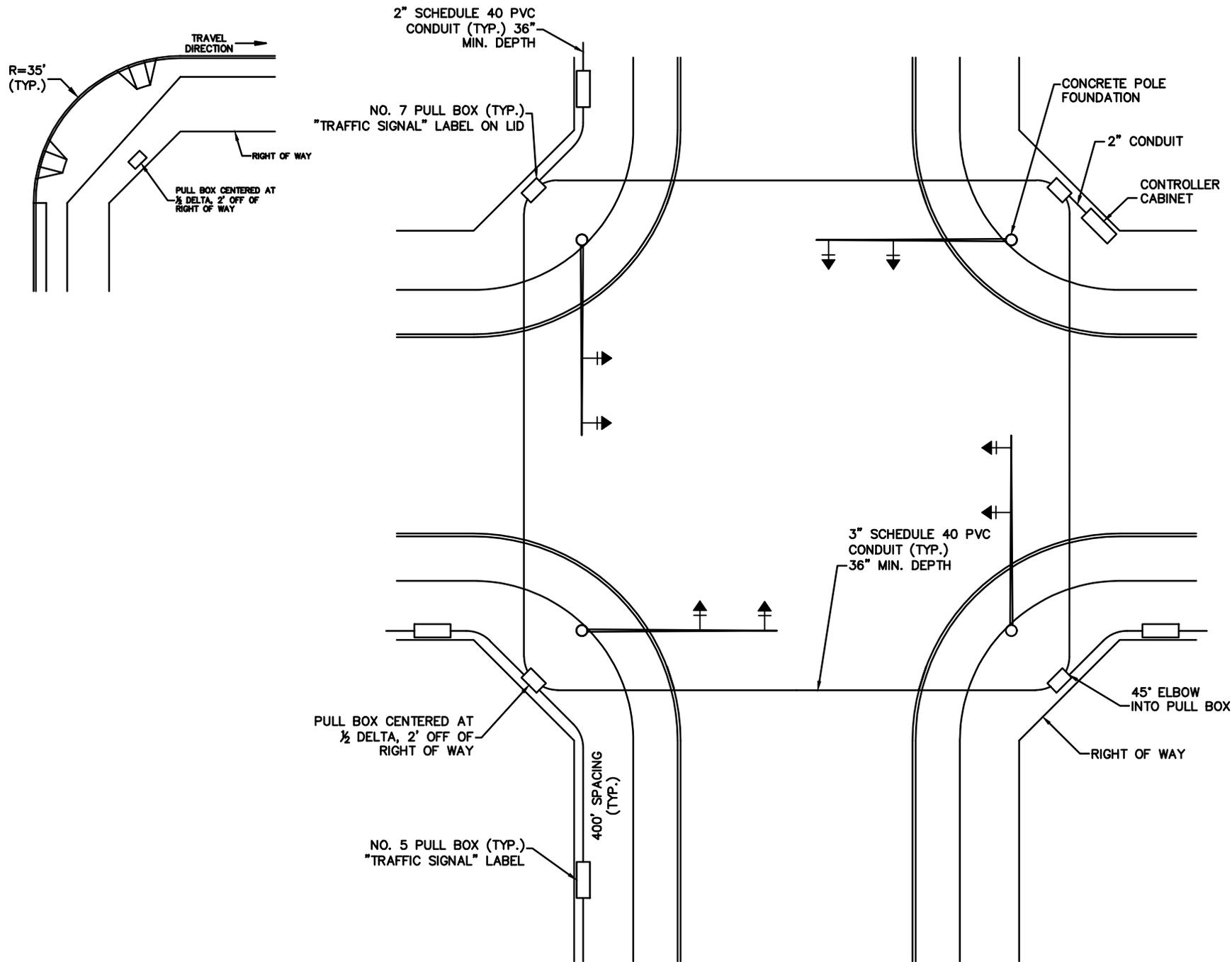
SECOND ARROW, SEE
NOTE FOR APPLICATION

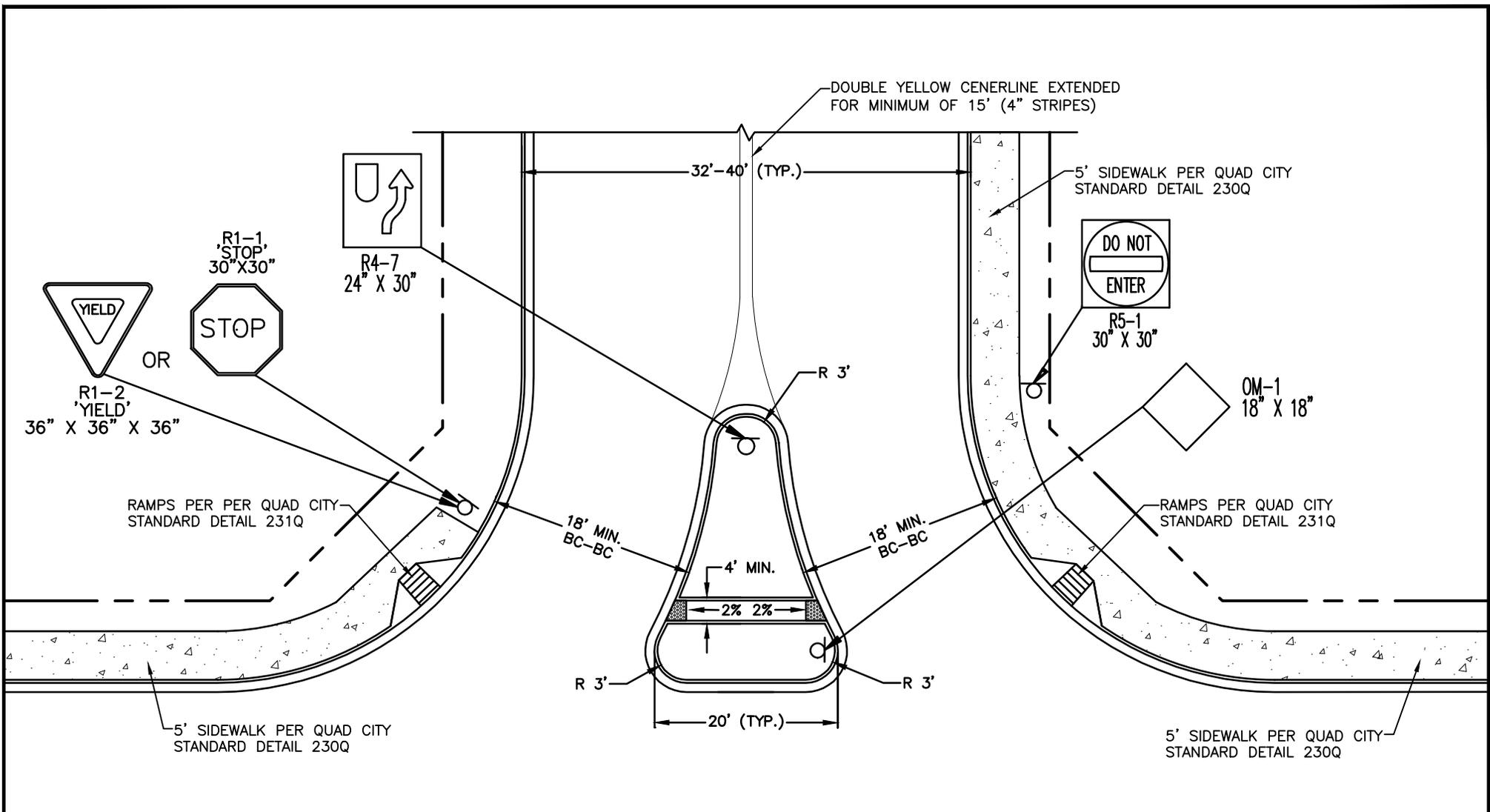
BIKE LANE MARKINGS



6' FROM CURB RETURN OR BEGINNING OF RIGHT TURN BAY

NOTE: ALL TURN LANE ARROWS AND BIKE SYMBOLS SHALL CONFORM TO ADOT SPECIFICATION 704 AND SHALL BE 90 MIL (0.090 INCH) THICK ALKYD EXTRUDED THERMOPLASTIC.

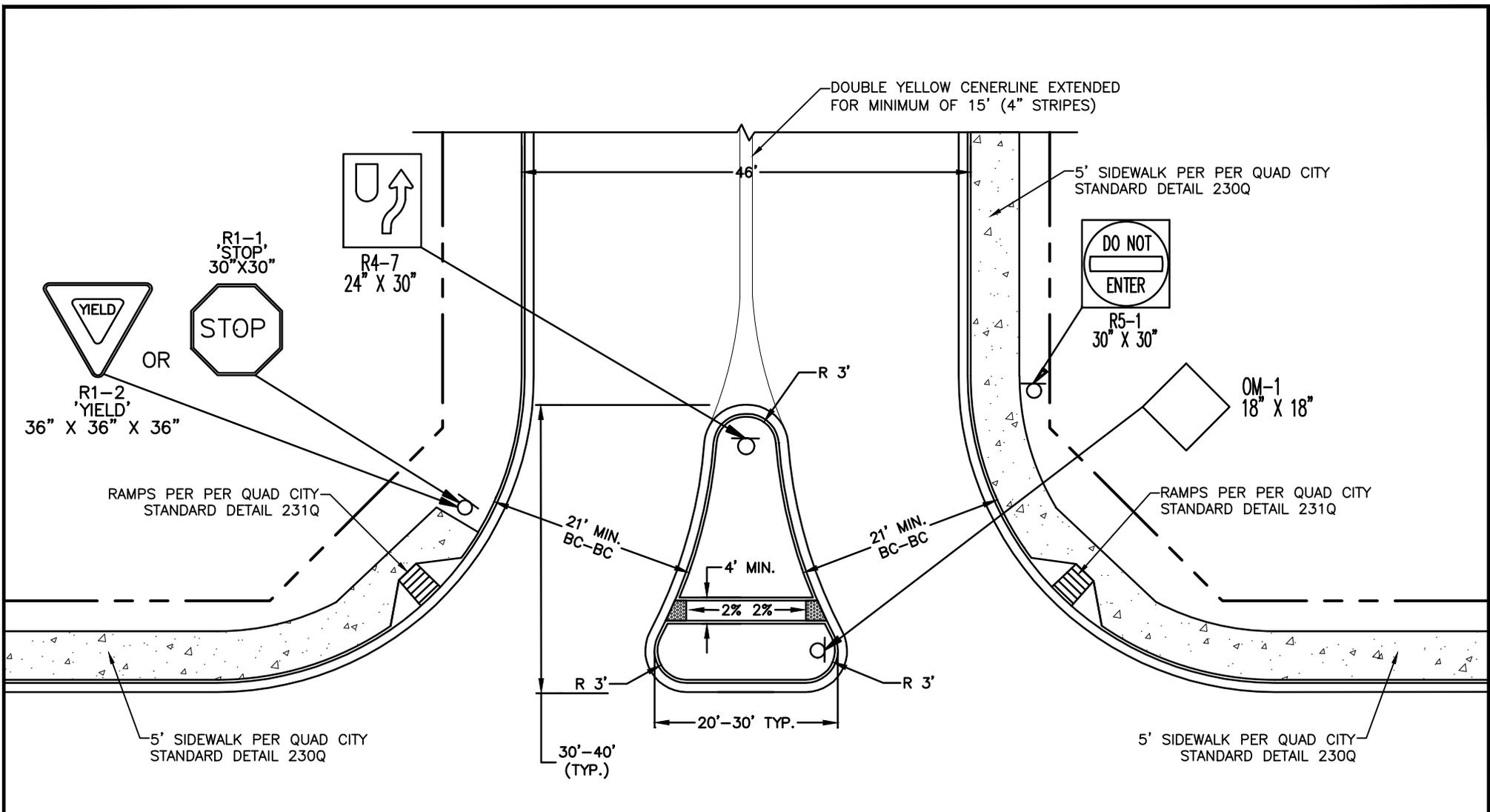




NOTES:

1. CONCRETE ISLAND SHALL BE TYPE 'A' CONCRETE.
2. TURNING LANES NOT SHOWN.
3. LANE WIDTH SHOWN FOR PASSENGER VEHICLE ACCESS.
4. CONCRETE ISLANDS SHALL INCLUDE ADA COMPLIANT PEDESTRIAN ACCESS WITH TRUNCATED DOMES.
5. IF SIDEWALK CONTINUES ONTO SITE, THEN QUAD CITY STANDARD 231Q SIDEWALK RAMP SHALL BE USED.

| | | | |
|----------------------------------|---|---------------------------|------------------------------|
| <p>QUAD CITY STANDARD DETAIL</p> | <p>RESIDENTIAL RIGHT IN/RIGHT OUT PASSENGER VEHICLES ONLY</p> | <p>REVISED: 07/16</p> | <p>DETAIL No. 650Q-1</p> |
|----------------------------------|---|---------------------------|------------------------------|



NOTES:

1. CONCRETE ISLAND SHALL BE TYPE 'A' CONCRETE.
2. TURNING LANES NOT SHOWN.
3. LANE WIDTH SHOWN FOR TRUCK ACCESS.
4. CONCRETE ISLANDS SHALL INCLUDE ADA COMPLIANT PEDESTRIAN ACCESS WITH TRUNCATED DOMES.
5. IF SIDEWALK CONTINUES ONTO SITE, THEN QUAD CITY STANDARD 231Q SIDEWALK RAMP SHALL BE USED.